GENERAL CONDITIONS AND STATEMENTS

1. THESE NOTES SHALL APPLY UNLESS INDICATED OTHERWISE BY DRAWINGS OR SPECIFICATIONS. IN THE EVENT THAT CONFLICTS OCCUR BETWEEN THESE NOTES, DRAWINGS OR SPECIFICATIONS NOTIFY THE STRUCTURAL ENGINEER FOR RESOLUTION PRIOR TO PROCEEDING WITH THE WORK.
2. STRUCTURAL DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH THE SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT.
3. SUBMIT SHOP DRAWINGS ON ALL STRUCTURAL MATERIALS FOR APPROVAL BEFORE FABRICATION. CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMISSION.
4. THE STRUCTURE INDICATED BY THE DRAWINGS AND SPECIFICATIONS IS STRUCTURALLY STABLE ONLY IN ITS COMPLETED FORM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, SEQUENCES AND OPERATIONS OF CONSTRUCTION AND SHALL PROVIDE TEMPORARY BRACING AS REQUIRED TO MAINTAIN THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION.
5. ALL DETAILS, SECTIONS, AND NOTES INDICATED ON THE DRAWINGS SHALL APPLY AT ALL LOCATIONS WHERE CONDITIONS ARE SIMILAR TO THOSE INDICATED BY THE DETAIL, SECTION, OR NOTE.
6. CENTERLINES OF COLUMNS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTIONS UNLESS NOTED OTHERWISE.
7. CENTERLINES OF FLOOR AND ROOF FRAMING MEMBERS SHALL COINCIDE WITH GRID LINES UNLESS NOTED OTHERWISE.
8. EQUALLY SPACE FLOOR AND ROOF FRAMING MEMBERS BETWEEN GRID LINES UNLESS NOTED OTHERWISE.
9. USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE THE DRAWINGS OR USE ANY DIMENSIONS TAKEN FROM ELECTRONIC DATA FILES.
10. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE STRUCTURAL WORK WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND ALL OTHER RELEVANT TRADES. IN CASE OF CONFLICT BETWEEN STRUCTURAL WORK AND DRAWINGS RELATED TO OTHER TRADES THE CONTRACTOR SHALL MAKE IN THEIR BID ALLOWANCE FOR THE MORE SEVERE REQUIREMENTS. CONFLICTS BETWEEN THE STRUCTURAL WORK AND THE DRAWINGS OF OTHER TRADES SHALL NOT BE A REASON FOR ANY ADDITIONAL COST OR DELAY IN EXECUTION OF THE WORK.
11. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES BETWEEN THE STRUCTURAL DOCUMENTS AND ANY OTHER DOCUMENTS OR EXISTING CONDITIONS FOR RESOLUTION PRIOR TO PROCEEDING WITH THE WORK.

ABBREVIATIONS

ADDL	ADDITIONAL	ELEC	ELECTRICAL	LSH	LONG SIDE HORIZONTAL
ADH	ADHESIVE	ELEV	ELEVATOR	LSV	LONG SIDE VERTICAL
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	EOD	EDGE OF DECK	LTS	TENSION LAP SPLICE LENGTH
		EOS	EDGE OF SLAB	LW	LIGHT WEIGHT
AFF	ABOVE FINISHED FLOOR	EQ	EQUAL	MAX	MAXIMUM
ALT	ALTERNATE	EQUIP	EQUIPMENT	MECH	MECHANICAL
ARCH	ARCHITECT	EW	EACH WAY	MFR	MANUFACTURER
B/	BOTTOM OF	EXP	EXPANSION	MIN	MINIMUM
BCB	BOTTOM CHORD BRACING	EXT	EXTERIOR	MOW	MIDDLE OF WALL
BCX	BOTTOM CHORD EXTENSION	EXIST; (E)	EXISTING	NS	NON-SHRINK
BFF	BELOW FINISHED FLOOR	FDN	FOUNDATION	NTS	NOT TO SCALE
BLDG	BUILDING	FFE	FINISHED FLOOR ELEVATION	NW	NORMAL WEIGHT
BOTT	BOTTOM	FIN	FINISHED	OC	ON CENTER
BP	BASE PL	FLR	FLOOR	OH	OPPOSITE HAND
BRG	BEARING	FOB	FACE OF BRICK	OPNG	OPENING
BTWN	BETWEEN	FOM	FACE OF MASONRY	PAF	POWDER/ POWER ACTUATED FASTENER
CIP	CAST IN PLACE	FOS	FACE OF STUD		
CJ	CONTRACTION OR CONSTRUCTION JOINT	FRTW	FIRE RETARDANT TREATED WOOD	PC	PRECAST or PILE CAP
				PCF	POUNDS PER CUBIC FOOT
CL	CENTERLINE	FTG	FOOTING	PJF	PRE-MOLDED JOINT FILLER
CLR	CLEAR	GA	GAGE	PL	PLATE
CMU	CONCRETE MASONRY UNIT	GALV	GALVANIZED	PLBG	PLUMBING
COL	COLUMN	GB	GRADE BEAM	PT	PRESSURE TREATED or POST
CONC	CONCRETE	GC	GENERAL CONTRACTOR		
CONN	CONNECTION	GLB	GLULAM BEAM	QTY	QUANTITY
CONT	CONTINUOUS	HD	HEADED	REINF	REINFORCEMENT
COORD	COORDINATE	HK	HOOKED	REF	REFERENCE
CTR	CENTER	HORIZ; H	HORIZONTAL	REQD	REQUIRED
DBA	DEFORMED BAR ANCHOR	INT	INTERIOR	SCHD	SCHEDULE
DBL	DOUBLE	JBE	JOIST BEARING ELEVATION	SIM	SIMILAR
DCJ	DOWELED CONSTRUCTION JOINT	JT	JOINT	SOG	SLAB ON GRADE
		KIPS	K	SPEC	SPECIFICATIONS
DEFL	DEFLECTION	KLF; PLF	KIPS/POUNDS PER LINEAR FOOT	STD	STANDARD
DEMO	DEMOLISH or DEMOLITION			T/	TOP OF
DIA; Ø	DIAMETER	KSI; PSI	KIPS/POUNDS PER SQUARE INCH	TCX	TOP CHORD EXTENSION
DIM	DIMENSION			TYP	TYPICAL
DWG	DRAWING	KSF; PSF	KIPS/POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
DWL	DOWEL			VERT; V	VERTICAL
EA	EACH	LB	POUND	VIF	VERFIY IN FIELD
EF	EACH FACE	LG	LONG	w/	WITH
EJ	EXPANSION JOINT	LLH	LONG LEG HORIZONTAL	WP	WORK POINT
EL	ELEVATION	LLV	LONG LEG VERTICAL	WWF	WELDED WIRE FABRIC

DESIGN CRITERIA

DESIGN CODES

1.	BUILDING CODE	2018 NORTH CAROLINA BUILDING CODE BASED ON THE 2015 INTERNATIONAL BUILDING CODE
2.	DESIGN LOADS	ASCE 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. (ALLOWED BY SCO)
3.	STEEL	AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
4.	CONCRETE	ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
5.	CONCRETE MASONRY	TMS 402/602-16 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
6.	COLD FORMED STEEL	AISI S100-16 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
7.	STEEL JOISTS	SJI-100-2015 STANDARD SPECIFICATIONS

