

- 12.0 GENERAL WOOD FRAMING NOTES:
- 12.1 All top plates shall be SYP #2 or better. All sill plates shall be SYP #3 or better. Pressure treat all sill plates in contact with concrete or masonry.
All pressure treated 2x material shall be SYP #2 or better and shall be treated in accordance with AWWA Standard U1 to the requirements of Use Category 3B (UC3B) or Use Category 4A (UC4A).
All pressure treated Parallam shall be Truss Joist MacMillan, Womanized Parallam PSL, or approved equal.
All pressure treated Glulam (GL) members shall be Rostoro Treated X-Beam 2400F_v-1.9C or approved equal.
All Glulams shall be Rostoro 24F-V4 or better.
All Blocking, Headers, beams and miscellaneous framing shall be SYP#2 U.N.O.
- 12.2 Roof Deck/Diaphragm
 - Roof sheathing shall be 5/8" Exposure 1 plywood
 - Place long direction perpendicular to framing
 - Stagger end joints
 - Provide roof sheathing clips, Simpson PSCL/PSCA or approved equal at all unsupported edges.
 - Nail to supporting members with 10d ring shank @ 6"o.c. edges and 12"o.c. field.
- 12.3 All strap and tie connections shall have z-max (g185) triple zinc coating (or hot-dipped galvanized). All nails in contact with pressure treated lumber shall be hot-dipped galvanized and approved for use in pressure treated lumber.
- 12.4 Do not bend coil straps.
- 12.5 Unless noted otherwise, connect all building components per table 2304.10.1 - fastening schedule, per IBC 2015.
- 12.0 SHOP DRAWING SUBMITTAL:
- 12.1 See Project Manual
- 12.2 Contractor shall submit Electronic copies (PDF format) of each shop drawing for review. Shop drawings shall be reviewed by the Contractor prior to submission to the Engineer. The Contractor shall allow 10 working days for shop drawing approval.
- 13.0 CONSTRUCTION AND SAFETY:
- 13.1 Woods Engineering P.A.'s responsibility is limited to the details and information shown on these drawings. It is the responsibility of the Contractor to provide adequate safety measures required by local codes as well as OSHA Standards for the Construction Industry. This should include, but not be limited to the following:
Shoring to protect new as well as existing structures.
Necessary Scaffolding.
Material Handling Equipment.
Trench Boxing.
- 14.0 SPECIAL INSPECTIONS:
- 14.1 Refer to Sheet S11 sheets and for all Special Inspections requirements.

ABBREVIATIONS

@	AT	HT	HIP TRUSS
&	AND	IFM	INSIDE FACE OF MASONRY
AB	ANCHOR BOLTS	INT	INTERIOR
ACI	AMERICAN CONCRETE INSTITUTE	JBE	JOIST BEARING ELEVATION
ADDL	ADDITIONAL	JT	JOINT
AFF	ABOVE FINISHED FLOOR	K	KIP-S
AISC	AMERICAN INSTITUTE OF STEEL	KB	KICKER BRACE
	CONSTRUCTION	KSI	KIPS PER SQUARE INCH
ASI	AMERICAN IRON AND STEEL	(L)	LONG SIDE REINFORCEMENT
	INSTITUTE	LB	LONG BAR
ALT	ALTERNATE	LBS	POUNDS
ARCH	ARCHITECTS - ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
ASTM	AMERICAN SOCIETY FOR	LLV	LONG LEG VERTICAL
	TESTING AND MATERIALS	LO	LOW
AWS	AMERICAN WELDING SOCIETY	LOC	LOCATION
B, BOT	BOTTOM	LWC	LIGHT WEIGHT CONCRETE
BCX	BOTTOM CHORD EXTENSION	MAX	MAXIMUM
BFF	BELOW FINISHED FLOOR	MC	MOMENT CONNECTION
BLDG	BUILDING	MECH	MECHANICAL
BM	BEAM	MFR	MANUFACTURER
BOS	BOTTOM OF STEEL	MID	MIDDLE
BRG	BEARING	MIN	MINIMUM
BTWN	BETWEEN	MISC	MISCELLANEOUS
CFS	COLD FORMED STEEL	NOW	MIDDLE OF WALL
CJ	CONTRACTION JOINT	NP	MASONRY PLASTER
CL	CENTERLINE	d	NAILS - PENNY
CLR	CLEAR	No	NUMBER
CMU	CONCRETE MASONRY UNITS	NS	NEAR SIDE
COL	COLUMN	NTS	NOT TO SCALE
CONC	CONCRETE	NWC	NORMAL WEIGHT CONCRETE
CONN	CONNECTION	OC	ON CENTER
CONST JT	CONSTRUCTION JOINT	OFB	OUTSIDE FACE OF BRICK
CONT	CONTINUOUS	OFM	OUTSIDE FACE OF MASONRY
CONTR	CONTRACTOR	OFS	OUTSIDE FACE OF STUD
CSJ	COMPOSITE STEEL JOIST	OPNG	OPENING
CTRD	CENTERED	OPP	OPPOSITE HAND
DBA	DEFORMED BAR ANCHOR	PEBS	PRE-ENGINEERED BUILDING
DD	DELEGATED DESIGN		SUPPLIER
DEFL	DEFLECTION	PED	PEDESTAL
DEPR	DEPRESSION - DEPRESSED	PL	PLATE
DET	DETAIL	PSF	POUNDS PER SQUARE FOOT
DIAG	DIAGONAL	PSI	POUNDS PER SQUARE INCH
D	DIAMETER	PFL	PARALLEL STRAND LUMBER
DIM	DIMENSION	PLF	POUNDS PER LINEAR FOOT
DIST	DISTANCE	PT	PRESSURE TREATED
DWG(S)	DRAWING(S)	REF	REFERENCE
DWL(S)	DOWEL(S)	REINF	REINFORCING
EA	EACH	REQD	REQUIRED
ELEV	ELEVATION	(S)	SHORT SIDE REINFORCEMENT
EMBED	EMBEDDED - EMBEDMENT	SB	SHORT BAR
ENG	ENGINEER	SCHD	SCHEDULE
EOR	ENGINEER OF RECORD	SF	STEP FOOTING
EQ	EQUAL	SIM	SIMILAR
EQUIP	EQUIPMENT	SOG	SLAB ON GRADE
EF	EACH FACE	SPEC(S)	SPECIFICATIONS(S)
EJ	EXPANSION JOINT	SPF	SPRUCE PINE FUR
EOD	EDGE OF DECK	SQ	SQUARE
EQM	EDGE OF MASONRY	STD	STANDARD
EOS	EDGE OF SLAB	STIFF	STIFFENER
EW	EDGE OF WALL	STIRR	STIRRUP
EW	EACH WAY	STL	STEEL
EXIST	EXISTING	STR	STRUCTURAL
EXP	EXPANSION	SW	SHEAR WALL
EXT	EXTERIOR	SYP	SOUTHERN YELLOW PINE
FDN	FOUNDATION	T	TOP
PFE	FINISHED FLOOR ELEVATION	TCX	TOP CHORD EXTENSION
FS	FAR SIDE	TOC	TOP OF CONCRETE
FTG	FOOTING	TOS	TOP OF STEEL
GA	GAUGE	TOW	TOP OF WALL
GALV	GALVANIZED	TYP	TYPICAL
GT	GIRDER TRUSS	UNO	UNLESS NOTED OTHERWISE
HD	HEADED	VB	VEHICLE BARRIER
HI	HIGH	VERT	VERTICAL
HORIZ	HORIZONTAL	VIF	VERIFY IN FIELD
HSS	HOLLOW STRUCTURAL SECTION	W	WITH
		WWF	WELDED WIRE FABRIC

DO NOT SCALE DIGITAL OR HARD COPIES OF THESE DRAWINGS:

Unless Specifically Noted - Drawings, Plans, Sections, Details, Etc. are a graphic representation of the framing conditions and/or requirements.

Rebar lengths, bands & etc. SHALL NOT be determined by scaling any drawings included in this set of documents. Lengths & sizes shall be determined by the schedules only or specifically requested if not numerically shown. Submit a written request to Woods Engineering, PA if further clarification is needed.

WE

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PROJECT TITLE

OCEANFRONT
AMENITY
PAVILION

LOTS 7-10, BLOCK 1
CAROLINA BEACH AVE N
CAROLINA BEACH, NC 28428

PRELIMINARY
DESIGN CONCEPT ONLY. SUBJECT
TO FURTHER REVISIONS. NOT FOR
CONSTRUCTION NOT FOR
PERMITS

ISSUED: 07/24/26

SHEET TITLE

GENERAL NOTES

ISSUE BLOCK

Mark	Date	Description
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PROJECT NO: 2023042.02

DATE: 07.24.26

SCALE: AS NOTED

DRAWN BY: JL PROJ MGR: AS

S102

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SHEET TITLE

**SPECIAL
INSPECTIONS**

ISSUE BLOCK

Mark Date Description

PROJECT NO: 2023042.02

DATE: 07.24.26

SCALE: AS NOTED

DRAWN BY: JL PROJ MGR: AS

S111

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Statement of Special Inspections

Project: Springhill Suites & Amenity Pavilion
Location: Carolina Beach, North Carolina
Owner's Representative: Page Johnson
Owner's Address: page.johnson@harmonycompanies.com

This Statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Special Inspection requirements of the 2018 North Carolina State Building Code. It includes a Schedule of Special Inspection Services applicable to this project, the name of the Special Inspector, the identity of other approved agencies retained for conducting Special Inspections, and the required inspector qualifications. This Statement of Special Inspections was prepared by the following Designers of Record:

Structural	Adam Sisk	6/25/2026
(Type or print name) (Signature) (Date)		
Architectural		
(Type or print name) (Signature) (Date)		
Mechanical		
(Type or print name) (Signature) (Date)		
Other		
(Type or print name) (Signature) (Date)		

The Special Inspector shall keep records of all special inspections and tests and shall furnish reports to the State Construction Office and the Designers of Record. Reports shall indicate if the work inspected or tested was or was not completed in conformance with the approved construction documents. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the State Construction Office and the Designers of Record. The Special Inspections program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the State Construction Office, Owner, and the Designers of Record.

Interim Report Frequency: Monthly

A Final Report of Special Inspections documenting completion of all required Special Inspections, testing, and correction of any discrepancies should be submitted prior to issuance of a Certificate of Use and Occupancy.

Job Site safety and means and methods of construction are solely the responsibility of the Contractor.

Owner's Authorization

Accepted by:

Signature Date

Signature Date

Schedule of Special Inspection Services,

The following sheets comprise the required schedule of special inspections for this project. The construction divisions which require special inspections for this project are as follows.

- ☐ Structural Steel & High-Strength Bolting
☐ Welding of Structural Steel
☐ Cold-Formed Steel Deck
☐ Open-Web Steel Joists & Jost Girders
☐ Cold-Formed Steel Framing
☐ Concrete Construction
☐ Masonry Construction
☐ Wood Construction
☐ Soils
☐ Driven Deep Foundations
☐ Cast-in-Place Deep Foundations
- ☐ Helical Pile Foundations
☐ Rammed Aggregate Piers & Stone Columns
☐ Sprayed Fire-Resistant Material
☐ Mastix & Intumescent Fire-Resistant Coatings
☐ Exterior Insulation & Finish System
☐ Fire-Resistant Penetrations & Joints
☐ Smoke Control
☐ Retaining Wall & Systems > 5 Feet
☐ Special Inspections for Wind Resistance
☐ Special Inspections for Seismic Resistance

- a. The inspection frequency indicated on the following inspection tables are "C" continuous, "I" periodic, & "O" random on a daily basis.
b. Level A is the minimum inspection program for empirically / prescriptively designed masonry in Risk Category I, II or III structures.
Level B is the minimum inspection program for empirically / prescriptively designed masonry in Risk Category IV structures and engineered masonry in Risk Category I, II or III structures. Level C is the minimum inspection program for engineered masonry in Risk Category IV structures. Engineered masonry structures are those designed in accordance with portions of the TMS 402-13 / ACI 308-13/ASCE 5-13 other than Part 4 or Appendix A.

Inspection Agents	Firm Name & Point of Contact	Address / Phone / E-mail
1. Special Inspector (SI-1)		
2. Testing Agency (TA-1)		
3. Testing Agency (TA-2)		
4. Geotechnical Engineer (GE-1)		
5. Other (O-1)		

Note: The inspection and testing agent(s) shall be engaged by the Owner or the Registered Design Professional of Record acting as the Owner's agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the State Construction Office, prior to commencing work.

Seismic Design Category: ☐ A ☒ B ☐ C ☐ D ☐ N/A

Basic Wind Speed (Vasd): ☐ 90-109mph ☒ 110-119mph ☐ ≥120mph N/A

Wind Exposure Category: ☐ B ☐ C ☒ D ☐ N/A

**Schedule of Special Inspection Services
Soils**

Inspection Task	Task Req'd	Freq	Reference for Criteria Standard	NCBC	Agents
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity	☒	P		1705.6	
2. Verify excavations extend to proper depth and have reached the correct soil material	☒	P		1705.6	
3. Perform classification and testing of compacted fill materials	☒	P		1705.6	
4. Verify that materials used, densities, lift thickness and procedures used during placement and compaction of compacted fill are in accordance with the approved soils report and the construction documents	☒	C		1705.6	
5. Prior to placement of compacted fill, verify that the subgrade has been prepared in accordance with the approved soils report and the construction documents	☒	P		1705.6	

**Schedule of Special Inspection Services
Concrete Construction**

Inspection Task	Task Req'd	Freq	Reference for Criteria Standard	NCBC	Agent
1. Inspect reinforcement, including prestressing tendons, and verify placement	☒	P	ACI Ch.20, 25.2, 25.3, 26.6.1-26.6.3	1908.4	
2. Reinforcing Bar Welding:					
e. Verify weldability of reinforcing bars other than ASTM A706 and collect reports	☒	P	AWS D1.4	1704.5	
f. Inspect single-pass fillet welds ≤ 5/16"	☒	P	ACI 26.6.4		
g. Inspect all welds other than single-pass fillet welds ≤ 5/16"	☒	C	ACI 26.6.4		
3. Concrete Anchors:					
a. Inspect anchors cast in concrete	☒	P	ACI 17.8.2		
b. Inspect adhesive anchors installed in hardened concrete with horizontally or upwardly inclined orientations that resist sustained tension loads	☒	C	ACI 17.8.2.4		
c. Inspect adhesive anchors installed in hardened concrete with orientations different from Item 3.b	☒	P	ACI 17.8.2		
d. Inspect mechanical anchors installed in hardened concrete	☒	P	ACI 17.8.2		
4. Collect mix designs and verify the correct mix used during installation	☒	P	ACI Ch.19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3	
5. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete	☐	C	ASTM C172, ASTM C31, ACI 26.4, 26.12	1908.10	
6. Inspect concrete and shotcrete placement for proper application techniques	☒	C	ACI 26.5	1908.6, 1908.7, 1908.8	
7. Collect reports of preconstruction tests for shotcrete when preconstruction tests are required by NCBC Section 1908.4	☐	C		1704.5, 1908.5	
8. Verify maintenance of specified curing temperature and techniques	☒	P	ACI 26.5.3-26.5.5	1908.9	
9. Inspections for prestressed concrete					
a. Observe application of prestressing force	☐	C	ACI 26.10		
b. Inspect grouting of bonded prestressing tendons	☐	C	ACI 26.10		
10. Verify concrete strength prior to stressing of PT tendons and prior to removal of shores and forms from PT & mild beams and structural slabs	☐	P	ACI 26.11.2		
11. Inspect erection of precast members	☐	P	ACI 26.8		
12. Inspect formwork for shape, location and dimensions of the concrete member being formed	☒	P	ACI 26.11.1,2(b)		
13. Collect mill test reports for ASTM A615 rebar used by SFRS special moment frames, special structural walls or coupling beams	☐	C	ACI 20.2.2.5	1704.5	

a. References to "ACI" in this table are to the ACI 318-14.

**Schedule of Special Inspection Services
Masonry – Level B**

Inspection Task	Task Req'd	Freq	Reference for Criteria TMS 402a TMS 602a	Agent
1. Test & verify slump flow & visual stability index as delivered to site for self-consolidating grout	☒	C	Table (Tbl) 3.1.2 Art. 1.5B.1.b.3	
2. Test & verify f'm & AAC prior to construction	☒	C	Tbl 3.1.2 Art. 1.4B	
3. Verify compliance with the approved submittals	☒	P	Tbl 3.1.2 Art. 1.5	
4. As masonry construction begins, verify that the following are in compliance:				
a. Proportions of site-prepared mortar	☒	P	Art. 2.1, 2.6A	
b. Construction of mortar joints	☒	P	Art. 3.3B	
c. Grade and size of prestressing tendons and anchorages	☒	P	Art. 2.4B, 2.4H	
d. Location of reinforcement, connectors and prestressing tendons and anchorages	☒	P	Art. 3.4, 3.6A	
e. Prestressing technique	☒	P	Art. 3.6B	
f. Properties of thin-bed mortar at AAC masonry	☒	C / Pa	Art. 2.1C	
5. Prior to grouting, verify that the following comply:				
a. Grout space is clean, and cleanouts provided when required	☒	P	Art. 3.2D, 3.2F	
b. Grade, type & size of reinforcement & anchor bolts, & prestressing tendons & anchorage	☒	P	Sec. 6.1 Art. 2.4, 3.4	
c. Placement of reinforcement, connectors, and prestressing tendons and anchorage	☒	P	Sec. 6.1, 6.2.1, 6.2.6, 6.2.7	Art. 3.2E, 3.4, 3.6A
d. Proportions of site-prepared grout and prestressing grout for bonded tendons	☒	P	Art. 2.6B, 2.4G.1.b	
e. Construction and size of mortar joints	☒	P	Art. 3.3B	
6. Verify during construction:				
a. Size and location of structural elements	☒	P	Art. 3.3F	
b. Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction	☒	P	Sec. 12.1(e), 6.1.4.3, 6.2.1	
c. Welding of reinforcement	☐	C	Sec. 8.1.6.7.2, 9.3.3.4(c), 11.3.3.4(b)	
d. Preparation, construction, and protection of masonry during cold weather (temperature < 40°F) or hot weather (temperature > 90°F)	☒	P	Art. 1.8C, 1.8D	
e. Application & measurement of prestress force	☒	C	Art. 3.6B	
f. Verify placement of grout and prestressing grout for bonded tendons	☒	C	Art. 3.5, 3.6C	
g. Placement of AAC masonry units and construction of thin-bed mortar joints	☒	C / Pb	Art. 3.3B.9, 3.3F.1.b	
7. Observe preparation of grout specimens, mortar specimens, and/or prisms	☒	P	Art. 1.4.B.2.a.3, 1.4.B.2.b.3, 1.4.B.2.c.3, 1.4.B.3, 1.4.B.4	

a. References to "TMS402" in this table are to the TMS402/ACI308/ASCE5-13. References to "TMS602" are to TMS602/ACI308.1/ASCE6-13.
b. AAC masonry shall be continuously inspected for the first 5000-square feet and periodically inspected afterwards.