DESIGN LOADS

1. BUILDING RISK CATEGORY II

2. SNOW LOAD — GROUND SNOW LOAD 10 PSF

I_s	1.0
C_t	1.1
C_e	0.9
P_f	7 PSF
P_s	7 PSF
RAIN-ON-SNOW SURCHARGE	5 PSF

3. LIVE LOAD

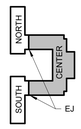
PARTITION ALLOWANCE	15 PSF
ROOF	20 PSF
DWELLING UNITS	40 PSF
ATTIC WALKWAYS	40 PSF
BATHROOMS	60 PSF
CORRIDORS & PUBLIC AREAS	100 PSF
MECH/ELECTRICAL/TELECOM	150 PSF
STAIRS	100 PSF
LIGHT WEIGHT STORAGE	125 PSF
SLAB-ON-GRADE	150 PSF
MECHANICAL PLATFORM	40 PSF + EQUIPMENT WEIGHT

4. SEISMIC LOAD

I_e	1.0
SITE CLASSIFICATION	D
S_S	0.152
S_1	0.067
S_{DS}	0.162
S_{D1}	0.107
SEISMIC DESIGN CATEGORY	B
SEISMIC FORCE RESISTING SYSTEM	INTERMEDIATE REINFORCED MASONRY SHEAR WALLS- BEARING WALLS SYSTEMS; R=3.5
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
SEISMIC BASE SHEAR NORTH BUILDING	120 KIPS
SEISMIC BASE SHEAR CENTER BUILDING	268 KIPS
SEISMIC BASE SHEAR SOUTH BUILDING	120 KIPS

5. WIND LOAD

WIND SPEED	V_{ult} = 144 MPH
EXPOSURE	C
K_d	0.85
K_{zt}	1.0
GC_{pi}	+/- 0.18
WIND BASE SHEAR NS NORTH BUILDING	121 KIPS
WIND BASE SHEAR EW NORTH BUILDING	374 KIPS
WIND BASE SHEAR NS CENTER BUILDING	396 KIPS
WIND BASE SHEAR EW CENTER BUILDING	499 KIPS
WIND BASE SHEAR NS SOUTH BUILDING	121 KIPS
WIND BASE SHEAR EW SOUTH BUILDING	374 KIPS



KEYPLAN

ULTIMATE WIND PRESSURE FOR ROOF COMPONENTS (PSF)

ROOF ZONE	COMPONENT TRIBUTARY AREA (FT²)			
	0 - 10	10 - 100	100 - 300	300+
NEGATIVE ZONE 1 & 2e	-86	-86	-65	-50
NEGATIVE ZONE 2n,2r & 3e	-137	-137	-81	-71
NEGATIVE ZONE 3r	-193	-160	-102	-102
ALL POSITIVE ZONES	+45	+37	+25	+25
OVERHANG ZONE	**COMPONENT TRIBUTARY AREA (FT²)**			
	0 - 10	**10 - 100**	**100 - 300**	**300+**
ZONE 1 & 2e	-103	-103	-97	-92
ZONE 2n & 2r	-154	-154	-119	-113
ZONE 3e	-184	-184	-102	-87
ZONE 3r	-240	-196	-118	-118

NOTES:
1. PRESSURES SHOWN ARE ULTIMATE LEVEL LOADS.
2. COMPONENTS AND CLADDING PRESSURES IN ACCORDANCE WITH ASCE 7-16.
3. BUILDING ZONES ARE DEFINED IN ASCE 7-16.
4. WIDTH OF EDGE AND CORNER ZONES 'a' = 20.10 FT.
5. USE WIND PRESSURES ABOVE FOR THE DESIGN OF COMPONENT AND CLADDING ELEMENTS OR RETAIN A PROFESSIONAL ENGINEER TO DETERMINE WIND PRESSURES FOR SPECIFIC EFFECTIVE WIND AREAS.
6. POSITIVE AND NEGATIVE SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES RESPECTIVELY.

ULTIMATE WIND PRESSURE FOR WALL COMPONENTS (PSF)

WALL ZONE	COMPONENT TRIBUTARY AREA (FT²)			
	10	100	200	500+
NEGATIVE ZONE 4	-66	-57	-54	-50
NEGATIVE ZONE 5	-81	-63	-58	-50
POSITIVE ZONE 4 & 5	+61	+52	+49	+45

ULTIMATE WIND PRESSURE FOR PARAPET (FRONT ENTRY) (PSF)

PARAPET ZONE	COMPONENT TRIBUTARY AREA (FT²)			
	10	100	200	300+
CASE A : ZONE 2e	+110	+84	+74	+68
CASE A : ZONE 2n, 2r & 3e	+154	+98	+87	+85
CASE A : ZONE 3r	+173	+115	+113	+112
CASE B : INTERIOR ZONE	-92	-77	-72	-70
CASE B : CORNER ZONE	-106	-82	-75	-71

DELEGATED DESIGN

THE FOLLOWING ITEMS SHALL BE DESIGNED BY A SPECIALTY ENGINEER FOR THE CONTRACTOR. DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW, SEALED AND SIGNED BY A STRUCTURAL ENGINEER LICENSED IN NORTH CAROLINA.

1. STEEL STAIRS, HANDRAILS, AND GUARDRAILS
2. NON-LOAD BEARING LIGHT GAGE METAL FRAMING
3. LIGHT GAGE METAL TRUSSES AND CONNECTIONS
4. STRUCTURAL STEEL CONNECTIONS

SOIL AND SUBSURFACE CONDITIONS

1. SOIL BEARING CAPACITY SHALL BE VERIFIED BY A GEOTECHNICAL ENGINEER LICENSED IN NORTH CAROLINA.
2. THE FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE REPORT OF GEOTECHNICAL EXPLORATION PREPARED BY ECS SOUTHEAST, LLC PROJECT NO. 22-36300 DATED JUNE 17,2025.
3. THE FOUNDATIONS HAVE BEEN DESIGNED BASED ON THE FOLLOWING DESIGN VALUES FROM THE GEOTECHNICAL REPORT:
   SPREAD FOOTING BEARING PRESSURE ON SOIL 3,000 PSF
4. THE CONTRACTOR SHALL VERIFY WITH THE GEOTECHNICAL ENGINEER THAT THE FOLLOWING ARE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT:
   A. THE BEARING STRATUM AT EACH FOUNDATION IS PER THE SPECIFICATIONS.
   B. THE ALLOWABLE BEARING PRESSURE MEETS OR EXCEEDS THE SPECIFICATIONS.
   C. ENGINEERED FILL IS INSTALLED IN ACCORDANCE WITH THE SPECIFICATIONS.
5. ALL FILL MATERIALS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. ALL FILL WITHIN 10' - 0" OF THE BUILDING FOUNDATION PERIMETER SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR, THE TOP 12" BELOW FLOOR SLABS AND FOOTINGS SHALL BE COMPACTED TO 98% OF STANDARD PROCTOR.
6. FOOTING BEARING ELEVATIONS SHALL BE ADJUSTED AT TIME OF EXCAVATION TO ACHIEVE THE REQUIRED BEARING CAPACITY IF SO REQUIRED.
7. BACKFILLING OF RETAINING WALLS SHALL BE PLACED SO THAT EQUAL LOADING IS MAINTAINED ON EACH SIDE OF WALL UNTIL THE LOWER GRADE IS REACHED.
8. PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING FOUNDATIONS BOTH DURING CONSTRUCTION AND PERMANENTLY.
9. MAINTAIN STABILITY OF EXCAVATIONS UNTIL PROPERLY BACKFILLED. KEEP EXCAVATIONS FREE OF LOOSE MATERIAL. REMOVE WATER ACCUMULATIONS AND REMOVE ANY UNSUITABLE MATERIAL PRIOR TO PLACING CONCRETE.
10. PLACE A 3" THICKNESS "MUDMAT" OF CONCRETE IN THE BOTTOM OF FOOTINGS THAT WILL BE EXPOSED TO RAIN OR LEFT OPEN OVER NIGHT.
11. HEAVY EQUIPMENT USED FOR PLACING OR COMPACTING BACKFILL SHALL NOT BE OPERATED WITHIN A DISTANCE EQUAL TO THE HEIGHT OF THE BACKFILL ABOVE THE TOP OF FOOTING, (1 HORIZONTAL TO 1 VERTICAL). HAND OPERATED COMPACTION EQUIPMENT SHALL BE USED FOR COMPACTION OPERATIONS IN THIS AREA.
12. GRADE SHALL BE SUCH THAT THE THICKNESS OF ANY FOUNDATION OR SLAB ON GRADE IS NOT REDUCED BY MORE THAN 5% OF THAT INDICATED.
13. EXCAVATION BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. EXCAVATION BRACING SHALL BE DESIGNED FOR LATERAL LOADING RESULTING FROM AN EQUIVALENT FLUID PRESSURE OF 60 PCF AND A SURFACE SURCHARGE OF 250 PSF.