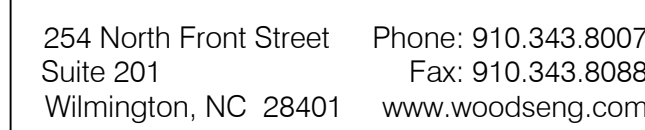
**Schedule of Special Inspection Services
Driven Deep Foundations**

Inspection Task	Task Req'd	Freq	Reference for Criteria Standard	NCBC	Agents
1. Verify that deep foundation materials, sizes and lengths comply with the construction documents	☒	C		1705.7	
2. Observe pile load tests and determine capacities of test elements ensuring compliance with the construction documents	☒	C		1705.7	
3. Inspect driving operations and maintain complete and accurate records for each element	☒	C		1705.7	
4. Verify placement, location, plumbness, hammer size and type, blow count per foot of penetration, required penetrations to achieve design capacity, tip and butt elevations, damage and anomalies	☒	C		1705.7	

a. For steel elements, perform additional inspections in accordance with Section 1705.5 of the North Carolina Building Code and the applicable Schedules included herein.
b. For concrete elements and concrete-filled elements, perform additional inspections in accordance with Section 1705.3 of the North Carolina Building Code and the applicable Schedules included herein.
c. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge and the applicable Schedules included herein.

**Schedule of Special Inspection Services
Structural Steel and High-Strength Bolting**

Inspection Task	Task Req'd	Freq	Reference for Criteria AISC 360	NCBC	Agent
1. Fabricator Certification / Verification of Quality Control Procedures					
a. Verify fabricator qualifications	☒	C		1704.2.5.1	
b. Review material test reports & certifications	☒	C	N5.2		
c. Collect certificates of compliance from the steel fabricator at completion of fabrication	☒	C		1704.5	
2. Inspections Prior to High-Strength Bolting at Pretensioned and Slip-Critical Joints					
a. Collect manufacturer's certifications for fastener materials	☒	C	Table (Tbl) N5.6-1		
b. Fasteners are marked per ASTM requirements	☒	P	Tbl N5.6-1		
c. Ensure correct fasteners and bolting procedures are selected for joint details	☒	P	Tbl N5.6-1		
d. Verify connecting elements, including the appropriate laying surface condition and hole preparation when specified, comply with the construction documents	☒	P	Tbl N5.6-1		
e. Observe and document pre-installation verification testing by installation personnel for fastener assemblies and methods	☒	P	Tbl N5.6-1		
f. Verify proper storage provided for all fastener components	☒	P	Tbl N5.6-1		
3. Inspections During High-Strength Bolting at Pretensioned and Slip-Critical Joints					
a. Ensure correct fastener assemblies placed in all holes and washers, when specified, are positioned as required	☒	P	Tbl N5.6-2		
b. Verify joint brought to snug-tight condition prior to pretensioning	☒	P	Tbl N5.6-2		
c. Verify fastener components not turned by the wrench prevented from rotating	☒	P	Tbl N5.6-2		
d. Ensure fasteners are pretensioned in accordance with RSC, progressing from the most rigid point towards free edges	☒	P	Tbl N5.6-2		
4. Document acceptance or rejection of bolted connections after high-strength bolting is complete	☒	C	Tbl N5.6-3		
5. Structural Details					
a. Verify diameter, grade, type and length of anchor rods and other embedded items supporting structural steel	☒	P	N5.7		
b. Inspection of fabricated assemblies & erected steel framing verifying compliance with the construction documents	☒	P	N5.7		
6. Composite Construction					
a. Verify placement & installation of steel deck	☐	P	Tbl N6.1		
b. Observe placement and installation of steel headed stud anchors	☐	P	Tbl N6.1		
c. Document acceptance or rejection of composite construction elements	☐	P	Tbl N6.1		



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FINAL REPORT OF SPECIAL INSPECTIONS

Project: Springhill Suites & Amenity Pavilion

Location: Carolina Beach, North Carolina

Owner: Page Johnson

Owner's Address: page.johnson@harmonycompanies.com

Architect of Record: ????????

Structural Engineer of Record:

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the State of Special Inspections submitted for permit, have been performed and all discovered discrepancies have been reported and resolved other than the following:

Comments:

(Attach continuation sheets if required to complete the description of corrections).

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,
Special Inspector

Licensed Professional Seal

Signature _____ Date _____



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ISSUED: 07/24/26

SHEET TITLE

SPECIAL
INSPECTIONS

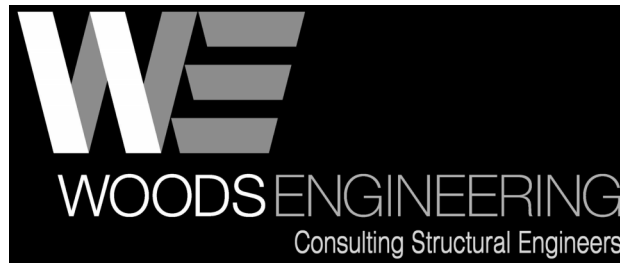
ISSUE BLOCK		
Mark	Date	Description

PROJECT NO:	2023042.02
DATE:	07.24.26
SCALE:	AS NOTED
DRAWN BY: JL	PROJ MGR: AS

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LEGEND - FLOOR PLAN

- INDICATES 8" CMU WALL. SEE S5 SHEETS FOR VERTICAL REINFORCING AND GROUTING REQUIREMENT. PROVIDE W1.7 HORIZONTAL JOINT REINFORCEMENT @ 16" o.c. INDICATES 8" CMU NON LOAD BEARING PARTITION WALLS W/ #5 @ 48" o.c. U.N.O. WITH W1.7 HORIZONTAL JOINT REINFORCEMENT @ 16" o.c. GROUT REINFORCED CELLS. SEE S40 SHEETS FOR BRACING REQUIREMENTS.
- INDICATES 8" LOAD BEARING CAST-IN-PLACE CONCRETE WALLS WITH #5 @ 12" o.c. VERTICAL AND #4 @ 12" o.c. HORIZONTAL.
- INDICATES 8" CONCRETE SHEAR WALL. SEE S5.0 SHEETS FOR REINFORCING DETAILS.
- INDICATES CMU WALLS ABOVE.
- INDICATES CONCRETE COLUMN (BELOW), SEE FOUNDATION PLAN.
- INDICATES WOOD POST (ABOVE), SEE PLAN FOR SIZE AND LOCATION.
- INDICATES BUILDING EXPANSION JOINT LOCATION, SEE SECTIONS FOR DETAILS.
- CONCRETE BEAM, WIDTH x DEPTH. SEE REINFORCING PLAN.
- CONCRETE TIE BEAM - SEE S3 SHEETS FOR SCHEDULE AND ADDITIONAL REINFORCING STEEL.
- HEADER, SEE SCHEDULE ON S4 SHEETS.
- MASONRY HEADER, SEE SCHEDULE S4 SHEETS.
- INDICATE SLAB STEP.
- INDICATES 12" THICK PT. SLAB, SEE PLAN.
- GRID DESIGNATION FOR CENTERLINE OF COLUMN U.N.O.

NOTES - FLOOR FRAMING PLAN

- SEE S1 SHEETS FOR ADDITIONAL GENERAL NOTES, MATERIAL NOTES AND MATERIAL SPECIFICATIONS. ALSO, SEE S1 SHEETS FOR TYPICAL DETAILS. TYPICAL DETAILS ARE GENERALLY NOT SHOWN ON PLAN BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS.
- TOP OF TYPICAL SLAB ELEVATION PER SCHEDULE SHEET S2.00, OTHER ELEVATIONS ARE NOTED AS (+ OR -) FROM DATUM ELEVATION.
- HEADERS, BEAMS & LOAD BEARING WALLS, AND SHEAR WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL. UNIT LABELS ARE FROM LEVEL BELOW AND ARE FOR COORDINATION ONLY - CONFIRM UNIT LOCATIONS WITH ARCH.
- WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O.
- STAIRS SHALL BE CONCRETE FILLED METAL PAN. SEE ARCH FOR ADDITIONAL DETAILS.

SECOND FLOOR FRAMING PLAN

SCALE: 1/8" = 1'-0"

PROJECT TITLE

OCEANFRONT AMENITY PAVILION

LOTS 7-10, BLOCK 1
CAROLINA BEACH AVE N
CAROLINA BEACH, NC 28428

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SHEET TITLE

SECOND FLOOR FRAMING PLAN

ISSUE BLOCK

Mark	Date	Description

PROJECT NO: Project Number

DATE: 07.24.26

SCALE: 1/8" = 1'-0"

DRAWN BY: JL PROJ MGR: AS

S202

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TENSION SLAB PLAN

- 2. SEE SHEET S1 SHEETS FOR ADDITIONAL GENERAL NOTES, FOUNDATION NOTES, CONCRETE NOTES, AND REINFORCING STEEL NOTES AND TYPICAL DETAILS. TYPICAL DETAILS ARE GENERALLY NOT SHOWN ON PLAN BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS.
- 3. TOP OF TYPICAL SLAB ELEVATION PER SCHEDULE. OTHER ELEVATIONS ARE NOTED AS (+ OR -) FROM DATUM ELEVATION.
- 3. STRUCTURAL FLOOR SYSTEM IS A THICK TWO-WAY CONCRETE SLAB PLATE (BANDS) AND UNIFORM TENDON LAYOUT). SEE PLAN FOR THICKNESS TYPICAL, L.I.N.O. SEE S7 SHEETS FOR TYPICAL POST-TENSIONING DETAILS. ANY REINFORCEMENT SHOWN ON S7 DETAILS IS IN ADDITION TO THE REINFORCING SHOWN ON REBAR PLANS.
- 4. SEE TABLE ON S3 SHEETS FOR CONCRETE STRENGTH REQUIREMENTS
- 5. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DISCIPLINE DRAWINGS FOR OPENINGS AND DEPRESSIONS NOT SHOWN ON THESE DRAWINGS. SEE S7 FOR ALLOWABLE SLEEVE LOCATIONS.
- 6. ALL POST INSTALLED ANCHORAGE TO HAVE A MAXIMUM EMBED OF 3/4" TO ENSURE TENDONS ARE NOT DAMAGED DURING INSTALLATION.
- 7. SR# INDICATES STUD RAIL TYPE. SEE THIS SHEET FOR DETAILS.

NOTES - FLOOR PLAN

MILD REINFORCING NOTES:

- a. ALL BOTTOM REINFORCING BARS ARE CENTERED WITHIN THE SLAB SPAN, UNO. DISTRIBUTE BOTTOM BARS UNIFORMLY THROUGHOUT THE EXTENT SHOWN ON PLAN, WHEN NO EXTENT IS SHOWN, BOTTOM BARS ARE TO BE PLACED BETWEEN THE COLUMN LINES.
 b. TOP REINFORCING BARS ARE CENTERED OVER THE COLUMNS, UNO. A MINIMUM OF 4#5 SHALL BE PLACED OVER EACH COLUMN IN EACH ORTHOGONAL DIRECTION, UNO ON PLAN, SEE S7 SCHEDULES FOR TYPICAL TOP REBAR PLACEMENT DETAILS.
 c. ALL REBAR SPICINGS SHALL BE FULL TENSION LAP SPICE "36", ALL REBAR ENDS @ SLAB EDGE TO BE PROVIDED W/ STANDARD HOOKS.
 d. TOP & BOTTOM REBAR SHOWN ON PLAN ARE IN ADDITION TO THE REINFORCING SHOWN ON TYPICAL DETAILS.
 e. MINIMUM REBAR COVER FOR THIS FLOOR SLAB:
 BOTTOM BARS = 0.75" UNO.
 TOP BARS = 0.75" UNO.
 PT = 0.75" UNO.

 = (2) #5 TOP AND BOTTOM TYP. WHERE INDICATED ON PLAN

POST-TENSION REINFORCING NOTES:

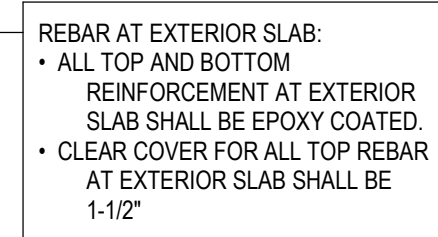
- a. ALL CABLES, ANCHORAGES AND COUPLERS SHALL BE ENCAPSULATED - SEE SPECIFICATIONS.
- b. IF ELEVATIONS OF BANDED & UNIFORM TENDONS CONFLICT, THEN PLACE UNIFORM TENDONS BELOW BANDED.
- c. MINIMUM (2) PT CABLES TO PASS DIRECTLY OVER EACH COLUMN IN AN ORTHOGONAL DIRECTION.
- d. HIGH CGS POINTS OF TENDON PROFILES ARE AT CENTERLINE OF SUPPORTS. LOW CGS POINTS OF CABLES SPAN UNLESS SPECIFICALLY NOTED OTHERWISE ON PLAN.
- e. FOR SINGLE END STRESSING OF BANDED & UNIFORM TENDONS - ALTER STRESSING ENDS WHERE POSSIBLE.
- f. ANCHORS AT SLAB EDGE ARE PLACED AT MID-DEPTH OF SLAB RUNNED UP TO THE CLOSES 1/4" UNLESS NOTED OTHERWISE ON PLAN.

TENDON PROFILE SYMBOL LEGEND:

- | | |
|---|--|
|  | LOW POINT = 1" ABOVE BOTTOM OF SLAB / BOTTOM OF BEAM, U.N.O.
X = DISTANCE ABOVE BOTTOM OF SLAB / BEAM |
|  | HIGH POINT = 1" BELOW TOP OF SLAB / TOP OF BEAM, U.N.O.
X = DISTANCE BELOW TOP OF SLAB / BEAM |
|  | DEAD END = AT CENTROID OF SLAB, U.N.O. |
|  | STRESSING END = AT CENTROID OF SLAB, U.N.O. |
|  | CONTINUATION OF TENDON NOT SHOWN -
MATCH FORCE AND PROFILE OF ADJACENT TENDONS |

LEGEND - FLOOR PLAN

- | | |
|--|---|
| | INDICATES CONC. SHEAR WALL, SEE SHEETS FOR REINFORCING DETAILS |
| | INDICATES CONCRETE COLUMN, SEE SCHEDULE ON S6 SHEETS |
| | GRID DESIGNATION FOR CENTERLINE OF COLUMN |
| | INDICATES CONCRETE BEAM SEE S8 SHEETS FOR REQUIREMENTS |
| | INDICATES REINFORCEMENT - (T) INDICATES TOP BARS |
| | (B) INDICATES BOTT. BARS |
| | INDICATES STANDARD 90deg. HOOK |
| | INDICATES #5@60" LONG WITH 90deg HOOK @ 120" = TYP. ALL 60" EDGES BETWEEN TOP COLUMN BARS |
| | INDICATES BUILDING EXPANSION JOINT LOCATION, SEE SECTIONS FOR DETAILS |



SECOND FLOOR PT PLAN

SCALE: 1/8" = 1'-0"

SLAB THICKNESS : 12" THICK PT SLAB
REBAR QUANTITY : 1.05 LBS/SQFT
TENDON QUANTITY : 1.35 LBS/SQFT

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SHEET TITLE

SECOND FLOOR PT
PLAN

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PROJECT NO.	10-1-111
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PROJECT NO:	Project Num
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PROJECT NO:	Project Name
DATE:	07.24

DATE:	07.24
SCALE:	1/8" = 1'

SCALE:	170 = 1
DRAWN BY: Author	PROJ: MChen

DRAWN BY Author	PROJ Mchec
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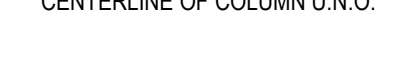
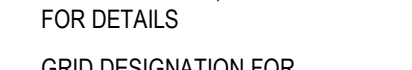
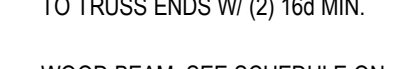
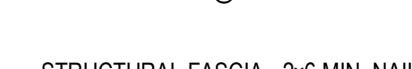
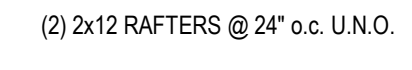
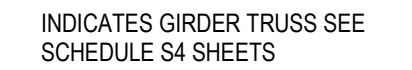
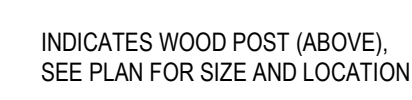
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BRIDGE FRAMING LEGEND



ROOF FRAMING NOTES

1. FOR GENERAL FRAMING INFORMATION AND MATERIAL SPECIFICATIONS, SEE S1 SHEETS.
2. HEADERS, BEAMS, SHEAR WALLS AND LOAD BEARING WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL.
3. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O.
4. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DISCIPLINE DRAWINGS FOR OPENINGS, SLOPES, AND EQUIPMENT NOT SHOWN.



PROJECT TITLE

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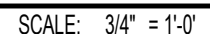
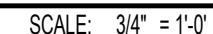
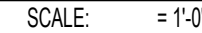
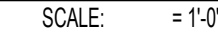
PEDESTRIAN BRIDGE
FRAMING PLANS

ISSUE BLOCK		
Mark	Date	Description

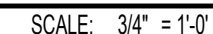
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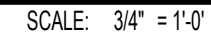
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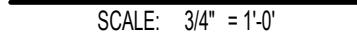
NOTE:
COORDINATE CONTROL JOINT
LOCATION W/ARCHITECTURAL
MAXIMUM SPACING OF 25' OR 3
TIMES THE WALL HEIGHT



3/4" PROVIDE BACKER ROD & CAULK FILL PER ARCH. ONLY @ EXTERIOR BUILDING WALLS



3/4" PROVIDE BACKER ROD & CAULK FILL PER ARCH. ONLY @ EXTERIOR BUILDING WALLS



3/4" PROVIDE BACKER ROD & CAULK FILL PER ARCH. ONLY @ EXTERIOR BUILDING WALLS

NOTES:
-ALL SLAB-ON-GRADES SHALL BEAR ON DRAINAGE LAYER AND WELL COMPACTED SUBGRADE PER GEOTECHNICAL REPORT. VERIFY COMPACTION AND MATERIAL WITH GEOTECHNICAL ENGINEER.

GENERAL NOTE:
DETAILS SHOWN ON THIS SHEET ARE GENERIC IN NATURE AND MAY NOT PORTRAY EXACT CONDITIONS. THESE DETAILS ARE INTENDED TO PROVIDE GENERAL GUIDELINES FOR TYPICAL CONSTRUCTION CONDITIONS.