CAST IN PLACE STRUCTURAL CONCRETE

1. SUBMIT MIX DESIGNS FOR EACH TYPE OF CONCRETE SPECIFIED.
2. SUBMIT DATA FOR ALL ADMIXTURES, CURING COMPOUNDS AND HARDENERS THAT ARE INTENDED FOR USE.
3. CONCRETE SHALL HAVE THE MINIMUM 28 DAY COMPRESSIVE STRENGTH AND WEIGHTS:

LOCATION	28 DAY STRENGTH	UNIT WEIGHT
FOUNDATIONS AND SLAB ON GRADE	3,500 PSI	145 PCF
ELEVATED SLAB ON DECK	4,000 PSI	145 PCF
STAIR INFILL	4,000 PSI	145 PCF

4. CONCRETE WORK SHALL CONFORM TO ACI 318.
5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60.
6. REINFORCING BARS TO BE WELDED SHALL CONFORM TO ASTM A706 GRADE 60.
7. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A82 AND A185. PROVIDE MATERIAL IN SHEETS. LAP ALL WELDED WIRE FABRIC ONE FULL SQUARE PLUS 2" AT ALL SHEET EDGES.
8. SLAB ON GRADE DOWELS SHALL BE SMOOTH RODS CONFORMING TO ASTM A36 WITH ENDS SMOOTH CUT.
9. REINFORCING BAR SUPPORTERS SHALL CONFORM TO CRSI MANUAL OF STANDARD PRACTICE.
10. CONCRETE CLEAR COVER ON EMBEDDED REINFORCING SHALL BE AS FOLLOWS:

LOCATION	BAR SIZE	MINIMUM CLEAR COVER
FOOTINGS	ALL	3" BOTTOM AND SIDES, 2" TOP
CONCRETE EXPOSED TO EARTH OR WEATHER	#5 AND SMALLER	1 1/2"
	#6 THROUGH #18	2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER SLABS	#11 AND SMALLER	3/4"

11. ALL CONTINUOUS BARS SHALL HAVE A CLASS B TENSION LAP SPLICE AT ALL SPLICES UNO. PROVIDE CORNER BARS FOR ALL CONTINUOUS BARS AT ALL FOUNDATION AND WALL CORNERS AND INTERSECTIONS. LAP CORNER BARS 48 BAR DIAMETERS EACH END.
12. PROVIDE (2) #5 x 4' - 0" LONG TOP DIAGONAL BARS AT ALL RE-ENTRANT CORNERS IN ALL SLABS ON GRADE AND ELEVATED SLABS.
13. PROVIDE DOWELS TO FOOTINGS TO MATCH ALL WALL, PIER AND COLUMN VERTICAL REINFORCING UNO. EMBED DOWELS IN FOOTING WITH HOOK TO WITHIN 3" OF BOTTOM OF FOOTING. EXTEND DOWELS ABOVE FOOTING FOR 48 BAR DIAMETER LAP SPLICE WITH VERTICAL REINFORCING UNO.
14. PROVIDE 1/2" PREMOLDED JOINT FILLER WHERE SLAB-ON-GRADE ABUT VERTICAL SURFACES
15. CONSTRUCTION OR CONTRACTION JOINTS SHALL BE INSTALLED IN SLABS ON GRADE AT A SPACING NOT TO EXCEED 12' - 0" ON CENTER EACH DIRECTION UNO ON FOUNDATION PLAN. ASPECT RATIO OF SLAB AREAS BETWEEN JOINTS (RATIO OF LONG SIDE TO SHORT SIDE) SHALL NOT EXCEED 1.5. SAW CUT JOINTS SHALL BE MADE AS SOON AS SLABS WILL SUPPORT WORKERS AND EQUIPMENT AND WITHIN 8 HOURS OF CONCRETE POUR. EMBEDDED EDGE ANGLES SHALL BE DISCONTINUOUS AT SLAB JOINT LOCATIONS.
16. CONFORM TO ACI 306 FOR COLD WEATHER CONCRETE AND ACI 305 FOR HOT WEATHER CONCRETE WORK WHEN ANY COMBINATION OF TEMPERATURE, HUMIDITY OR WIND SPEED RESULTS IN CONDITIONS THAT WOULD IMPAIR THE QUALITY OF CONCRETE. CONCRETE IS TO BE REJECTED IF ITS TEMPERATURE AT TIME OF PLACEMENT IS 95 DEGREES F OR ABOVE.
17. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNO. SEE ARCHITECTURAL DRAWINGS FOR DETAILS.
18. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL EMBEDDED ITEMS IN CONCRETE WORK. COORDINATE WITH THE FOLLOWING: CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS, PRECAST SHOP DRAWINGS, OTHER TRADES, ELECTRICAL AND PLUMBING EQUIPMENT AND FIXTURE REQUIREMENTS.
19. DO NOT PLACE ALUMINIUM CONDUITS, PIPES OR ACCESSORIES IN DIRECT CONTACT WITH CONRETE UNLESS COATED TO PREVENT ELECTROLYSIS.