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FOUNDATION DETAILS AND SCHEDULES

ISSUE BLOCK

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DATE:	07.24.

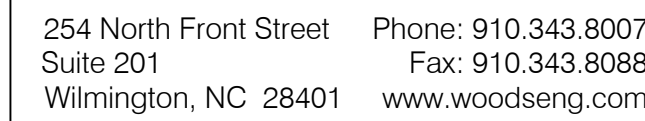
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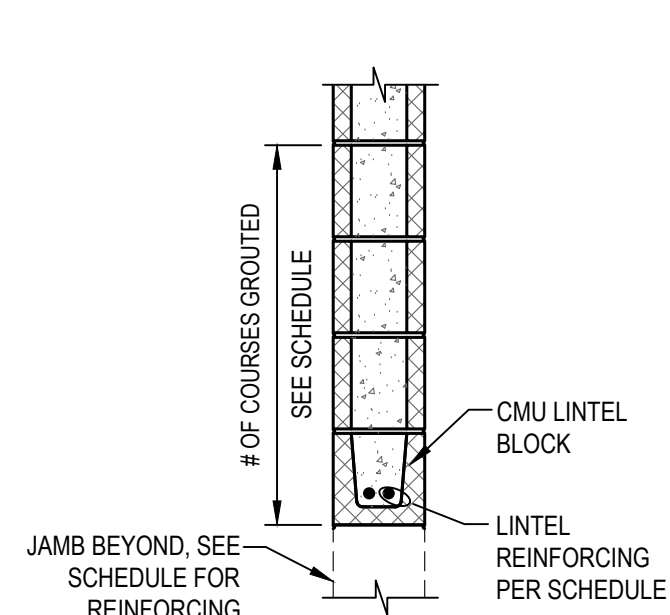
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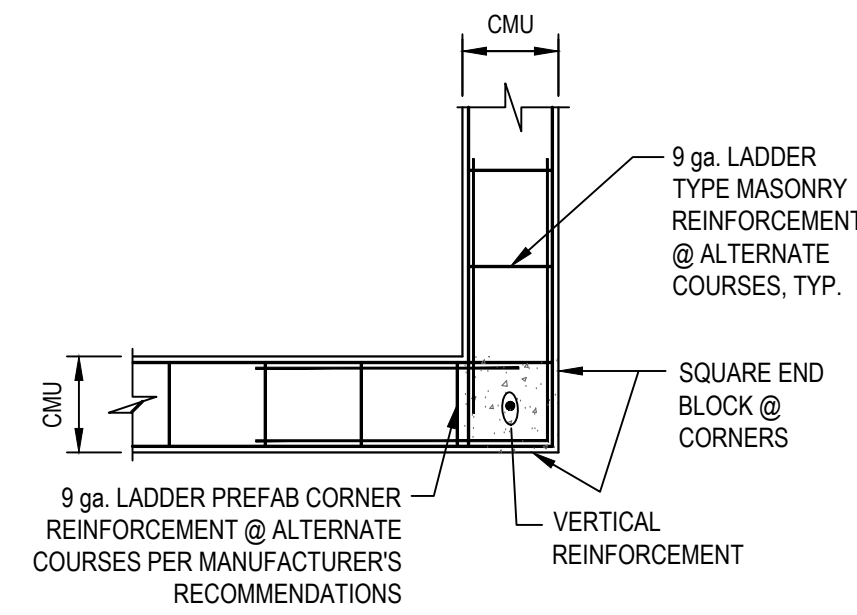
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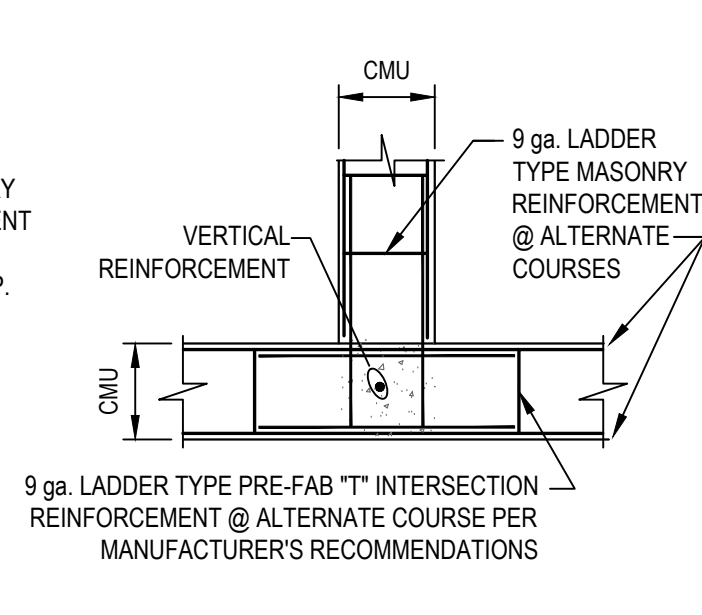
CMU HEADER DETAIL
DETAIL "A"
SCALE: 3/4" = 1'-0"



TYPICAL CMU CORNER

DETAIL "B"

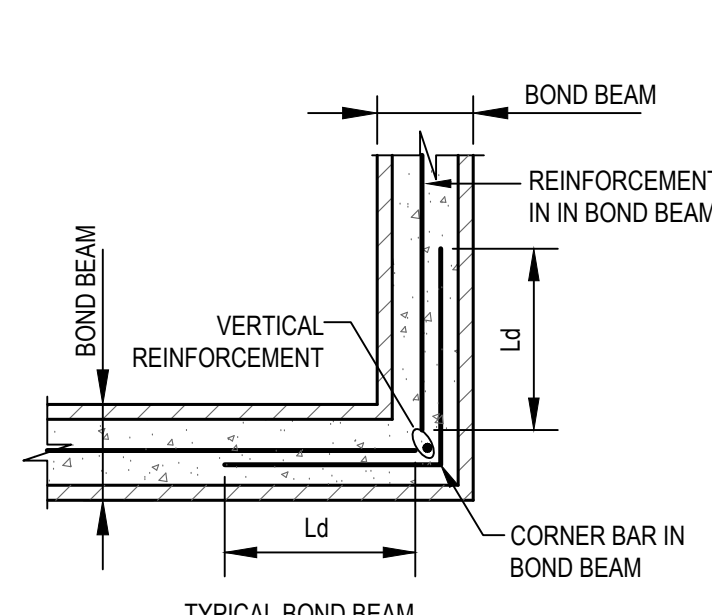
SCALE: 3/4" = 1'-0"



TYPICAL CMU INTERSECTION

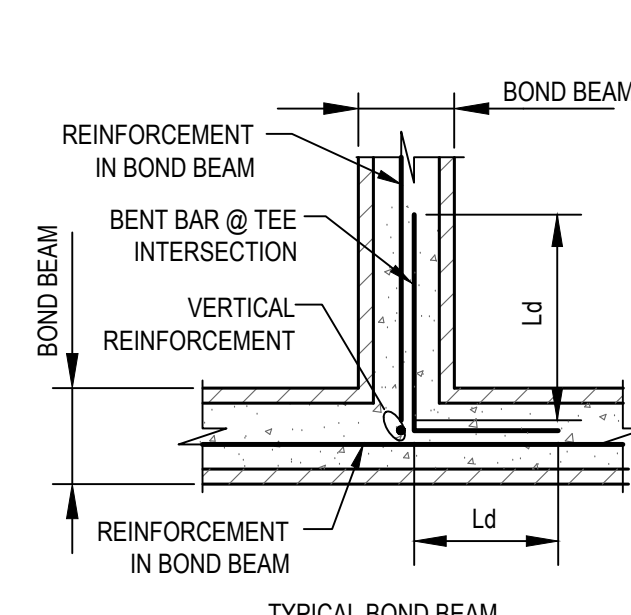
DETAIL "C"

SCALE: 3/4" = 1'-0"



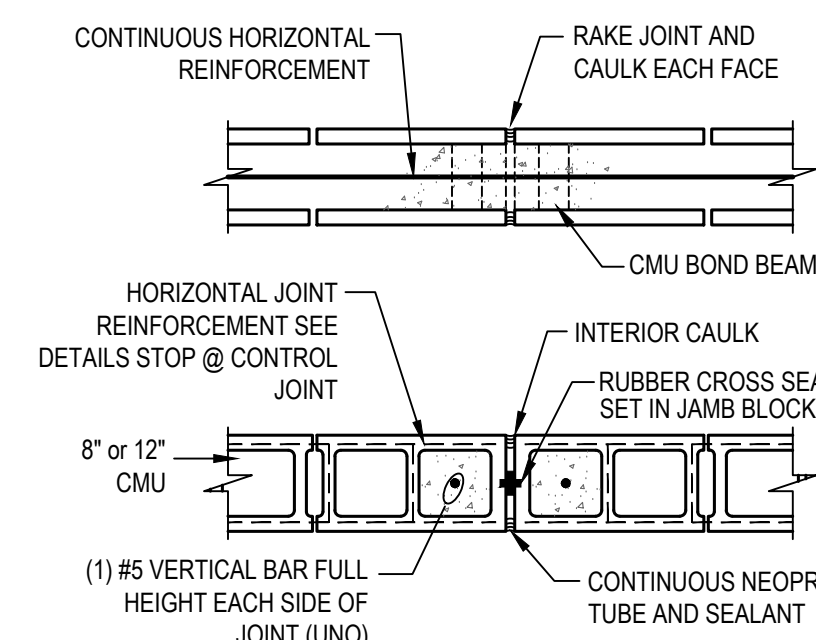
DETAIL "D"

SCALE: 3/4" = 1'-0"



DETAIL "E"

SCALE: 3/4" = 1'-0"

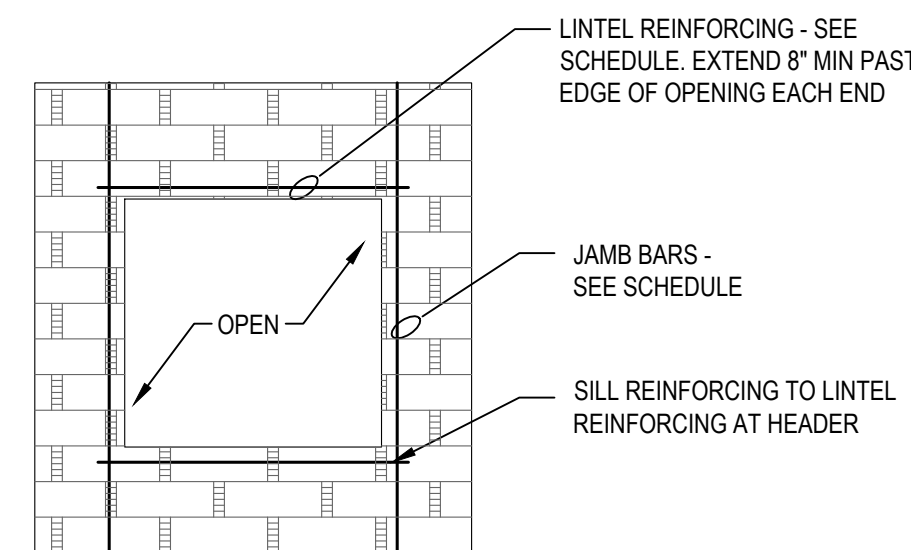


TYPICAL CONTROL JOINT

DETAIL "F"

SCALE: 3/4" = 1'-0"

NOTE:
COORDINATE CONTROL JOINT LOCATION
W/ARCHITECTURAL MAXIMUM SPACING OF
25' OR 3 TIMES THE WALL HEIGHT



TYPICAL CMU OPENING ELEVATION
SCALE: = 1'-0"



	PROJECT TITLE
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CMU DETAILS AND SCHEDULES

MASONRY HEADER (MHX) SCHEDULE				
MARK	THICKNESS	LINTEL REINFORCING	JAMB REINFORCING	# OF COURSES GROUTED
MH1	8" CMU	(1) #5	(1) #5	(2)
MH2	8" CMU	(1) #5	(1) #5	
MH3	8" CMU	(1) #5	(1) #5	
MH4	8" CMU	(1) #5	(1) #5	
MH5	8" CMU	(1) #5	(1) #5	

- NOTES:
1. 8" BEARING EACH END
 2. GROUT ALL JAMBS SOLID
 3. IF WALL THICKNESS DIFFERS IN THIS SCHEDULE FROM PLANS OR SHEAR WALL SCHEDULE, PLANS OR SHEAR WALL SCHEDULE SHOULD GOVERN.
 4. SEE DETAIL A THIS SHEET FOR HEADER DETAIL

ISSUE BLOCK		
Mark	Date	Description

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DATE:	07.24.26

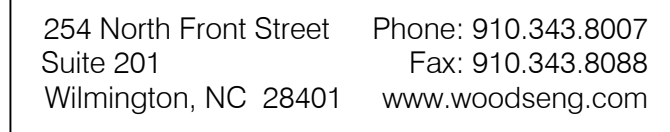
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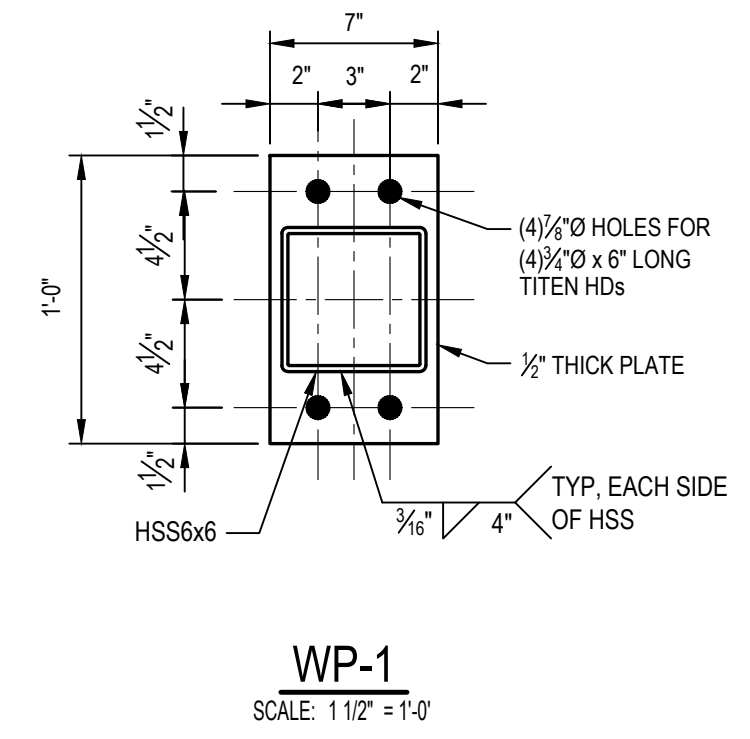
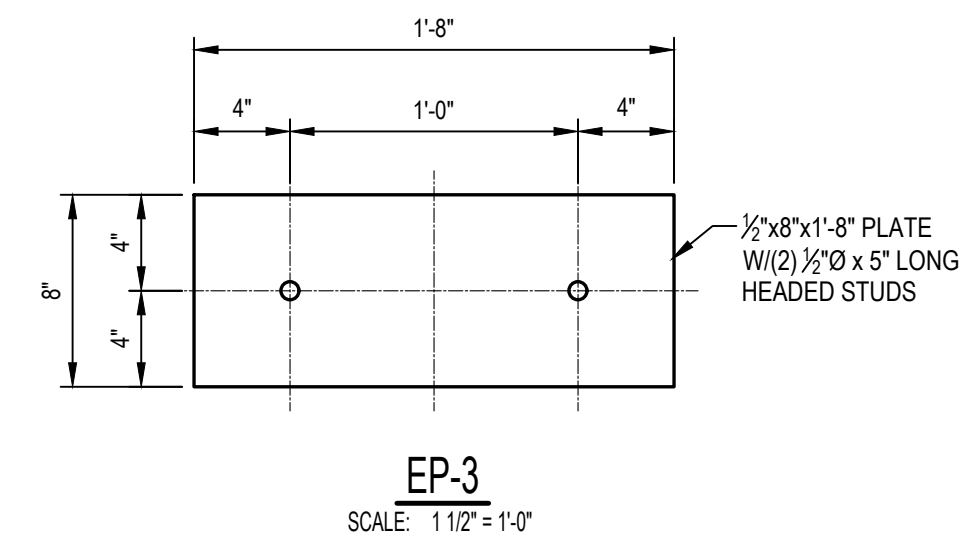
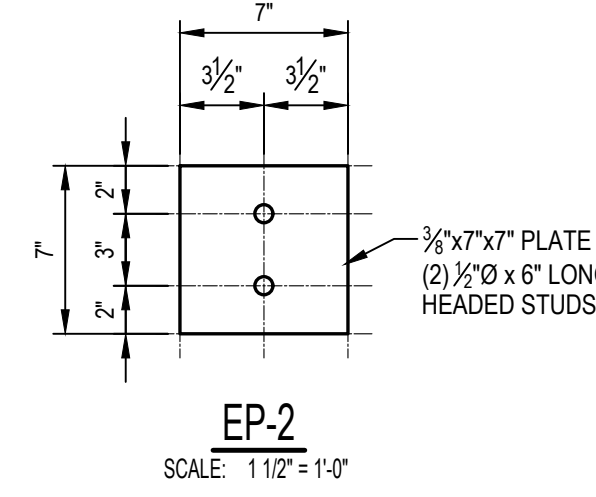
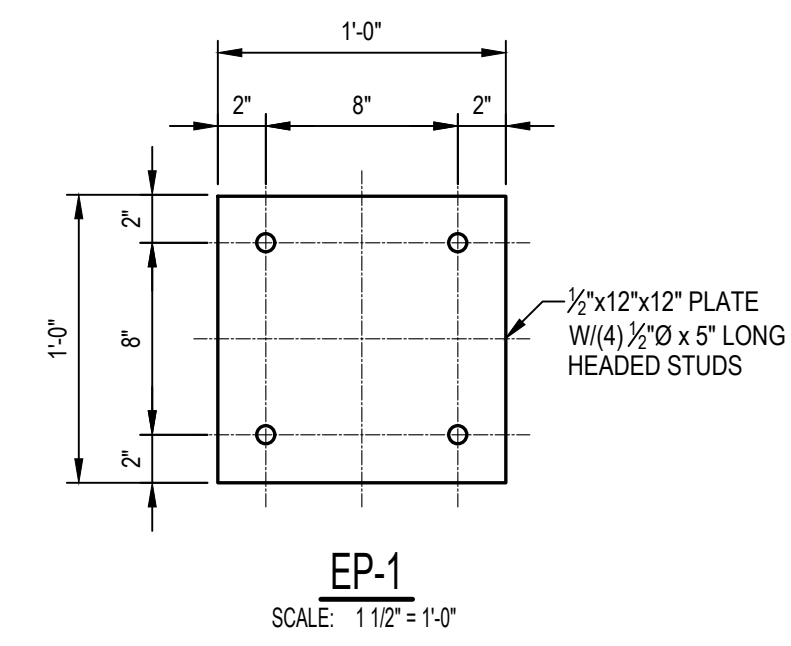


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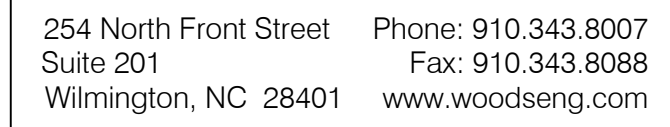
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STEEL EMBED
PLATE DETAILS

ISSUE BLOCK		
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PROJECT NO:	2023042.0
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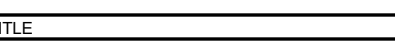
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Date	Description
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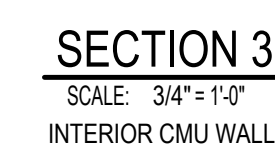
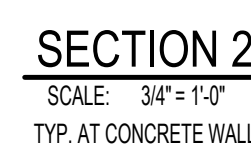
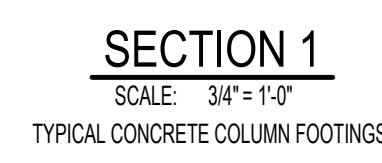
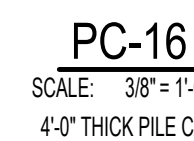
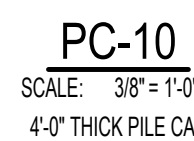
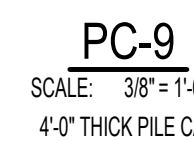
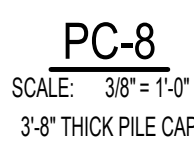
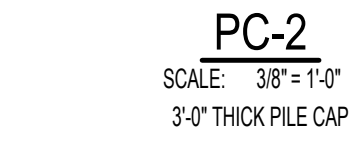
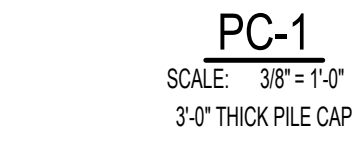
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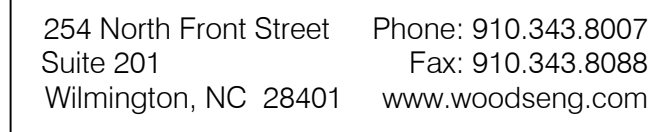


NOTES:
- CENTER REINFORCING IN CMU CELL, TYP.
- GROUT ALL WALLS SOLID BELOW SLAB
- FOR FOOTING ELEVATIONS BETWEEN
VALUES SHOWN USE REINFORCEMENT
FOR LOWER FOOTING

STEM WALL REINFORCEMENT		
T/FTG (FT.)	WALL THICKNESS	VERTICAL REINFORCEMENT
-4'-8"	8" CMU	#5 @ 48"o.c.
-8'-8"	12" CMU	#7 @ 24"o.c.

FOUNDATION SECTION FASTENING SCHEDULE			
MARK	LOCATION	FASTENER REQUIREMENTS	COMMENTS
①	SHEATHING TO SILL PLATE, TYP	8d @ 6" o.c.	U.N.O. @ SHEAR WALLS
②	SIMPSON MASA TYP SPACING	24" o.c. @ SHEAR WALLS 32" o.c. @ NON-SHEAR WALLS	SEE DETAIL A THIS SHEET
③	SHEATHING TO SILL PLATE, 1-STORY BLDGS	8d @ 3" o.c.	
④	MASA SPACING @ 1-STORY BLDGS	24" o.c. @ SHEAR WALLS 32" o.c. @ NON-SHEAR WALLS	SEE DETAIL A THIS SHEET
⑤	PINS AT SILL PLATE TO SLAB @ INTERIOR	HILTI X-CP @ 32" o.c. @ NON-SHEAR WALLS HILTI X-CP @ 4" o.c. @ SHEAR WALLS	
⑥	PINS AT SILL PLATE TO SLAB @ EXTERIOR	HILTI X-CP @ 8" o.c.	

<u>FOUNDATION SECTION NOTES</u>	
1.	DO NOT SCALE SECTIONS. SEE PLANS AND SCHEDULES FOR SIZES NOT SHOWN.
2.	REBAR IS SHOWN FOR REFERENCE ONLY. SEE PLANS AND SCHEDULES FOR REINFORCEMENT REQUIREMENTS. WHERE REINFORCEMENT IS SPECIFIED IN SECTIONS IT IS IN ADDITION TO SCHEDULES.
3.	IF A HOOK IS SHOWN ON REINFORCEMENT A STANDARD HOOK PER ACI IS REQUIRED U.N.O.
4.	IF A DISCREPANCY EXISTS BETWEEN THE SECTIONS AND PLAN THE MORE STRINGENT REQUIREMENTS SHALL APPLY
5.	WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED. U.N.O.
6.	ALL CMU SHALL HAVE JOINTS @ 48" O.C. VERTICAL AND W/17 1/2" HORIZONTAL. JOINT REINFORCEMENT @ 16" O.C. U.N.O. GROUT REINFORCED CELLS
7.	ALL EXTERIOR WORK FRAMING AND SHEATHING SHALL BE FT



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SCALE: $1/2" = 1'-0"$
GIRDER BOTTOM FLANGE
(BRACE TYP.)



SCALE: = 1'-0"



SCALE: = 1'-

NOTES:

1. SEE ARCHITECTURAL, MECHANICAL AND HVAC DRAWINGS FOR ROOF OPENING SIZES AND LOCATIONS.
2. SEE ARCHITECTURAL DRAWINGS FOR DETAILS OF CURBING AND MISCELLANEOUS STEEL.
3. ALL ROOF OPENING SIZES AND LOCATIONS SHALL BE VERIFIED WITH MECHANICAL CONTRACTOR.
4. PROVIDE SUPPORT FRAMING FOR ROOF OPENINGS 12"x12" AND LARGER.

AT CONTRACTOR'S OPTION:
USE QUICKFRAMES
WWW.QUICKFRAMES.US



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TYPICAL STEEL AND WOOD DETAILS

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ECT NO: 2023042

ECT NO: 2023042

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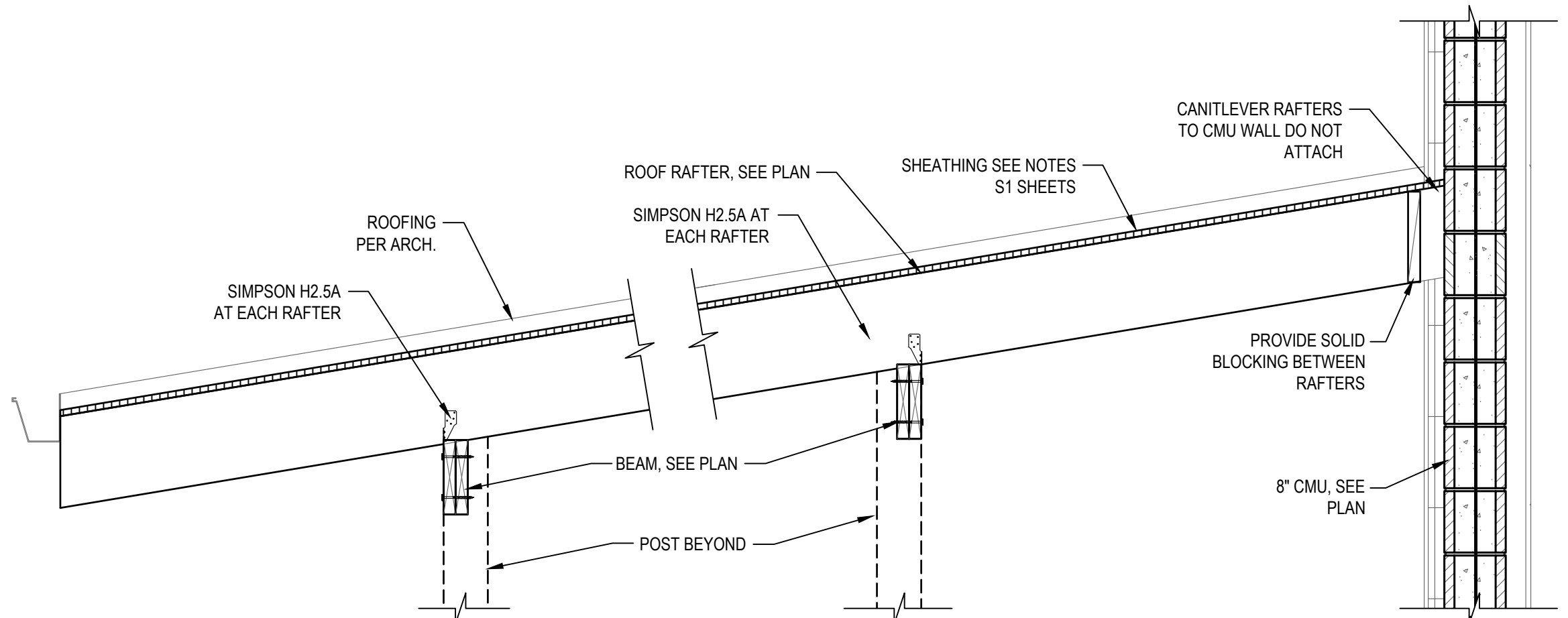
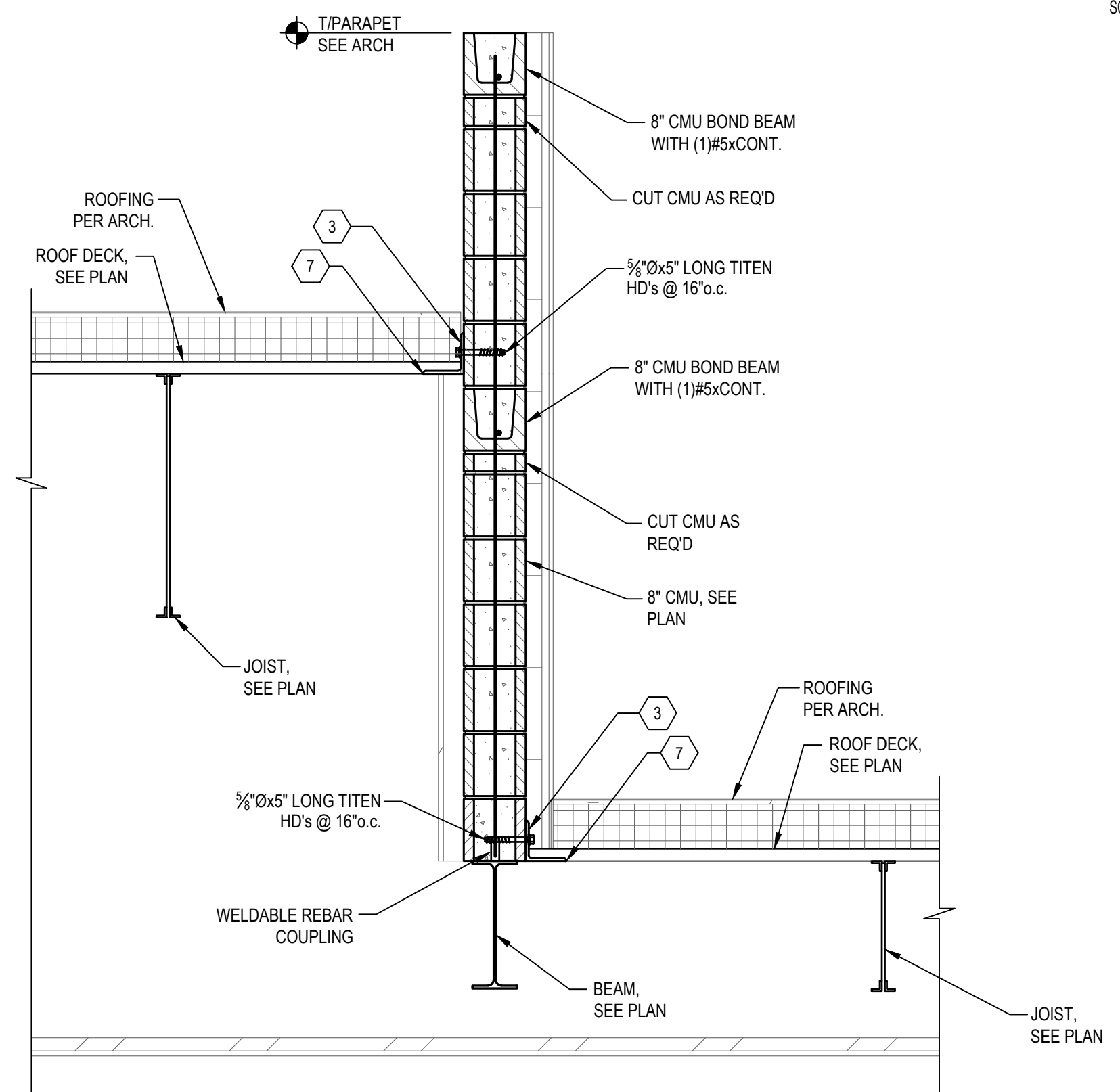
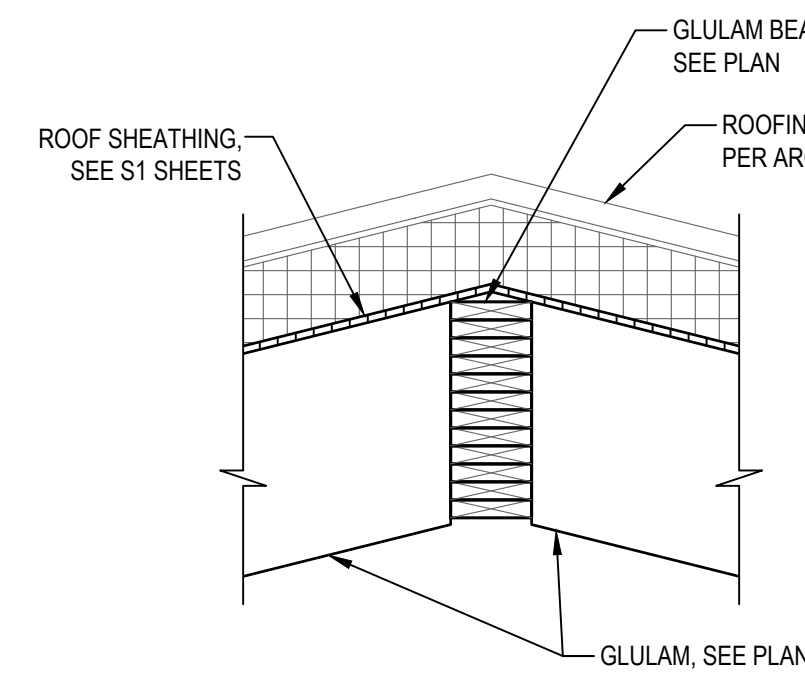
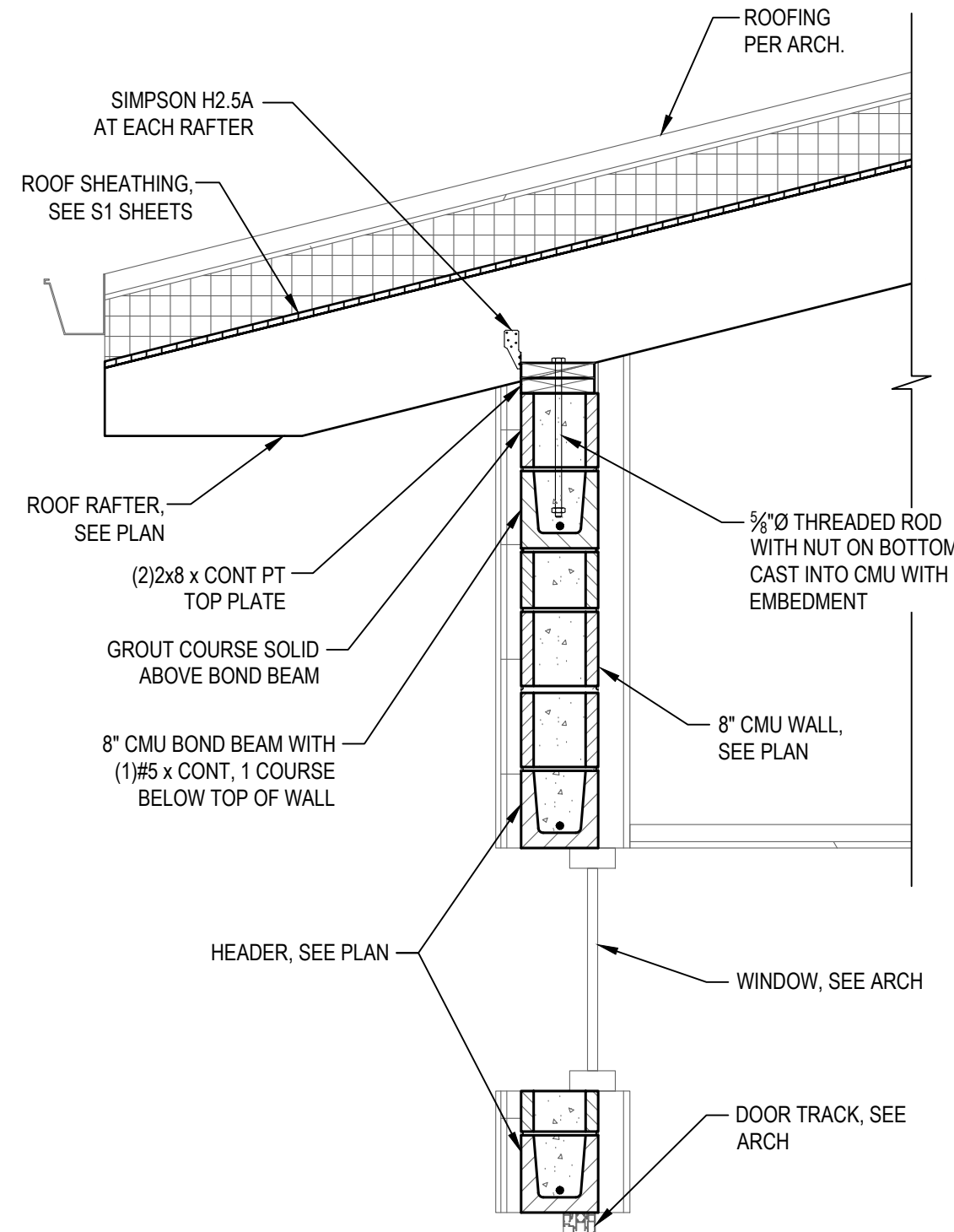
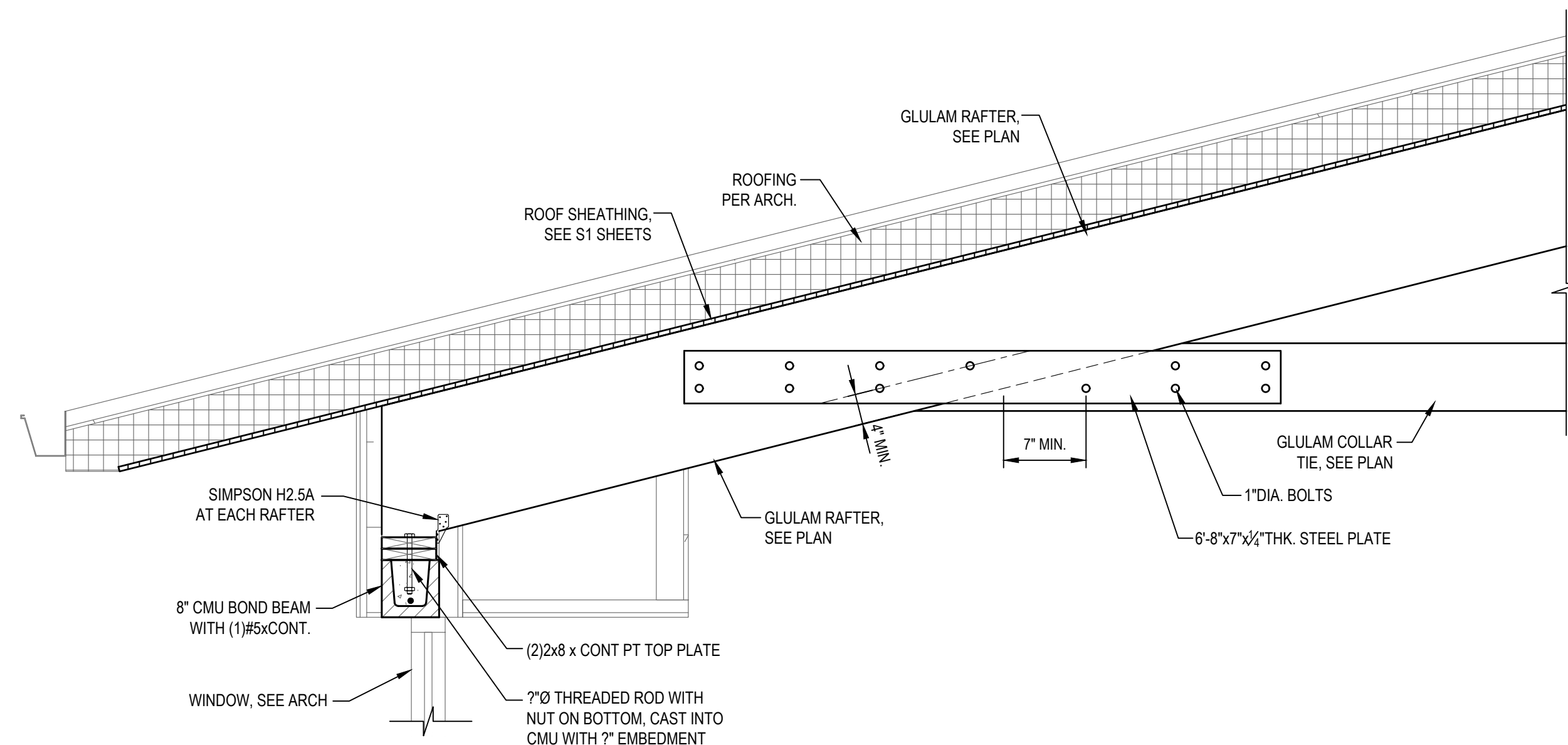
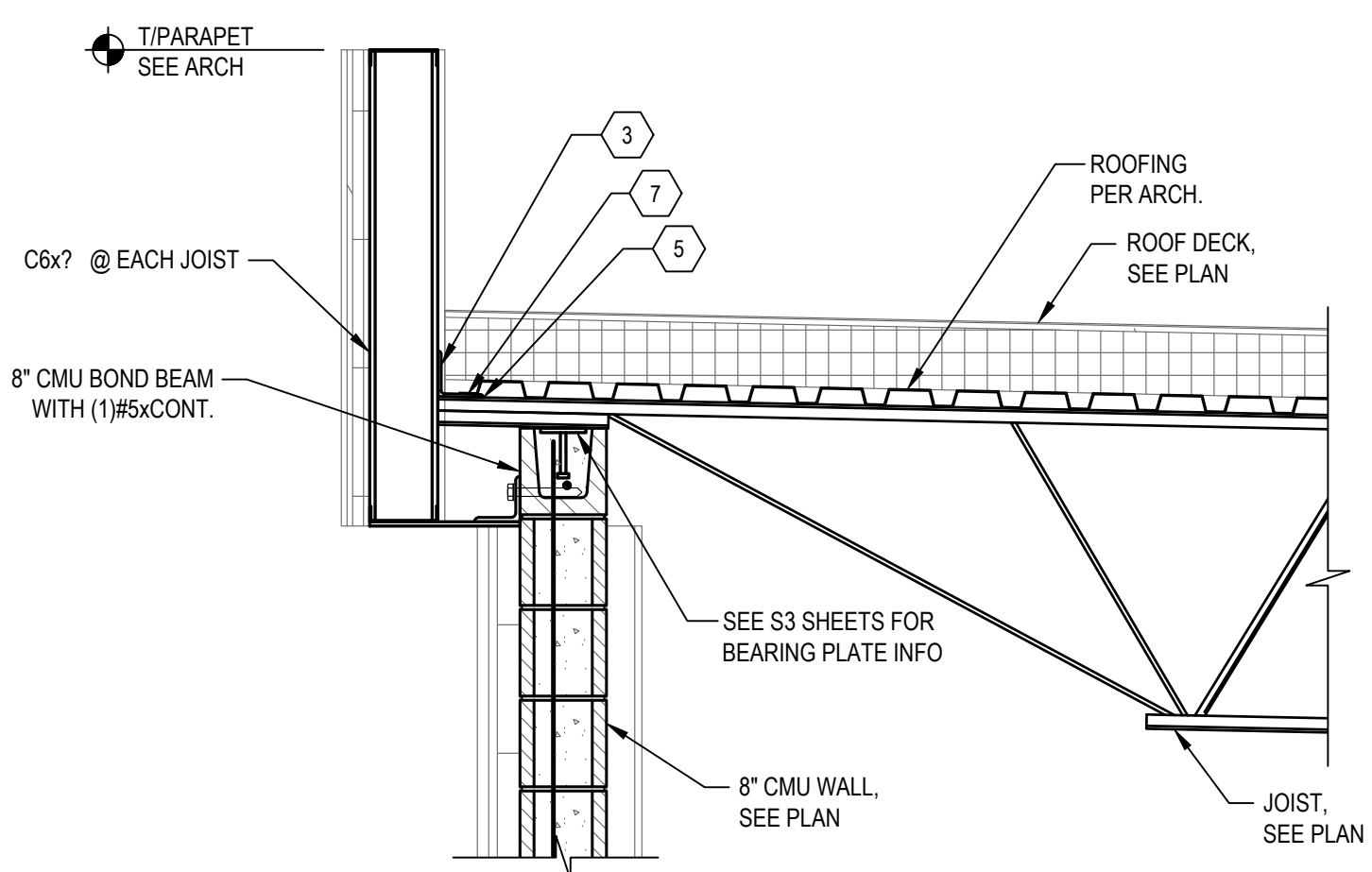
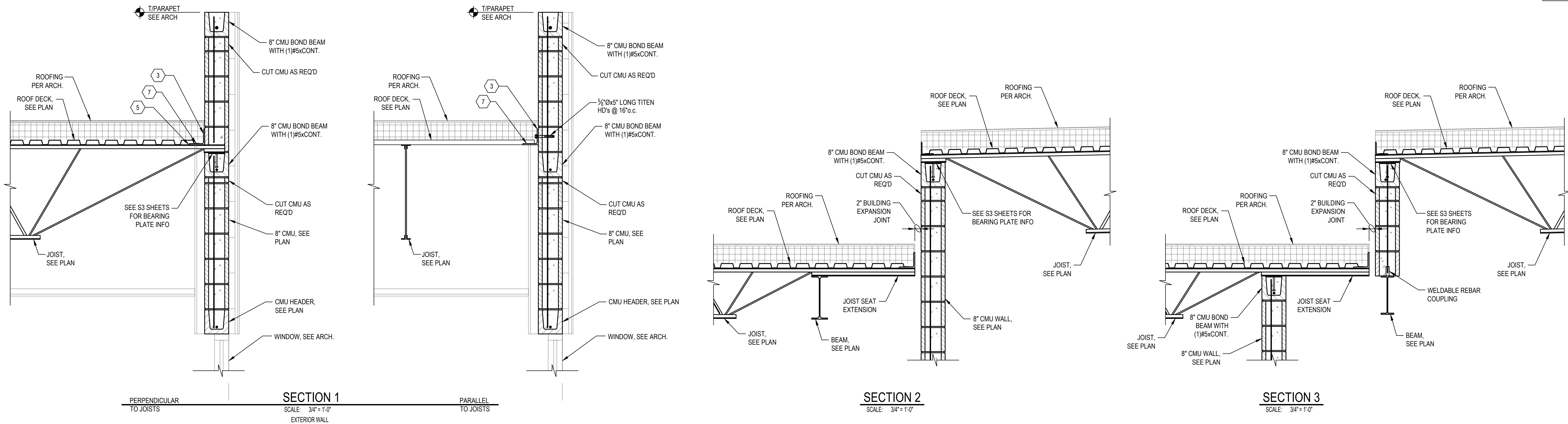
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SHEET TITLE