CONCRETE MASONRY

1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY f'm = 2,000 PSI
2. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, MEDIUM WEIGHT (OVEN-DRY UNIT WEIGHT = 125 PCF, MAX), WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2,000 PSI FOR MORTAR TYPES M & S AND 2,650 PSI FOR MORTAR TYPE N.
3. MORTAR SHALL CONFORM TO ASTM C270 PORTLAND CEMENTLIME TYPE M OR S WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,800 PSI. AT ALL INTERIOR LOAD BEARING WALLS, TYPE N IS PERMITTED WITH A MINIMUM COMPRESSIVE STRENGTH OF 750 PSI.
4. COURSE MASONRY GROUT SHALL CONFORM TO ASTM C476 WITH MAXIMUM AGGREGATE SIZE OF 3/8". MINIMUM COMPRESSIVE STRENGTH SHALL BE 3,000 PSI AT 28 DAYS.
5. GROUT POURS SHALL NOT EXCEED 5' - 4" ABOVE THE CONSTRUCTION SURFACE OR PREVIOUSLY GROUTED MASONRY UNLESS 'HIGH LIFT' GROUT PROCEDURES ARE FOLLOWED & CLEAN OUTS PROVIDED.
6. ALL GROUT POURS SHALL STOP 1 1/2" BELOW THE TOP OF COURSE TO FORM A KEY AT POURED JOINTS.
7. CONCRETE MASONRY QUALITY CONTROL:
   A. WORK IN PROGRESS SHALL BE INSPECTED FOR CONFORMANCE WITH SPECIFIED MATERIALS AND THAT WORKMANSHIP AND CONSTRUCTION IS IN COMPLIANCE WITH PLANS, SPECIFICATIONS AND INDUSTRY STANDARDS.
   B. MORTAR: INSPECT PROPORTIONING OF MORTARS IN ACCORDANCE WITH ASTM C780. VERIFY ALL MATERIALS ARE AS APPROVED FOR THE PROJECT.
   C. GROUT: TEST 3"x3" PRISMS IN ACCORDANCE WITH ASTM C1019. TEST (2) PRISMS FOR EACH 30 CUBIC YARDS OR FRACTION THEREOF PLACED EACH DAY AND WHEN MIX PROPORTIONS ARE CHANGED.
8. PROVIDE THE FOLLOWING MINIMUM REINFORCING FOR ALL CONCRETE MASONRY WALLS UNO ON PLANS:
   A. PROVIDE W1.7 HORIZONTAL JOINT REINFORCEMENT AT 16" ON CENTER WITH FORMED "L" AND "T" SECTIONS AT WALL CORNERS AND INTERSECTIONS.
   B. REINFORCE VERTICALLY WITH (1) #5 CENTERED IN GROUT FILLED CELL FULL HEIGHT OF WALL WITH DOWEL TO FOUNDATION AT WALL CORNERS, ENDS, INTERSECTIONS, OPENING JAMBS, EACH SIDE OF CONTROL JOINTS AND SPACED AT 48" OC MAXIMUM. LAP VERTICAL REINFORCING PER LAP SPLICE SCHEDULE AT ALL SPLICES UNO.
   C. BOND BEAMS SHALL BE REINFORCED WITH (2) #5 CONTINUOUS. PROVIDE CORNER BARS AT ALL WALL CORNERS AND INTERSECTIONS. LAP REINFORCING PER LAP SPLICE SCHEDULE AT ALL SPLICES UNO.
9. MINIMUM MASONRY WALL FOOTINGS SHALL PROJECT 4" MINIMUM ON EACH SIDE OF WALL AND BE 12" DEEP WITH (2) #5 CONTINUOUS.
10. PROVIDE DOWELS TO MATCH VERTICAL REINFORCING AT THE BASE OF ALL WALLS. LAP DOWELS WITH VERTICAL REINFORCING PER LAP SPLICE SCHEDULE UNO.
11. ALL CELLS AND CAVITIES BELOW GRADE SHALL BE SOLID GROUTED.
12. LAP REINFORCING A MINIMUM OF 72 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
13. PLACE PIPES AND CONDUITS PASSING HORIZONTALLY THROUGH MASONRY IN SCHEDULE 40 STEEL SLEEVES FOR CIRCULAR OPENINGS LESS THAN 12" IN DIAMETER. ALL OTHER HORIZONTAL PENETRATIONS SHALL FOLLOW LINTEL SCHEDULE.
14. DO NOT PLACE ALUMINUM CONDUITS, PIPES, OR ACCESSORIES IN DIRECT CONTACT WITH MASONRY UNLESS COATED TO PREVENT ELECTROLYSIS.
15. PENETRATIONS THROUGH BOND BEAMS IS NOT PERMITTED.

STRUCTURAL STEEL

1. STRUCTURAL STEEL CONSTRUCTION DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
2. STRUCTURAL STEEL MEMBERS SHALL CONFORM TO THE FOLLOWING STANDARDS:

WIDE FLANGE SHAPES	ASTM A992
ANGLES, CHANNELS AND PLATES	ASTM A36
ANCHOR RODS ≤ 3/4"Ø	ASTM F1554 GRADE 36
ANCHOR RODS ≥ 7/8"Ø	ASTM F1554 GRADE 55
PIPE	ASTM A53
RECTANGULAR HSS	ASTM A500 GRADE C, 50 ksi
ROUND HSS	ASTM A500 GRADE C, 46 ksi
HEADED STUDS	ASTM A108, GRADE 1015-1020

3. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.
4. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD FOR THE LOCATION AND TYPE OF SPLICE.
5. CAMBER BEAMS WHERE INDICATED, WHERE NO CAMBER IS INDICATED, BEAMS SHALL BE FABRICATED SO THAT AFTER ERECTION, ANY NATURAL CAMBER IS UPWARD.
6. ALL COPES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES SHALL BE INDICATED ON THE SHOP DRAWINGS AT TIME OF SUBMITTAL FOR REVIEW.
7. FIELD MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
8. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL STEEL AND SHALL INDICATE COMPLETE CONNECTION INFORMATION, BOTH SHOP AND FIELD.
9. PROVIDE A SHOP COAT OF FABRICATOR'S STANDARD RUST INHIBITIVE PRIMER TO ALL STEEL UNO.
10. SEE ARCHITECTURAL DRAWINGS FOR FIRE PROTECTIVE MATERIAL APPLIED TO STRUCTURAL STEEL. DO NOT PRIME STEEL WHICH IS TO RECEIVE SPRAY APPLIED FIRE PROTECTIVE MATERIAL. DO PRIME STEEL WHICH IS TO RECEIVE INTUMESCENT FIRE PROTECTIVE COATING.
11. FILL SOLID WITH NON-SHRINK GROUT UNDER ALL BASE AND BEARING PLATES.
12. PROVIDE HEADED STUDS AT 12" OC MAXIMUM FOR ALL BEAMS SUPPORTING COMPOSITE FLOOR SYSTEMS UNO.
13. CONNECTION NOTES:
   A. STRUCTURAL STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR TO SUPPORT MEMBER REACTIONS INDICATED. REACTIONS INDICATED ARE FACTORED LOAD FORCES (LFRD) FOR APPLICABLE LOAD COMBINATIONS. WHERE NO REACTION IS INDICATED PROVIDE A CONNECTION DESIGNED TO SUPPORT A VERTICAL AND SHEAR REACTION OF 80% OF THE MAXIMUM TOTAL UNIFORM LOAD FOR THE APPROPRIATE BEAM SECTION AND SPAN AS DETERMINED PER THE MAXIMUM TOTAL UNIFORM LOAD TABLES IN THE AISC MANUAL.
   B. CONNECTION MATERIALS SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES:

ANGLES	ASTM A36
WT	ASTM A992
PLATES	ASTM A36
BOLTS	ASTM A325 OR ASTM A490
NUTS	ASTM A563
WASHERS	ASTM F436
WELDING ELECTRODES	E70XX