ROOF SECTIONS

ISSUE BLOCK

Mark Date Description

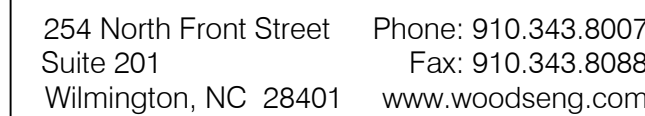
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BRIDGE SECTIONS

ISSUE BLOCK		
Mark	Date	Description

PROJECT NO.	5555-15-00
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PROJECT NO:	2023042.02
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PROJECT NO:	2023042.02
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DATE:	07.24.26
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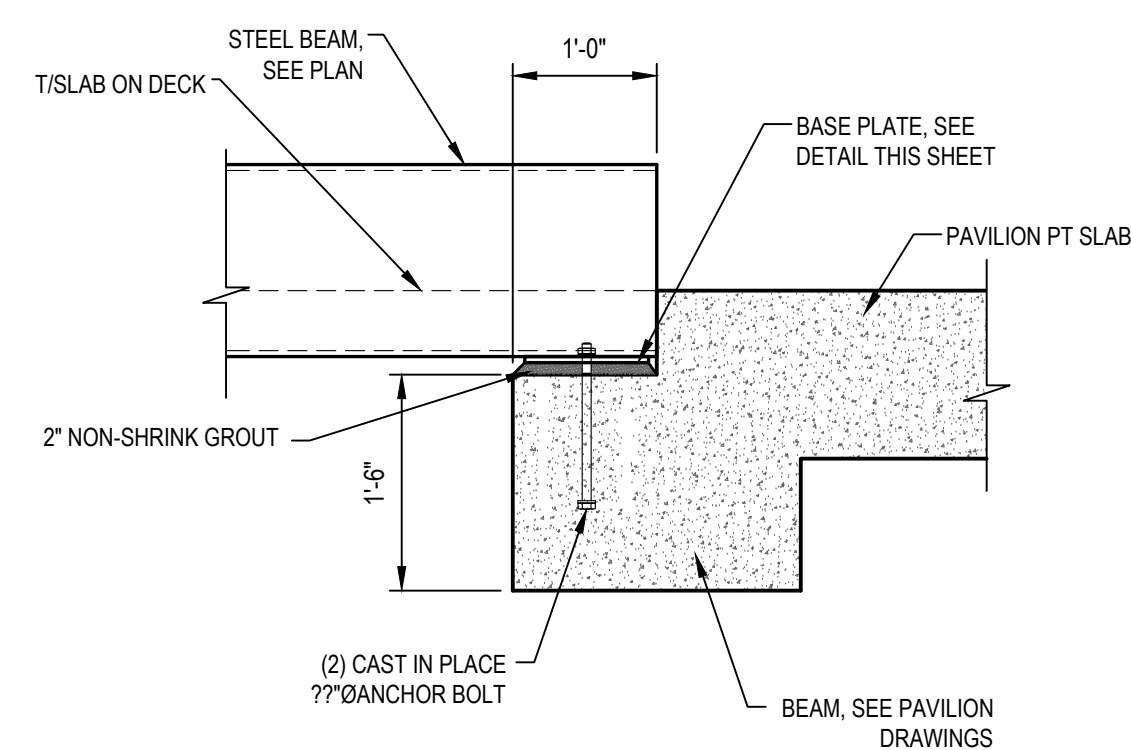
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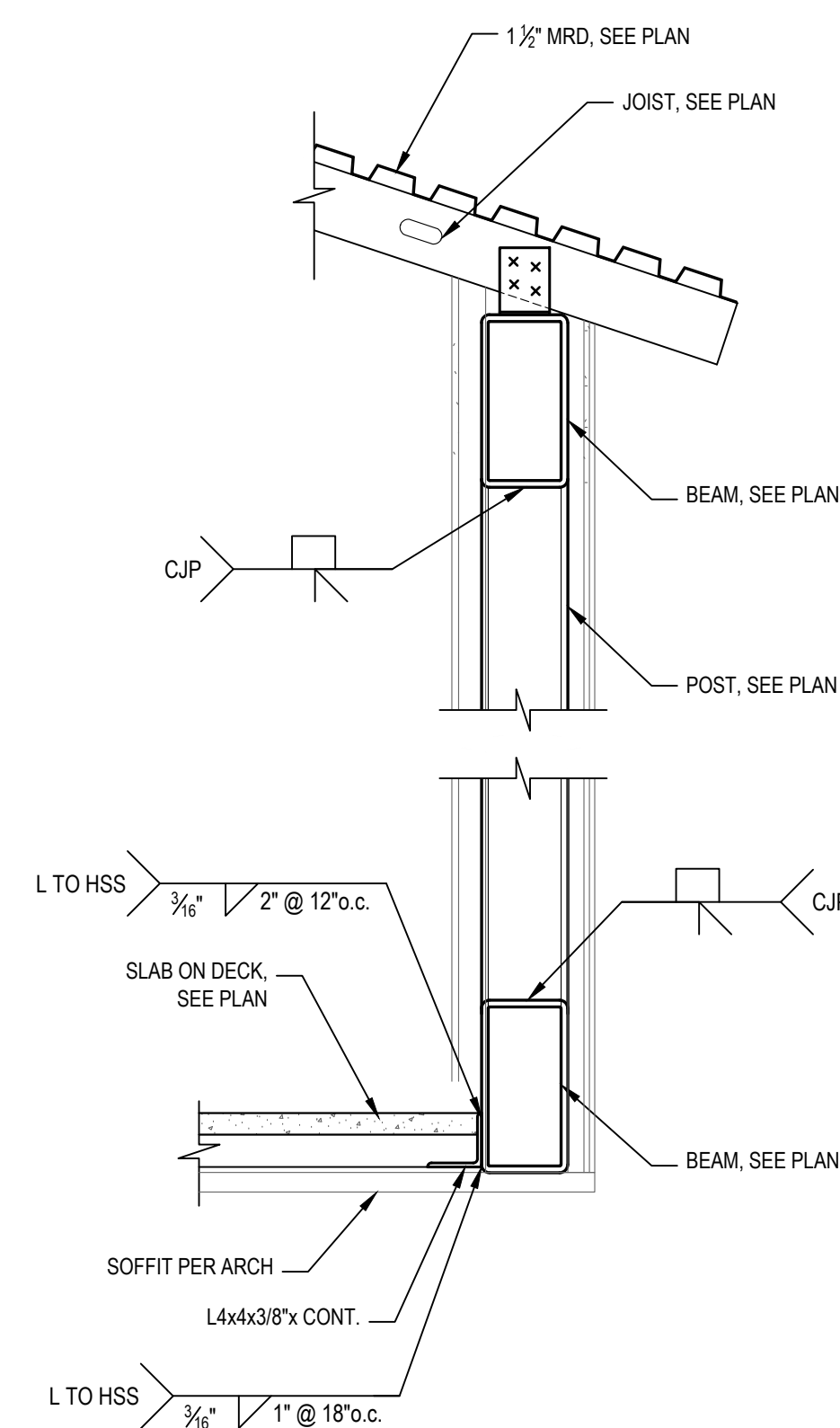
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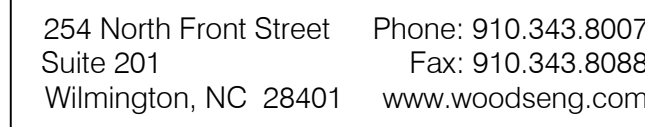
SECTION 1

SCALE: $3/4" = 1'-0"$
AT EXTERIOR LOAD BEARING WALL



SECTION 2

SCALE: $3/4" = 1'-0"$
AT EXTERIOR WALL



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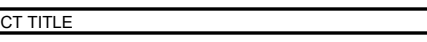
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CMU SHEAR WALL DETAILS AND SCHEDULE