   C. STANDARD SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE ANGLE OR SINGLE PLATE CONNECTIONS IN ACCORDANCE WITH THE CONNECTION TABLES IN THE AISC "STEEL CONSTRUCTION MANUAL". BOLTED CONNECTIONS SHALL BE DETAILED USING TYPE N BOLTS INSTALLED IN SNUG TIGHTENED JOINTS UNO.
   D. BRACED FRAME CONNECTIONS, MOMENT CONNECTIONS AND COLLECTOR ELEMENT CONNECTIONS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN NORTH CAROLINA. STRUCTURAL CALCULATIONS FOR THESE CONNECTIONS SHALL BE SUBMITTED AND SHALL BE SEALED BY THE ENGINEER RESPONSIBLE FOR THE DESIGN.
   E. PROVIDE STIFFENERS, CONTINUITY PLATES, DOUBLER PLATES OR OTHER ADDITIONAL MEMBER LOCAL STRENGTHENING MEASURES AS REQUIRED FOR THE CONNECTION DESIGN.
   F. BOLTED CONNECTIONS SHALL BE MADE WITH A MINIMUM OF (2) 3/4" Ø BOLTS AND HAVE A MINIMUM SHEAR CAPACITY OF 10 KIPS.
   G. BOLTED CONNECTIONS SHALL CONFORM TO THE PROVISIONS OF THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
   H. WELDED CONNECTIONS SHALL BE MADE WITH CONTINUOUS FILLET WELDS UNO. MINIMUM WELD SIZE SHALL BE 1/4" OR AS REQUIRED BY AISC SPECIFICATION, WHICHEVER IS LARGER. MINIMUM WELD LENGTH SHALL BE 2".
   I. ALL WELDS SHALL BE MADE BY CERTIFIED WELDERS.
   J. BOLTED CONNECTIONS OF MOMENT CONNECTIONS, TENSION CONNECTIONS, BRACED FRAME CONNECTIONS, MOMENT FRAME CONNECTIONS, COLLECTOR ELEMENT CONNECTIONS AND AS INDICATED SHALL BE SLIP-CRITICAL.

LIGHT GAGE METAL FRAMING

1. LIGHT GAGE METAL FRAMING INDICATED ON THE DRAWINGS INDICATES TYPICAL CONDITIONS AND MINIMUM REQUIREMENTS.
2. NON-LOAD BEARING LIGHT GAGE METAL FRAMING SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN NORTH CAROLINA. DESIGN CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/STRUCTURAL ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE LAYOUT OF ALL LIGHT GAGE METAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK.
3. LIGHT GAGE METAL FRAMING DESIGN AND CONSTRUCTION SHALL CONFORM TO THE AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
4. MINIMUM GAGE OF MEMBERS PROVIDING LATERAL SUPPORT FOR MASONRY VENEER SHALL BE 18 GAGE (43 MILS). LIMIT LATERAL DEFLECTION OF STUDS PROVIDING LATERAL SUPPORT FOR MASONRY VENEER TO H/600.
5. MINIMUM YIELD STRENGTH (Fy) FOR LIGHT GAGE METAL FRAMING MEMBERS SHALL BE 33,000 PSI FOR 18 GAGE (43 MILS) AND THINNER. MINIMUM YIELD STRENGTH (

STATEMENT OF SPECIAL INSPECTIONS

Project:
Location:
Owner's Representative:
Owner's Address:

This Statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Special Inspection requirements of the 2018 North Carolina State Building Code. It includes a Schedule of Special Inspection Services applicable to this project, the name of the Special Inspector, the identity of other approved agencies retained for conducting Special Inspections, and the required inspector qualifications. This Statement of Special Inspections was prepared by the following Designers of Record:

Structural _____ (Type or print name) _____ (Signature) _____ (Date)
Architectural _____ (Type or print name) _____ (Signature) _____ (Date)
Mechanical _____ (Type or print name) _____ (Signature) _____ (Date)
Other _____ (Type or print name) _____ (Signature) _____ (Date)

The Special Inspector shall keep records of all special inspections and tests and shall furnish reports to the State Construction Office and the Designers of Record. Reports shall indicate if the work inspected or tested was or was not completed in conformance with the approved construction documents. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the State Construction Office and the Designers of Record. The Special Inspections program does not relieve the Contractor of his or her responsibilities.

Interim reports shall be submitted to the State Construction Office, Owner, and the Designers of Record.

Interim Report Frequency: Monthly

A Final Report of Special Inspections documenting completion of all required Special Inspections, testing, and correction of any discrepancies should be submitted prior to issuance of a Certificate of Use and Occupancy.

Job Site safety and means and methods of construction are solely the responsibility of the Contractor.

Owner's Authorization — Signature _____ Date _____

Accepted for the SCO by: — Signature _____ Date _____

SCHEDULE OF SPECIAL INSPECTION SERVICES [a]

The following sheets comprise the required schedule of special inspections for this project. The construction divisions which require special inspections for this project are as follows.

- ☑ Structural Steel & High Strength Bolting
- ☑ Welding of Structural Steel
- ☑ Cold-Formed Steel Deck
- ☑ Open-Web Steel Joists & Joist Girders
- ☑ Cold-Formed Steel Framing
- ☑ Concrete Construction
- ☑ Masonry Construction [b]
- ☐ Wood Construction
- ☑ Soils
- ☐ Driven Deep Foundations
- ☐ Cast-in-Place Deep Foundations
- ☐ Helical Pile Foundations
- ☐ Rammed Aggregate Pier & Stone Columns
- ☐ Sprayed Fire-Resistant Material
- ☐ Mastic & Intumescent Fire-Resistant Coatings
- ☐ Exterior Insulation & Finish System
- ☐ Fire-Resistant Penetrations & Joints
- ☐ Smoke Control
- ☐ Retaining Wall & Systems > 5 Feet
- ☑ Special Inspections for Wind Resistance
- ☐ Special Inspections for Seismic Resistance

a. The inspection frequency indicated on the following inspection tables are "C" continuous, "P" periodic, & "O" random on a daily basis.
b. Level A is the minimum inspection program for empirically / prescriptively designed masonry in Risk Category I, II or III structures. Level B is the minimum inspection program for empirically / prescriptively designed masonry in Risk Category IV structures and engineered masonry in Risk Category I, II or III structures. Level C is the minimum inspection program for engineered masonry in Risk Category IV structures. Engineered masonry structures are those designed in accordance with portions of the TMS 402-13 / ACI 530-13/ASCE 5-13 other than Part 4 or Appendix A.

Inspection Agents	Firm Name & Point of Contact	Address / Phone / E-mail
1. Special Inspector (SI-1)		
2. Testing Agency (TA-1)		
3. Testing Agency (TA-2)		
4. Geotechnical Engineer (GE-1)		
5. Other (O-1)		

Note: The inspection and testing agent(s) shall be engaged by the Owner or the Registered Design Professional of Record acting as the Owner's agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the State Construction Office, prior to commencing work.