ISSUE BLOCK		
Mark	Date	Description

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PROJECT NO:	202304
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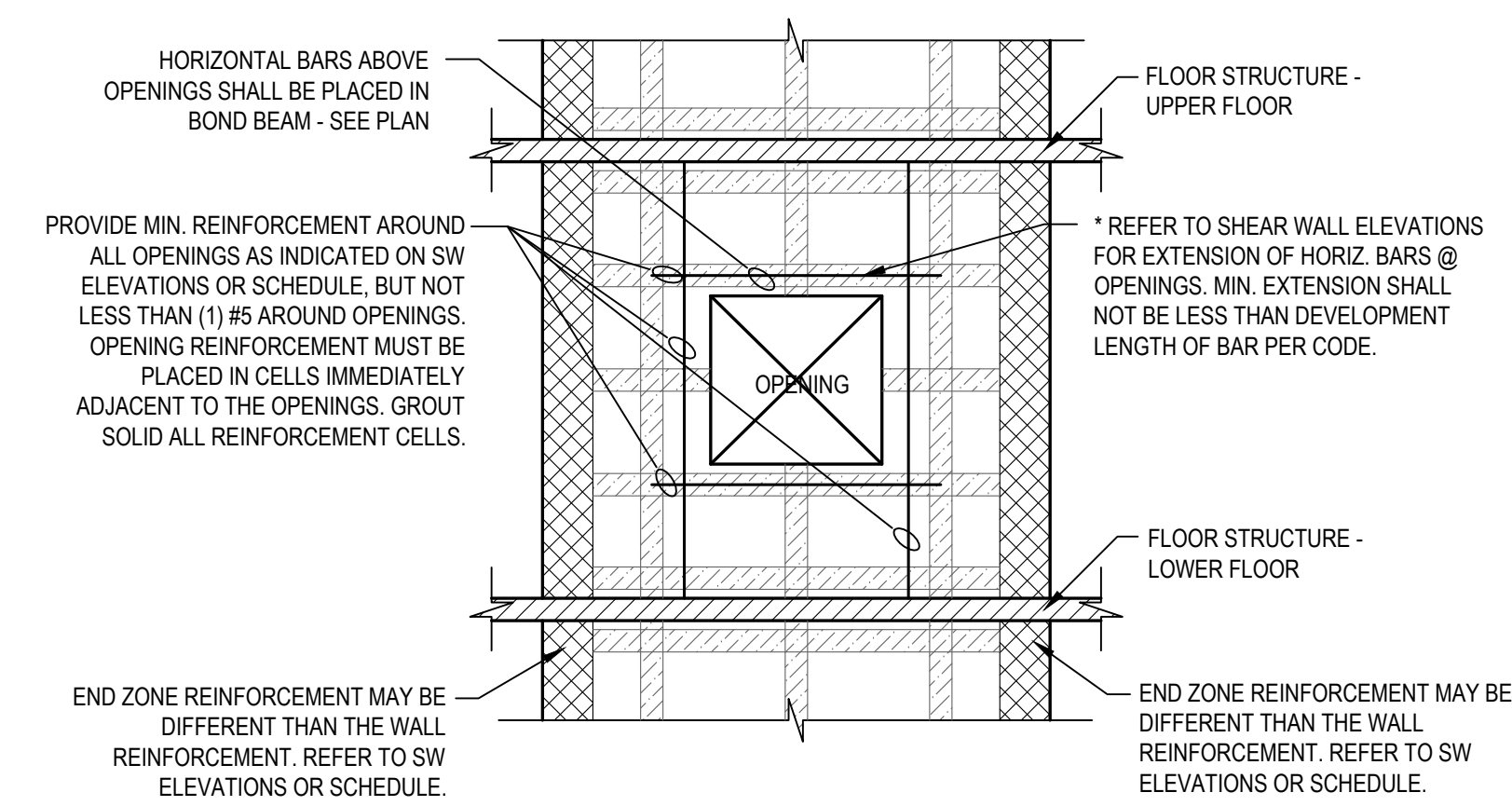
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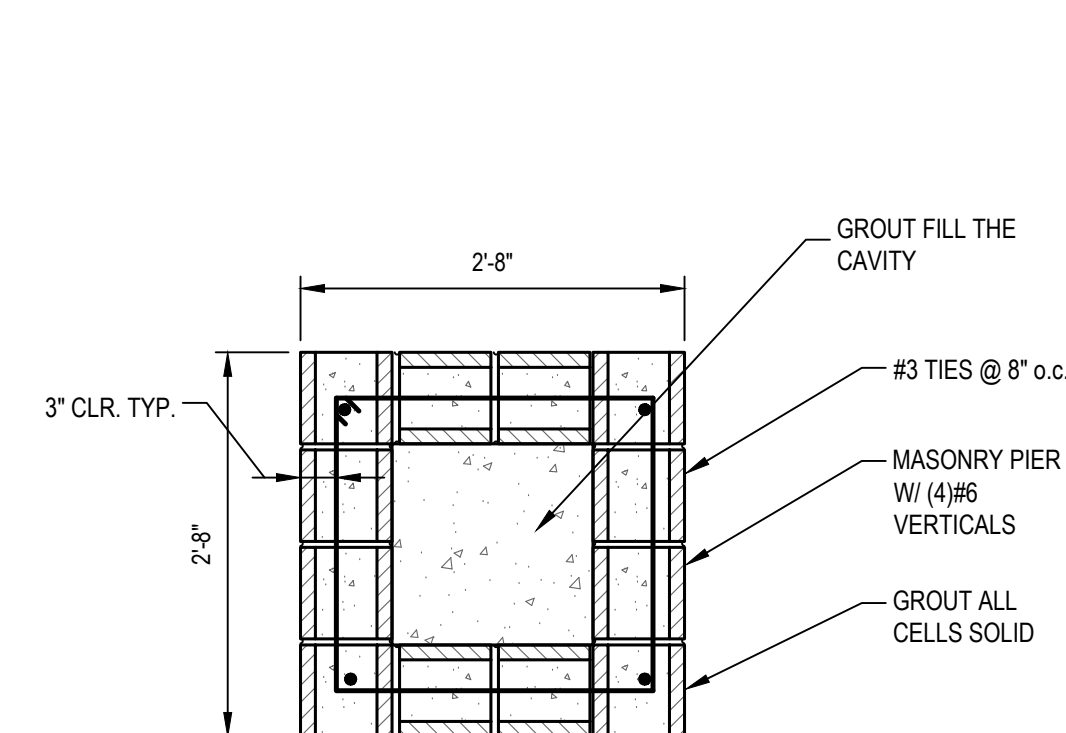
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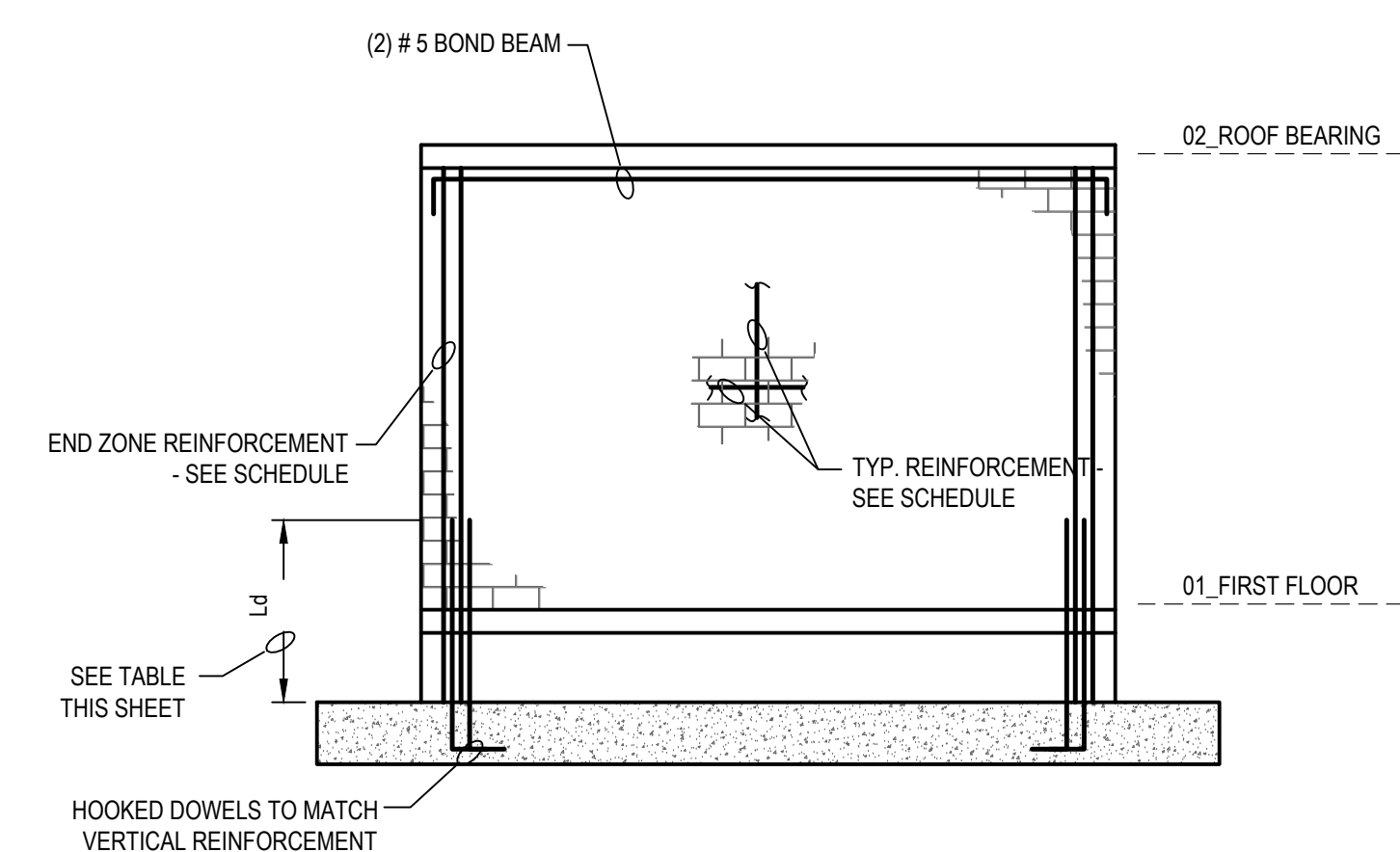
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TYPICAL CMU OPENING ELEVATION @ SHEAR WALLS



MASONRY PIER MP-1
SCALE: 3/4" = 1'-0"



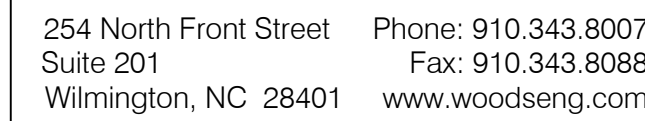
GENERIC ELEVATION
SPECIAL REINFORCEMENT

SCALE: 3/16" = 1'-0"

(SEE PLAN FOR SW LOCATIONS)

CMU SHEAR WALL NOTES

1. SHEAR WALLS 8" CMU, U.N.O. PROVIDE (2) #5 BAR AT EACH WALL CORNER & AT EACH WALL INTERSECTION - U.N.O. ON WALL SCHEDULE OR ELEVATIONS. GROUT SOLID.
2. SPACING BETWEEN VERTICAL BARS SHALL BE NO LESS THAN INDICATED BY TABLE. SEE THIS SHEET FOR REQUIRED SPICE LENGTH ACCORDING TO BAR SIZE.
3. EXTEND HORIZONTAL REINFORCEMENT AS INDICATED ON SHEAR WALL ELEVATIONS OR SCHEDULE BUT NOT LESS THAN 16 PER CODE.
4. IN FULLY GROUTED WALLS HORIZONTAL BARS SHALL BE PLACED IN NOTCHED BLOCK. IF CMU WALL IS NOT FULLY GROUTED, HORIZONTAL REINFORCEMENT SHALL BE PLACED IN A BOND BEAM. TYP.
5. T & B - TOP & BOTTOM
6. ANY EMBED DOWELS REQUIRED THAT ARE NOT SHOWN ON PLANS OR ELEVATIONS SHALL BE SAME SIZE AND SPACING AS REINFORCEMENT FOR THE NEXT FLOOR. MATCH REINFORCED CELLS ON THE FLOOR BELOW WHERE POSSIBLE.
7. ALL WALL DIMENSIONS SHOWN ON THE ELEVATIONS AND ASSOCIATED SECTIONS ARE FOR REFERENCE ONLY. COORDINATE ALL FINAL DIMENSIONS WITH ARCHITECTURAL PLANS. IF SIGNIFICANT CONFLICT EXISTS, CONTACT STRUCTURAL ENGINEER AND ARCHITECT FOR RESOLUTION.
8. 48% MAX SPACING OF VERT. REINFORCEMENT OR AS INDICATED ON SW SCHEDULE OR ELEVATIONS. REINFORCEMENT THAT STOPS AT OPENINGS OR DOESN'T CONTINUE ON THE FLOOR ABOVE SHALL BE TERMINATED WITH STANDARD HOOK & FULLY ENGAGED IN BOND BEAM.
9. CMU WALLS NOT INDICATED AS SHEAR WALLS SHALL HAVE A VERTICAL #5 @ EACH CORNER @ EACH OPENING, AND @ 48% MAX. GROUT REINFORCED CELLS. PROVIDE W1.7 HORIZONTAL JOINT REINFORCEMENT @ 16" C.
10. ALL CMU SHEAR WALLS SHALL HAVE W1.7 HORIZONTAL JOINT REINFORCEMENT @ 16" U.N.O. IN SCHEDULE OR ELEVATIONS. HORIZONTAL BARS REPLACE W1.7 JOINT REINFORCEMENT.
11. GROUT SHALL BE HELD DOWN 1/2" BELOW TOP OF A COURSE TO FORM A KEY AT THE JOINT. SEE STRUCTURAL MASONRY NOTES ON S1.01 FOR ADDITIONAL INFORMATION.
12. SEE TYPICAL OPENING DETAIL ON SHEET S5.01 FOR TYPICAL REINF.
13. DUCT OPENINGS SHALL NOT BE LOCATED WITHIN 2'-0" OF ANY WALL END, U.N.O. - OPENINGS SHOULD NOT INTERRUPT ANY CONTINUOUSLY REINFORCED DOOR JAMBS.
14. DO NOT PROVIDE CONTROL JOINTS IN CMU SHEAR WALLS.
15. ALL MASONRY WALL CELLS OR CAVITIES INDICATED AS REINFORCED SHALL BE GROUTED FOR THE FULL HEIGHT OF THE WALL, UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. UNREINFORCED CELLS INDICATED AS GROUTED SHALL BE GROUTED FULL HEIGHT, UNLESS SPECIFICALLY NOTED OTHERWISE. MORTAR FILL IS NOT PERMITTED.
16. ALL BOND BEAMS SHALL BE FILLED WITH GROUT AND REINFORCED AS INDICATED ON THE DRAWINGS (DETAILS OR SCHEDULES). MORTAR FILL IS NOT PERMITTED.
17. SEE S1 SHEETS FOR CMU NOTES AND TYPICAL DETAILS.



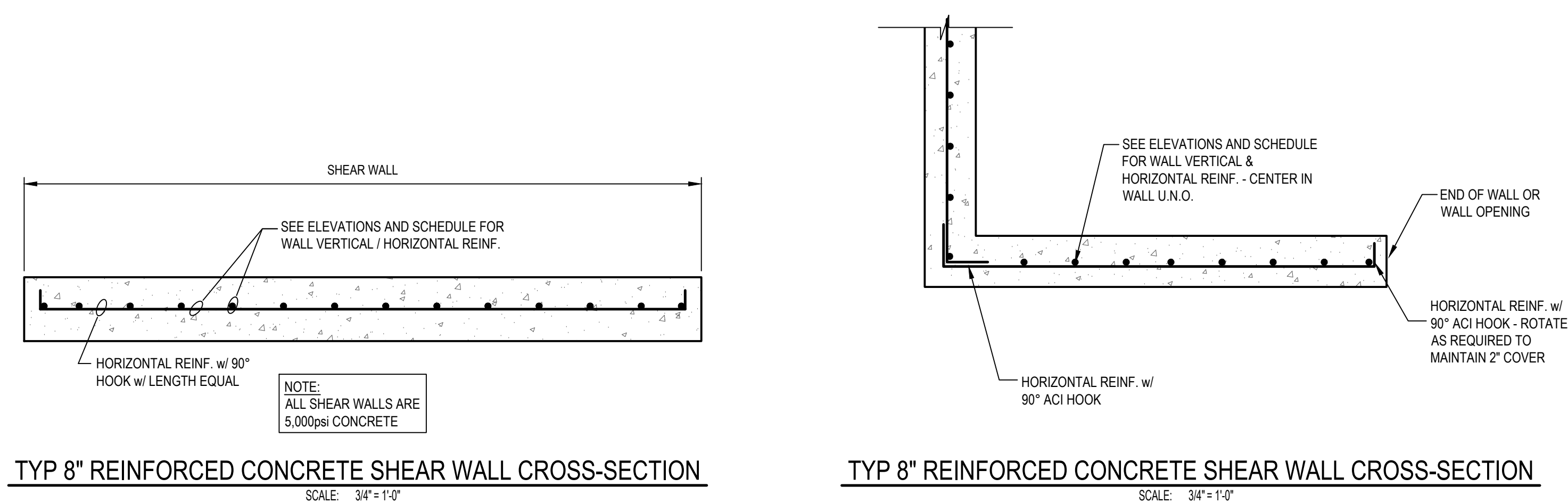
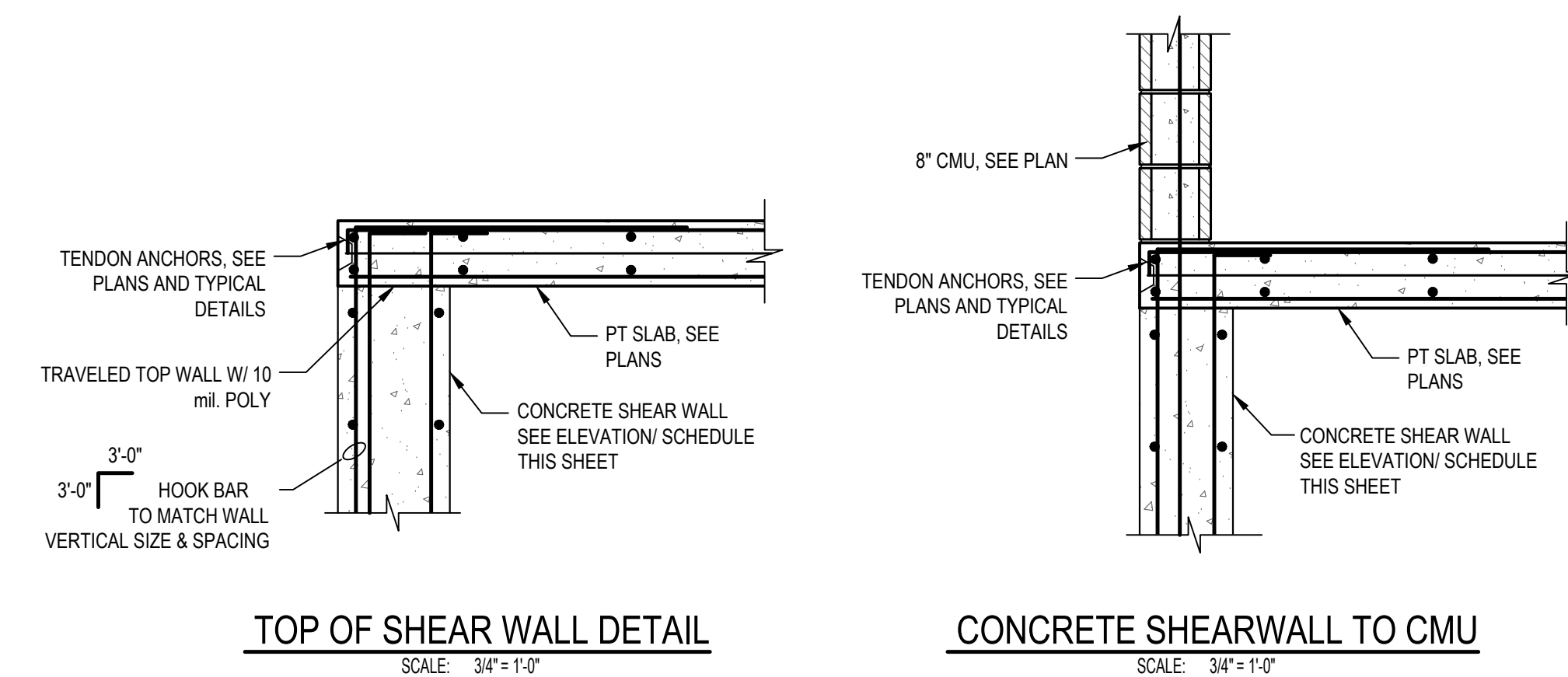
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CONCRETE SHEAR WALL (CSW) SCHEDULE						
WALLS ID →	CSW-1 (8" THICK)		CSW-2 (8" THICK)		CSW-3 (8" THICK)	
	VERTICAL BARS EACH FACE	HORIZONTAL BARS EACH FACE	VERTICAL BARS EACH FACE	HORIZONTAL BARS EACH FACE	VERTICAL BARS EACH FACE	HORIZONTAL BARS EACH FACE
FLOOR ↓						
FDN TO 2ND	#4@12" o.c.	TYP	#5@12" o.c.	TYP	#5@12" o.c.	TYP