Seismic Design Category: ☐ A ☑ B ☐ C ☐ D

Basic Wind Speed (V_{asd}): ☐ 90-109mph ☐ 110-119mph ☑ ≥120mph

Wind Exposure Category: ☐ B ☑ C ☐ D
☐

STRUCTURAL STEEL AND HIGH-STRENGTH BOLTING

Inspection Task	Task Req'd	Freq	Reference for Criteria: AISC 360	Reference for Criteria: NCBC	Agent
1. Fabricator Certification / Verification of Quality Control Procedures					
a. Verify fabricator qualifications	☐	C		1704.2.5.1	
b. Review material test reports & certifications	☐	C	N5.2		
c. Collect certificates of compliance from the steel fabricator at completion of fabrication	☐	C		1704.5	
2. Inspections Prior to High-Strength Bolting at Pretensioned and Slip-Critical Joints					
a. Collect manufacturer's certifications for fastener materials	☐	C	Table (Tbl) N5.6-1		
b. Fasteners are marked per ASTM requirements	☐	P	Tbl N5.6-1		
c. Ensure correct fasteners and bolting procedures are selected for joint details	☐	P	Tbl N5.6-1		
d. Verify connecting elements, including the appropriate faying surface condition and hole preparation when specified, comply with the construction documents	☐	P	Tbl N5.6-1		
e. Observe and document pre-installation verification testing by installation personal for fastener assemblies and methods	☐	P	Tbl N5.6-1		
f. Verify proper storage provided for all fastener components	☐	P	Tbl N5.6-1		
3. Inspections During High-Strength Bolting at Pretensioned and Slip-Critical Joints					
a. Ensure correct fastener assemblies placed in all holes and washers, when specified, are positioned as required	☐	P	Tbl N5.6-2		
b. Verify joint brought to snug-tight condition prior to pretensioning	☐	P	Tbl N5.6-2		
c. Verify fastener components not turned by the wrench prevented from rotating	☐	P	Tbl N5.6-2		
d. Ensure fasteners are pretensioned in accordance with RCSC, progressing from the most rigid point towards free edges	☐	P	Tbl N5.6-2		
4. Document acceptance or rejection of bolted connections after high-strength bolting is complete	☐	C	Tbl N5.6-3		
5. Structural Details					
a. Verify diameter, grade, type and length of anchor rods and other embedded items supporting structural steel	☐	P	N5.7		
b. Inspection of fabricated assemblies & erected steel framing verifying compliance with the construction documents	☐	P	N5.7		
6. Composite Construction					
a. Verify placement & installation of steel deck	☐	P	Tbl N6.1		
b. Observe placement and installation of steel headed stud anchors	☐	P	Tbl N6.1		
c. Document acceptance or rejection of composite construction elements	☐	P	Tbl N6.1		

MASONRY - LEVEL B

Inspection Task	Task Req'd	Freq	Reference for Criteria: TMS 402 [a]	Reference for Criteria: TMS 602 [a]	Agent
1. Test & verify slump flow & visual stability index as delivered to site for self-consolidating grout	☐	C	Table (Tbl) 3.1.2	Art. 1.5B.1.b.3	
2. Test & verify f'_m & f'_{AAC} prior to construction	☐	C	Tbl 3.1.2	Art. 1.4B	
3. Verify compliance with the approved submittals	☐	P	Tbl 3.1.2	Art. 1.5	
4. As masonry construction begins, verify that the following are in compliance:					
a. Proportions of site-prepared mortar	☐	P		Art. 2.1, 2.6A	
b. Construction of mortar joints	☐	P		Art. 3.3B	
c. Grade and size of prestressing tendons and anchorages	☐	P		Art. 2.4B, 2.4H	
d. Location of reinforcement, connectors and prestressing tendons and anchorages	☐	P		Art. 3.4, 3.6A	
e. Prestressing technique	☐	P		Art. 3.6B	
f. Properties of thin-bed mortar at AAC masonry	☐	C / P_b		Art. 2.1C	
5. Prior to grouting, verify that the following comply:					
a. Grout space is clean, and cleanouts provided when required	☐	P		Art. 3.2D, 3.2F	
b. Grade, type & size of reinforcement & anchor bolts, & prestressing tendons & anchorage	☐	P	Sec. 6.1	Art. 2.4, 3.4	
c. Placement of reinforcement, connectors, and prestressing tendons and anchorage	☐	P	Sec. 6.1, 6.2.1, 6.2.6, 6.2.7	Art.3.2E, 3.4, 3.6A	
d. Proportions of site-prepared grout and prestressing grout for bonded tendons	☐	P		Art. 2.6B, 2.4G.1.b	
e. Construction and size of mortar joints	☐	P		Art. 3.3B	
6. Verify during construction:					
a. Size and location of structural elements	☐	P		Art. 3.3F	
b. Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction	☐	P	Sec. 1.2.1(e), 6.1.4.3, 6.2.1		
c. Welding of reinforcement	☐	C	Sec. 8.1.6.7.2, 9.3.3.4(c), 11.3.3.4(b)		
d. Preparation, construction, and protection of masonry during cold weather (temperature < 40°F) or hot weather (temperature > 90°F)	☐	P		Art. 1.8C, 1.8D	
e. Application & measurement of prestress force	☐	C		Art. 3.6B	
f. Verify placement of grout and prestressing grout for bonded tendons	☐	C		Art. 3.5, 3.6C	
g. Placement of AAC masonry units and construction of thin-bed mortar joints	☐	C / P_b		Art. 3.3B.9, 3.3F.1.b	
7. Observe preparation of grout specimens, mortar specimens, and or prisms	☐	P		Art. 1.4.B.2.a.3, 1.4.B.2.b.3, 1.4.B.2.c.3, 1.4.B.3, 1.4.B.4	

a. References to "TMS402" in this table are to the TMS402/ACI530/ASCE5-13. References to "TMS602" are to TMS602/ACI530.1/ASCE6-13.
b. AAC masonry shall be continuously inspected for the first 5000-square feet and periodically inspected afterwards.

SPECIAL INSPECTIONS FOR WIND RESISTANCE

Inspection Task	Task Req'd	Freq	Reference for Criteria: STANDARD	Reference for Criteria: NCBC	Agent
1. Prior to any work taking place, each contractor responsible for the construction of a wind-resisting system or component shall submit a written statement of contractor responsibility	☒	C		1704.4	
2. Structural Wood					
a. Verify field gluing operations pertinent to the main wind force-resisting system	☐	C		1705.11.1	
b. Inspect nailing, anchoring, and fastening of components within the main windforce-resisting system including shear walls, diaphragms, drag struts, braces & hold-downs	☐	P		1705.11.1	
3. Cold-Formed Steel Light Frame Construction					
a. Inspect welding operations at elements of the main windforce-resisting system	☒	P		1705.11.2	
b. Inspect screw attachment, bolting, anchoring, and fastening of elements within the main windforce-resisting system including shear walls, braces, diaphragms collectors, drag struts and hold-downs	☒	P		1705.11.2	
4. Wind-resisting components					
a. Inspect the fastening of roof covering, roof deck and supporting roof framing connections	☒	P		1705.11.3.1	
b. Inspect the fastening of exterior wall coverings & the wall connections to the roof / floor diaphragms & framing members	☒	P		1705.11.3.2	

Structural Wood and Cold-Formed Steel Light-Frame Construction Main Wind-Force Resisting System(s) Subject to Special Inspections:
ROOF TRUSS ATTACHMENTS AT SHEAR WALLS, ROOF TRUSS DRAG STRUTS, DIAPHRAGMS

Roof Cladding Components and Connections Subject to Special Inspections:

Wall Cladding Components and Connections Subject to Special Inspections:

SOILS

Inspection Task	Task Req'd	Freq	Reference for Criteria: STANDARD	Reference for Criteria: NCBC	Agent
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity	☐	P		1705.6	
2. Verify excavations extend to proper depth and have reached the correct soil material	☐	P		1705.6	
3. Perform classification and testing of compacted fill materials	☐	P		1705.6	
4. Verify that materials used, densities, lift thickness and procedures used during placement and compaction of compacted fill are in accordance with the approved soils report and the construction documents	☐	C		1705.6	
5. Prior to placement of compacted fill, verify that the subgrade has been prepared in accordance with the approved soils report and the construction documents	☐	P		1705.6	

WELDING OF STRUCTURAL STEEL

Inspection Task	Task Req'd	Freq	Reference for Criteria: AISC 360	Reference for Criteria: NCBC	Agent
1. Inspections Prior to Welding			N5.4		
a. Collect & review welding procedure specification (WPS) and verify manufacturer certifications for welding consumables	☐	C	Table (Tbl) N5.4-1		
b. Confirm weld material type & grade	☐	P	Tbl N5.4-1		
c. Confirm method of welder identification	☐	P	Tbl N5.4-1		
d. Inspection of fit-up for groove & fillet welds including access hole configuration & finish	☐	P	Tbl N5.4-1		
2. Inspections During Welding			N5.4		
a. Verify welder qualifications	☐	P	Tbl N5.4-2		
b. Verify proper control and handling of welding consumables	☐	P	Tbl N5.4-2		
c. Monitor environmental conditions	☐	P	Tbl N5.4-2		
d. Monitor proper implementation of WPS	☐	P	Tbl N5.4-2		
e. Inspection of welding techniques including no welding over cracked tack welds	☐	P	Tbl N5.4-2		
3. Inspections After Welding			N5.4, N5.5		
a. Verify welds have been cleaned	☐	P	Tbl N5.4-3		
b. Confirm the installed size, length and location of welds matches the contract documents	☐	C	Tbl N5.4-3		
c. Verify welds meet visual acceptance criteria	☐	C	Tbl N5.4-3		
d. Confirm arc strikes comply with Part 5.28 of AWS D1.1	☐	C	Tbl N5.4-3		
e. Visually observe web k-area for cracks within 3" of welded doubler plates, continuity plates and stiffeners	☐	C	Tbl N5.4-3		
f. Backing and weld tabs removed per contract documents	☐	C	Tbl N5.4-3		
g. Observe and inspect weld repair activities	☐	C	Tbl N5.4-3		
h. For Risk Category III or IV structures, conduct ultrasonic testing (UT) of CJP groove welds in materials ≥ 5/16" at butt, T- and corner joints subject to transversely applied tension loading	☐	C	N.5.5b, N5.5e		
i. For Risk Category II structures, conduct ultrasonic testing (UT) of CJP groove welds in materials ≥ 5/16" at butt, T- and corner joints subject to transversely applied tension loading	☐	P	N.5.5b, N5.5f		
j. Conduct magnetic particle testing (MT) or liquid penetrant testing (PT) at thermally cut surfaces of access holes for rolled section with t_f > 2" and built-up shape with t_w > 2"	☐	C	N5.5c		
k. Radiographic or ultrasonic inspection at joints subject to fatigue	☐	C	N5.5d,Tbl A-3.1		
l. Document acceptance / rejection of welded joints and members	☐	C	Tbl N5.4-3, N5.5g		

COLD-FORMED STEEL DECK

Inspection Task	Task Req'd	Freq	Reference for Criteria: SDI QA/QC	Reference for Criteria: NCBC	Agent
1. Prior to deck placement, verify deck and deck accessories comply with the construction documents	☐	C	Table (Tbl) 1.1		
2. Inspection Tasks After Deck Placement					
a. Verify the installation of deck & deck accessories complies with the construction documents	☐	C	Tbl 1.2		
b. Verify that deck materials' mill certifications comply with the construction documents	☐	C	Tbl 1.2		
3. Inspection Tasks Prior to Deck Welding					
a. Collect welding procedure specification (WPS)	☐	P	Tbl 1.3		
b. Collect manufacturer certifications for welding consumables	☐	P	Tbl 1.3		
c. Verify material type and grade	☐	P	Tbl 1.3		
d. Check welding equipment	☐	P	Tbl 1.3		
4. Inspection Tasks During Deck Welding					
a. Verify welder qualifications	☐	P	Tbl 1.4		
b. Verify proper control and handling of welding consumables	☐	P	Tbl 1.4		
c. Monitor environmental conditions	☐	P	Tbl 1.4		
d. Monitor proper implementation of WPS	☐	P	Tbl 1.4		
5. Inspection Tasks After Welding					
a. Verify size and location of welds, including support, sidelap and perimeter welds	☐	C	Tbl 1.5		
b. Verify welds meet visual acceptance criteria	☐	C	Tbl 1.5		
c. Observe weld repair activities	☐	C	Tbl 1.5		
6. Inspection Tasks Prior to Mechanical Fastening					
a. Verify manufacturer installation instructions available for mechanical fasteners	☐	P	Tbl 1.6		
b. Proper tools available for fastener installation	☐	P	Tbl 1.6		
c. Verify proper storage of mechanical fasteners	☐	P	Tbl 1.6		
7. Inspection Tasks During Mechanical Fastening					
a. Observe fastener spacing and position	☐	P	Tbl 1.7		
b. Verify fasteners are installed in accordance with manufacturer's instructions	☐	P	Tbl 1.7		
8. Inspection Tasks After Mechanical Fastening					
a. Check spacing, type and installation of support fasteners	☐	C	Tbl 1.8		
b. Check spacing, type, and installation of sidelap fasteners	☐	C	Tbl 1.8		
c. Check spacing, type, and installation of perimeter fasteners	☐	C	Tbl 1.8		
d. Verify repair activities	☐	C	Tbl 1.8		
9. Document acceptance or rejection of deck & deck accessories for all phases of construction	☐	C	Tbls 1.1 thru 1.8		

COLD-FORMED STEEL FRAMING

Inspection Task	Task Req'd	Freq	Reference for Criteria: STANDARD	Reference for Criteria: NCBC	Agent
1. Fabricator Certification / Verification of Quality Control Procedures					
a. Verify fabricator qualifications	☒	C		1704.2.5.1	
b. Collect certificates of compliance from the steel fabricator at completion of fabrication	☒	C		1704.5	
2. For trusses clear spanning 60 feet or more, verify that both temporary and permanent restraints and braces are installed in accordance with the approved truss submittal package.	☒	P		1705.2.4	