1. 'TYP' INDICATES #4's @ 16"o.c.
2. FOR WALLS NOT LISTED IN THIS TABLE SEE ELEVATIONS THIS SHEET AND SUBSEQUENT SHEETS
4. SEE DETAIL A AND SECTION B THIS SHEET FOR SHEAR WALL SECTION

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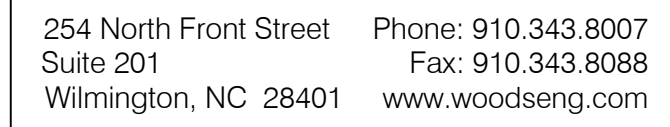
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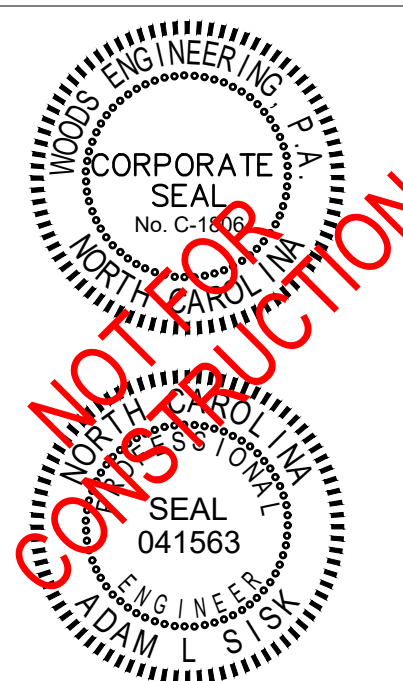
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CONCRETE COLUMN SCHEDULE AND DETAILS

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Date	Description
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ECT NO: 202304:

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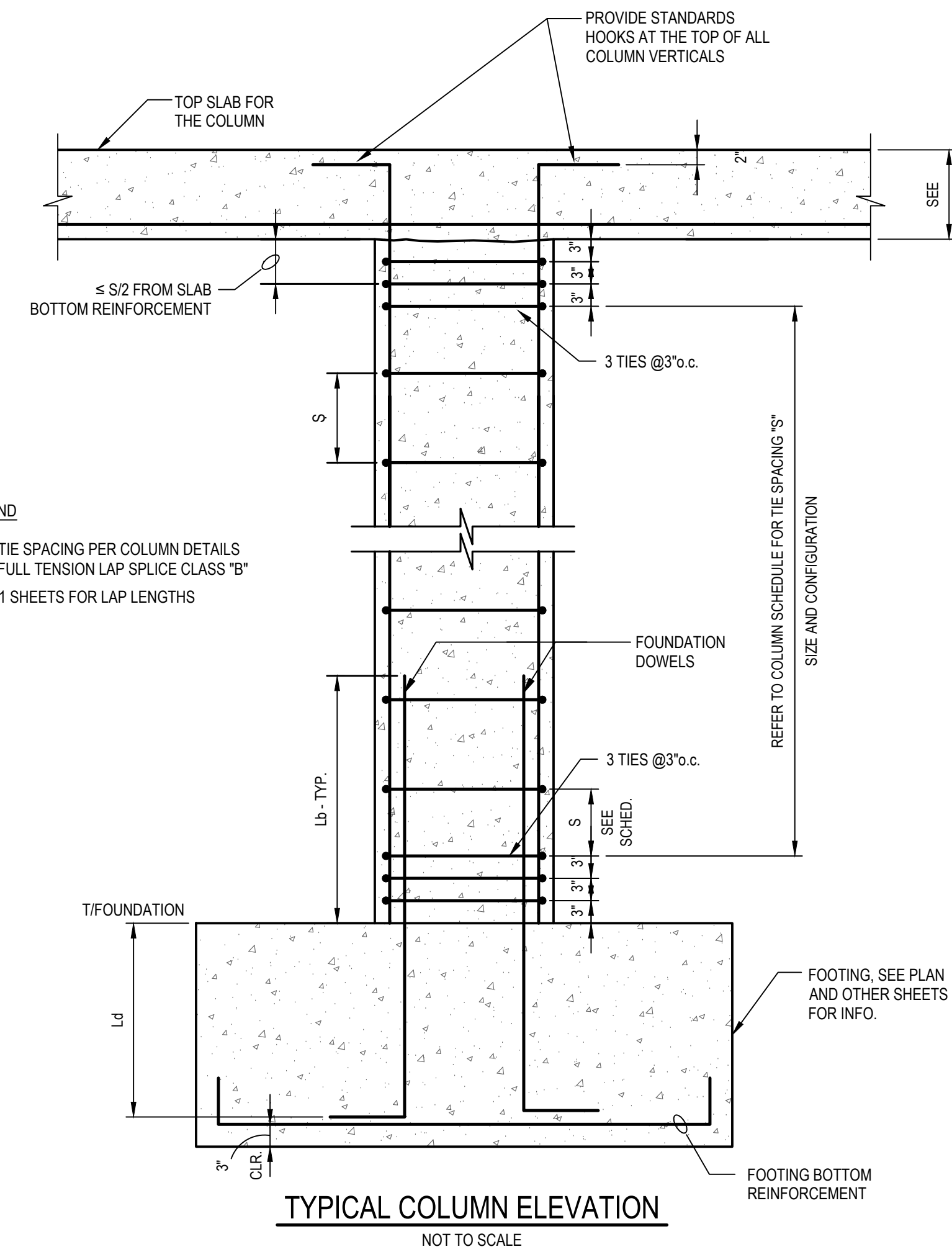
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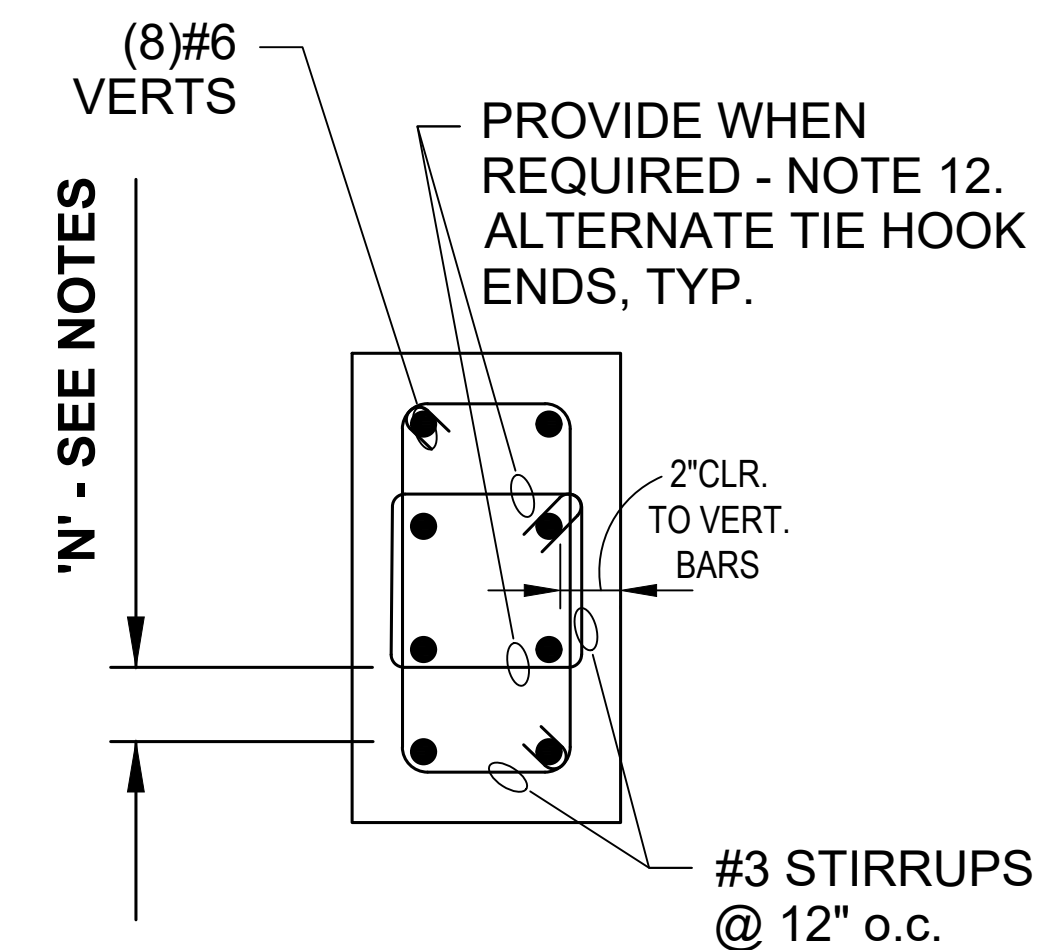
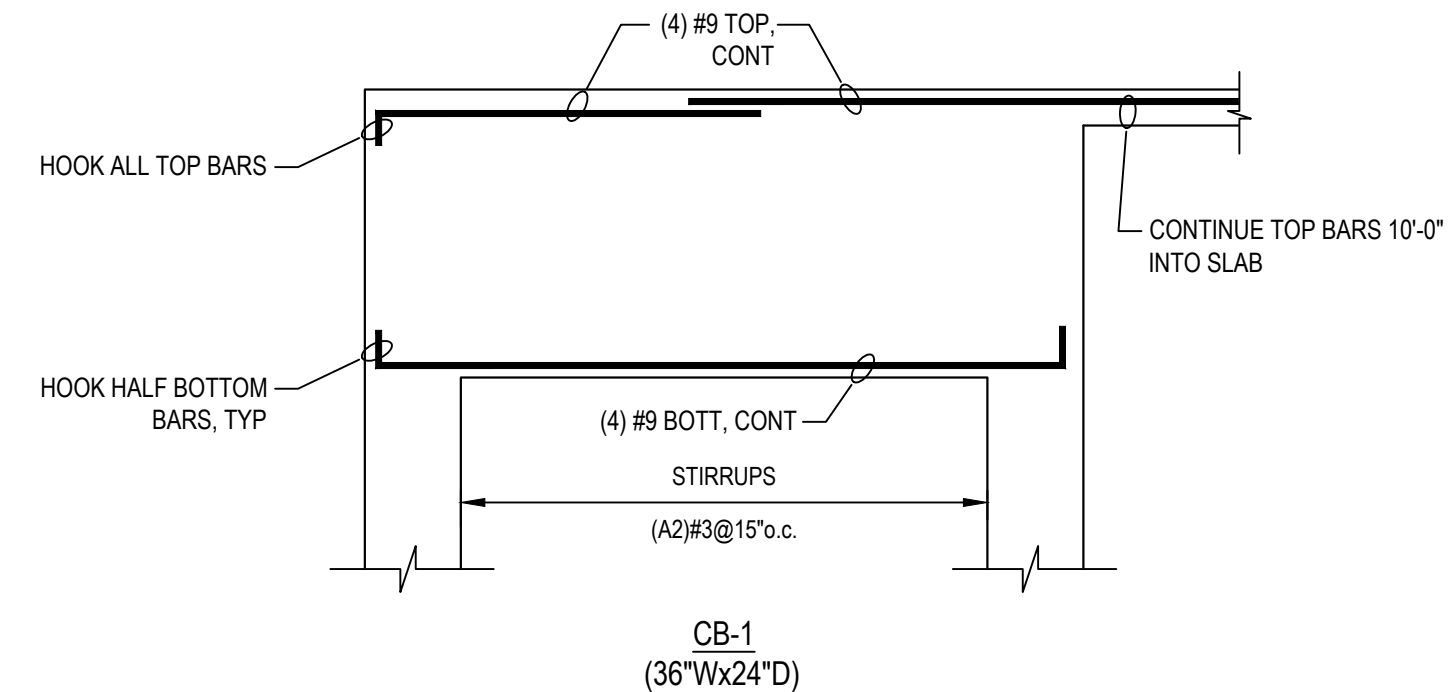
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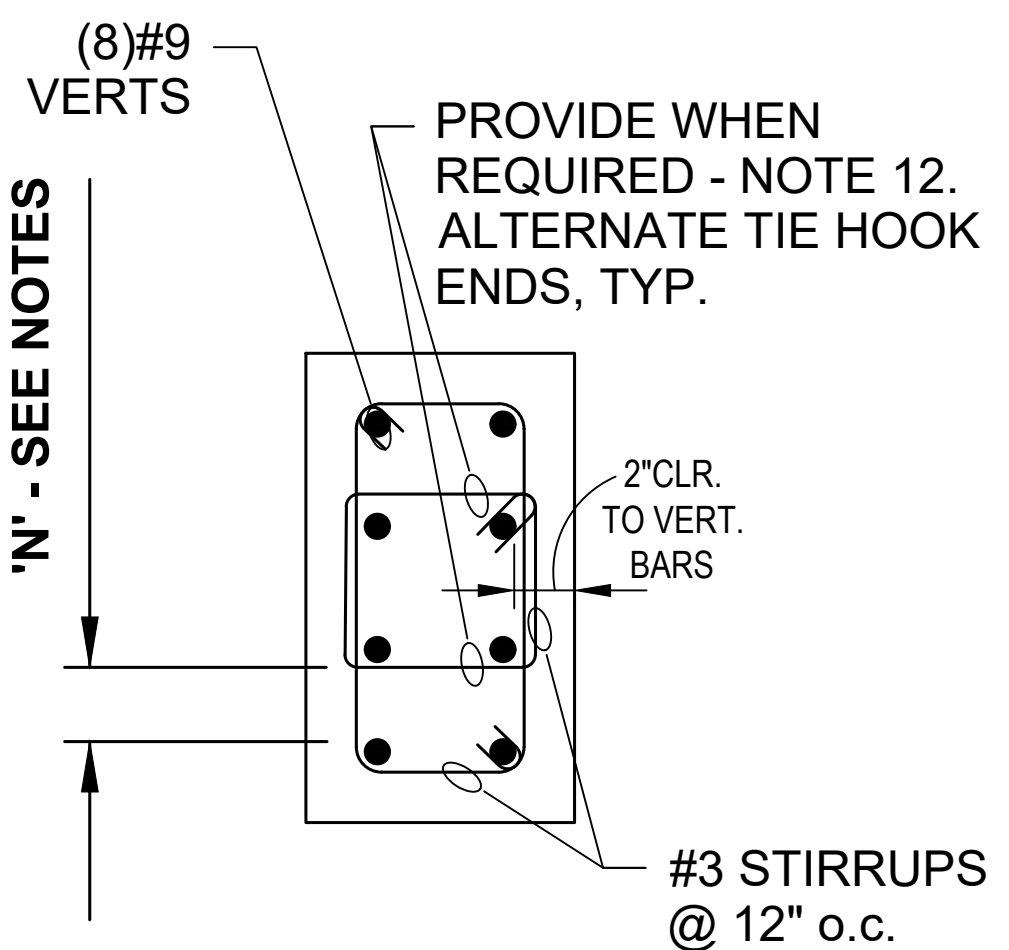


COLUMN SCHEDULE NOTES:

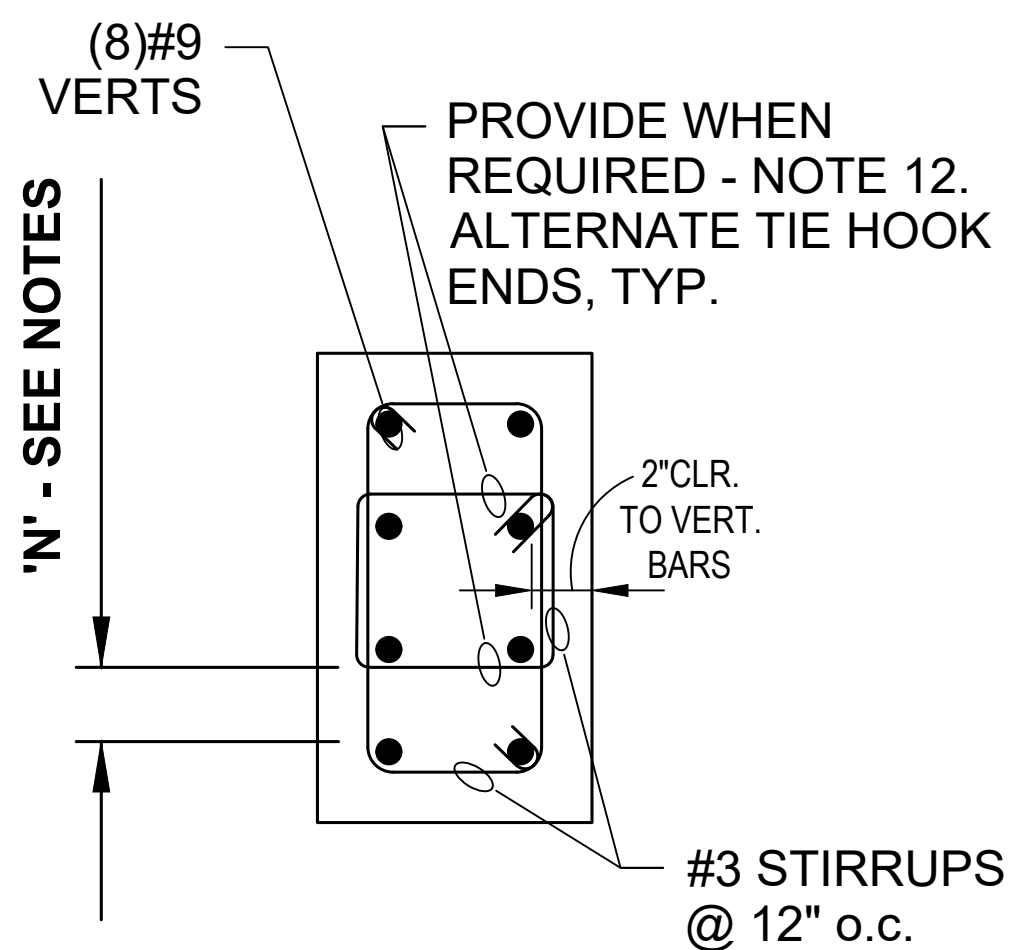
1. EXTEND COLUMN DOWELS DOWN TO FOOTING BOTTOM REINFORCEMENT AND PROVIDE STANDARD HOOKS. DOWELS SHALL EXTEND ABOVE TOP OF FOUNDATION FOR THE FULL TENSION LAP SPlice CLASS "B" LENGTH PER ACI 318-11. - SEE TYP. COLUMN ELEVATION.
2. EACH CONSECUTIVE LEVEL OF PERIAPICAL REINFORCEMENT SHALL EXTEND FROM TOP OF LEVEL BELOW TO TOP OF LEVEL ABOVE PLUS FULL TENSION LAP SPlice CLASS "B" - SEE TYP. COLUMN ELEVATION.
3. PROVIDE STANDARD HOOKS AT TOP OF ALL COLUMN VERTICALS.
4. ALL COLUMNS VERTICAL REINFORCEMENT LAP SPICES ARE CLASS "B".
5. STIRRUP SPACING IS AS SHOWN ON SCHEDULE. INSTALL FIRST AND LAST STIRRUP AT EACH LEVEL AS SHOWN ON TYP. COLUMN ELEVATION.
6. ALL COLUMNS SHALL BE CONCENTRIC FULL HEIGHT U.N.O. ON PLANS.
7. PROVIDE DOVETAIL SLOTS IN EACH FACE OF COLUMN THAT MEETS MASONRY WALL (WHERE APPLICABLE).
8. TYP. CLEAR CONCRETE COVER TO COLUMN VERTICALS IS 2"
9. CROSS TIES HAVE SAME DIA. & SPACING AS MAIN STIRRUPS U.N.O. ON SCHEDULE. ALTERNATE 90° HOOK EA. CONSECUTIVE CROSS TIE - TYP.
10. ALL STIRRUPS, TIES & VERTICAL REINFORCEMENT (WHERE INDICATED) TO HAVE SEISMIC HOOKS - TYP.
11. LB WHERE INDICATED REFERS TO THE FULL TENSION LAP SPlice LENGTH, CLASS "B" PER ACI



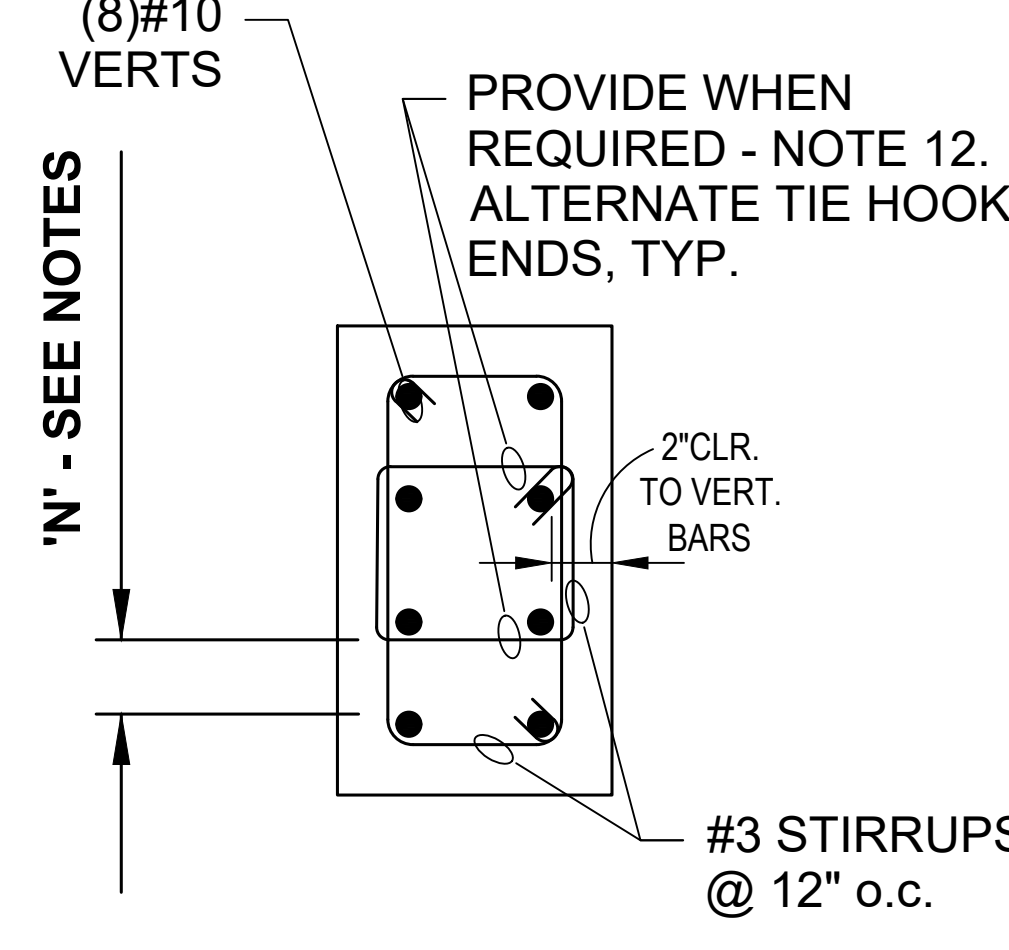
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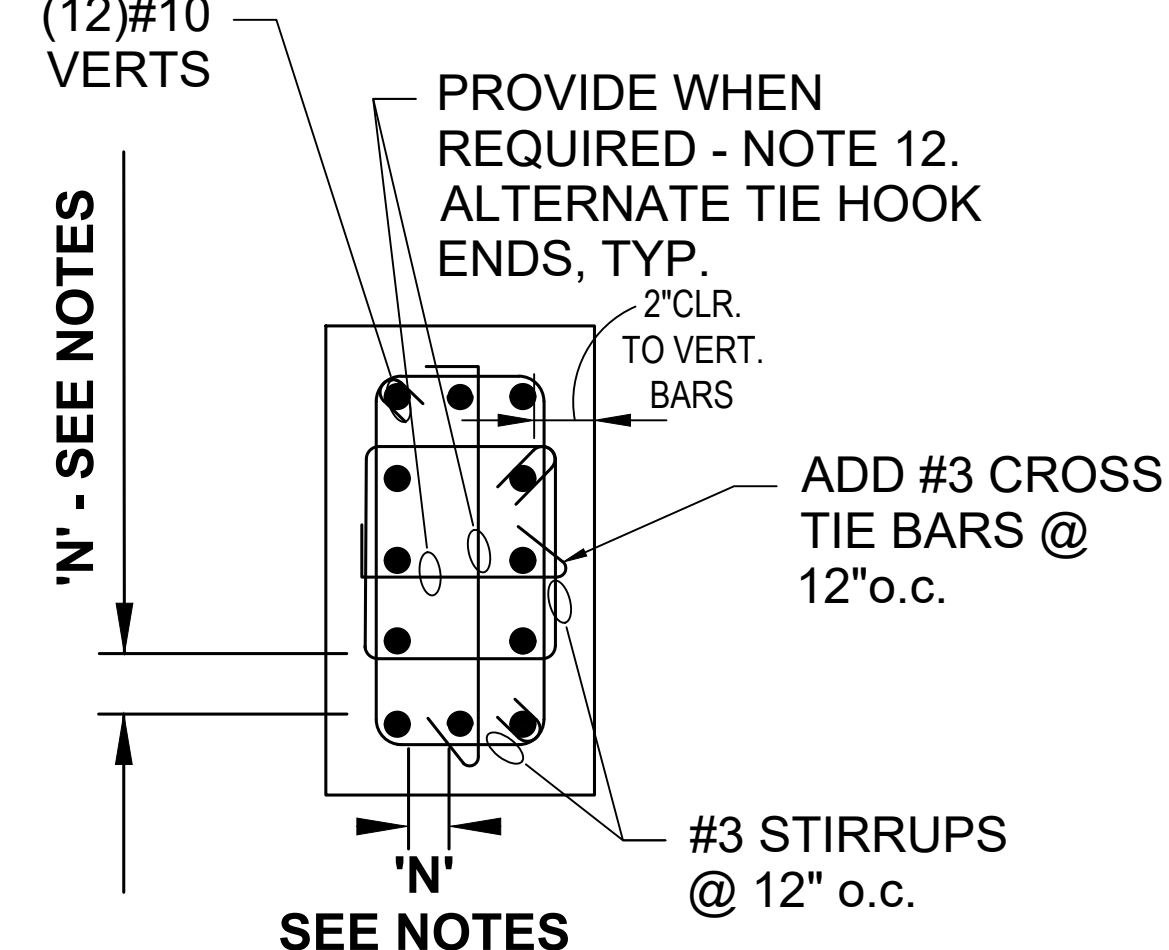
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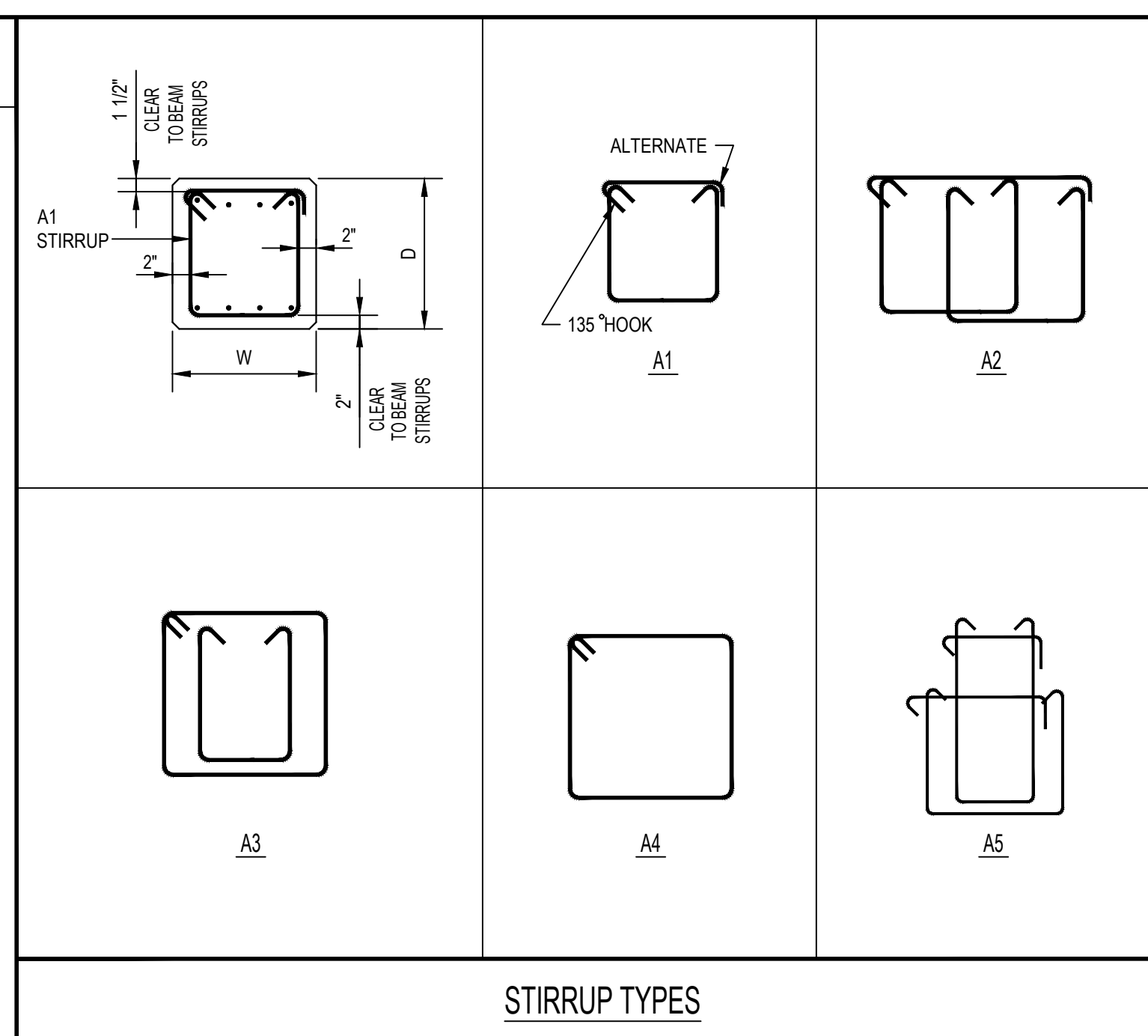
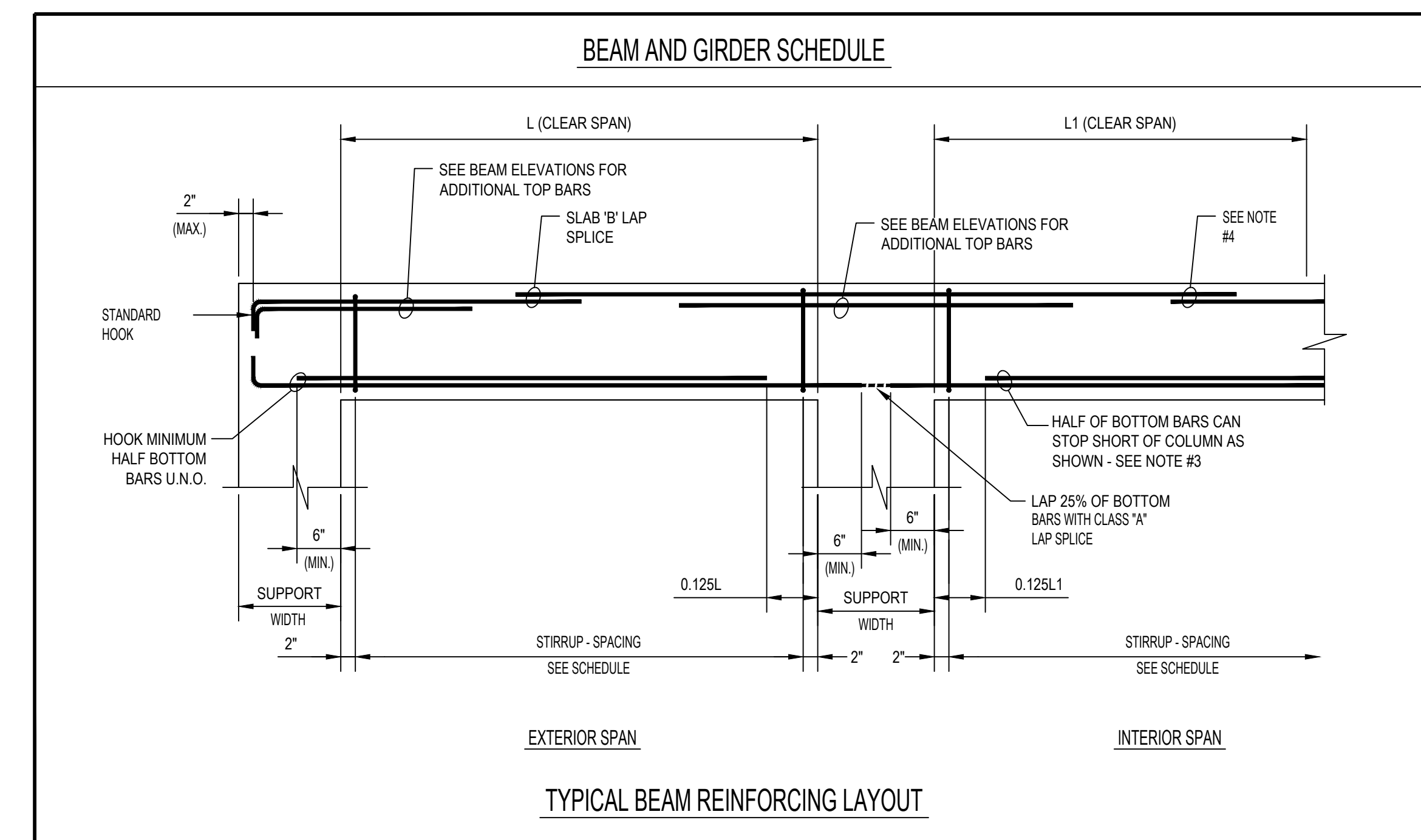
C3-14x24



C4-14x24



C5-16x32



SCHEDULE GENERAL NOTES

1. REINFORCING INDICATED BY THE DASHED LINE IS BILLED WITH THE ADJACENT SPAN
2. AT DISCONTINUOUS ENDS, EXTEND ALL OF THE BOTTOM BARS INTO SUPPORTS AS SHOWN ON LAYOUT ELEVATION ABOVE. PROVIDE STANDARD HOOKS FOR AT LEAST 25% OF THE BOTTOM BARS AND EXTEND TO FAR END OF SUPPORT (SEE ELEVATION).
3. UNLESS NOTED OTHERWISE IN SCHEDULE, AT INTERIOR SUPPORTS A MINIMUM OF 50% OF THE BOTTOM REINFORCING SHALL EXTEND INTO SUPPORTS AS SHOWN ON LAYOUT ELEVATION, OF WHICH AT LEAST ONE-THIRD SHALL BE CONTINUOUS THROUGH SUPPORT OR SPLICED WITH A CLASS "A" TENSION LAP OVER SUPPORT. THE REMAINDER OF THE BOTTOM BARS MAY TERMINATE AT 1/2L FROM FACE OF SUPPORT AS SHOWN IN ELEVATION TO LEFT.
4. WHERE TOP BARS ARE SHOWN TO BE CONTINUOUS IN SCHEDULE, A CLASS "B" TENSION LAP SPLICE MAY BE PROVIDED AT BEAM CENTERLINE, UNLESS NOTED OTHERWISE IN DETAILS.
5. AT PERIMETER BEAMS OR GIRDS AT LEAST ONE-SIXTH OF THE TOP REINFORCING REQUIRED AT SUPPORT SHALL BE CONTINUOUS THROUGH SPAN. THE REMAINDER AT SUPPORT WITH A CLASS "B" TENSION LAP, UNLESS NOTED OTHERWISE IN DETAILS.
6. BAR LENGTH TO BE DETERMINED BY ABOVE TYPICAL DETAIL UNLESS LENGTH IS CALLED OUT IN THE BEAM DETAILS.

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