CONCRETE CONSTRUCTION

Inspection Task	Task Req'd	Freq	Reference for Criteria: STANDARD [a]	Reference for Criteria: NCBC	Agent
1. Inspect reinforcement, including prestressing tendons, and verify placement	☐	P	ACI Ch.20, 25.2, 25.3, 26.6.1-26.6.3	1908.4	
2. Reinforcing Bar Welding:			AWS D1.4		
a. Verify weldability of reinforcing bars other than ASTM A706 and collect reports	☐	P	ACI 26.6.4	1704.5	
b. Inspect single-pass fillet welds ≤ 5/16"	☐	P	ACI 26.6.4		
c. Inspect all welds other than single-pass fillet welds ≤ 5/16"	☐	C	ACI 26.6.4		
3. Concrete Anchors:					
a. Inspect anchors cast in concrete	☐	P	ACI 17.8.2		
b. Inspect adhesive anchors installed in hardened concrete with horizontally or upwardly inclined orientations that resist sustained tension loads	☐	C	ACI 17.8.2.4		
c. Inspect adhesive anchors installed in hardened concrete with orientations different from Item 3.b	☐	P	ACI 17.8.2		
d. Inspect mechanical anchors installed in hardened concrete	☐	P	ACI 17.8.2		
4. Collect mix designs and verify the correct mix used during installation	☐	P	ACI Ch19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3	
5. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete	☐	C	ASTM C172, ASTM C31, ACI 26.4, 26.12	1908.10	
6. Inspect concrete and shotcrete placement for proper application techniques	☐	C	ACI 26.5	1908.6, 1908.7, 1908.8	
7. Collect reports of preconstruction tests for shotcrete when preconstruction tests are required by NCBC Section 1908.4	☐	C		1704.5, 1908.5	
8. Verify maintenance of specified curing temperature and techniques	☐	P	ACI 26.5.3-26.5.5	1908.9	
9. Inspections for prestressed concrete					
a. Observe application of prestressing force	☐	C	ACI 26.10		
b. Inspect grouting of bonded prestressing tendons	☐	C	ACI 26.10		
10. Verify concrete strength prior to stressing of PT tendons and prior to removal of shores and forms from PT & mild beams and structural slabs	☐	P	ACI 26.11.2		
11. Inspect erection of precast members	☐	P	ACI 26.8		
12. Inspect formwork for shape, location and dimensions of the concrete member being formed	☐	P	ACI 26.11.1.2(b)		
13. Collect mill test reports for ASTM A615 rebar used by SFRS special moment frames, special structural walls or coupling beams	☐	C	ACI 20.2.2.5	1704.5	

a. References to "ACI" in this table are to the ACI 318-14.

LITTLE
DIVERSIFIED ARCHITECTURAL CONSULTING
410 Blackwell Street, Suite 10
Durham, NC 27701
(919) 474-2500
www.littleonline.com

ISSUED FOR BP-05D BID PACKAGE

CORPORATE LICENSE F-0467

NORTH CAROLINA PROFESSIONAL SEAL 42006 ENGINEER CARL SCHUETT

10/06/2025

SCO #24-29053-02A

ISSUE FOR
EARLY PROCUREMENT PACKAGES

ISSUE DATE
10.06.2025

REVISIONS

NO.	REASON	DATE

PROJECT TEAM
PRINCIPAL IN CHARGE
WILLIAM STEWART, AIA
PROJECT MANAGER
ERIC SCHOENAGEL, AIA
DESIGN TEAM
CARL SCHUETT, PE

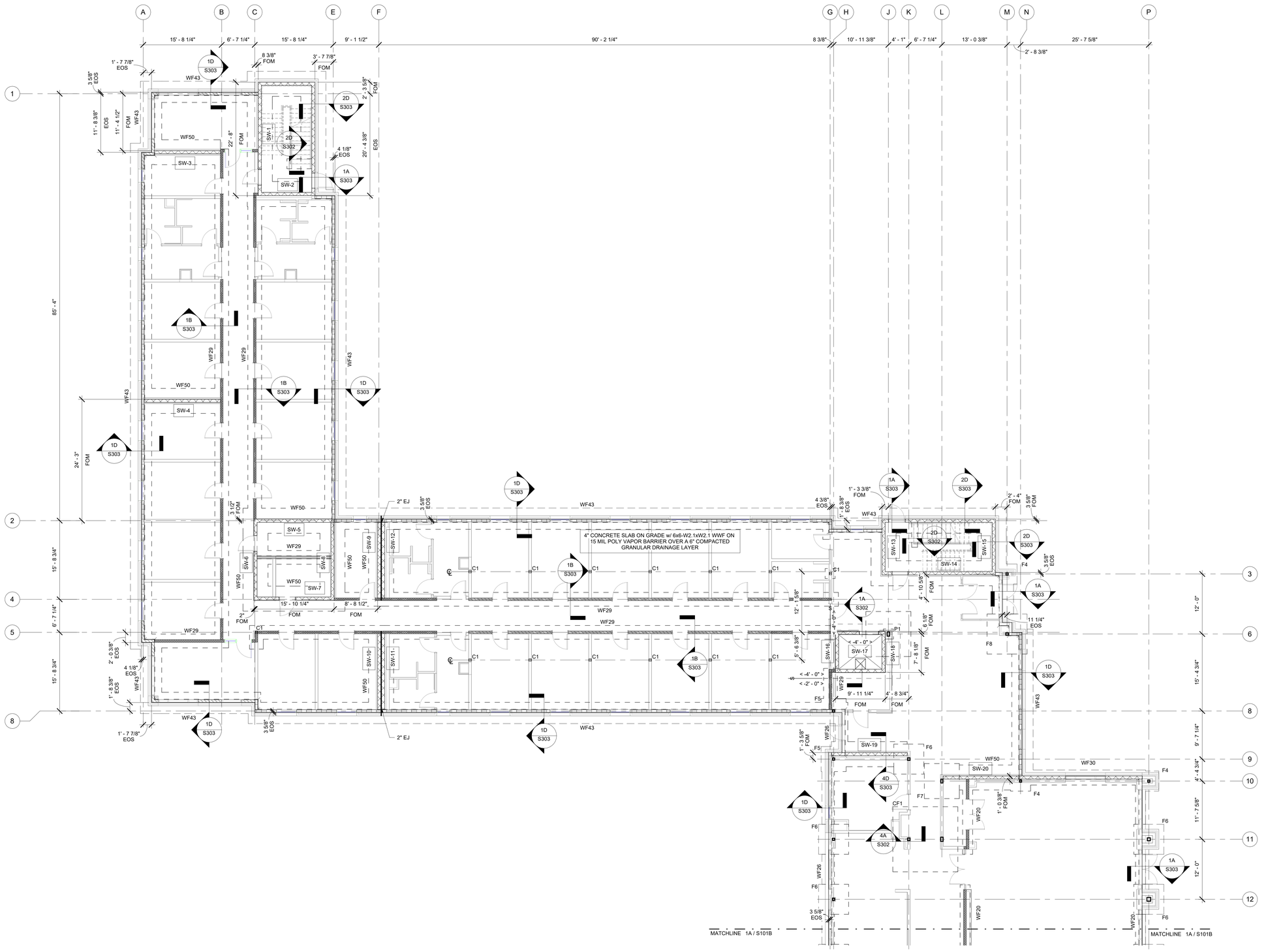
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
SPECIAL INSPECTIONS

SHEET NUMBER
S002

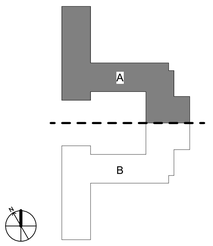


LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



NOT TO SCALE

1A / S101A FOUNDATION PLAN - REGION A

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISHED FLOOR ELEVATION 44.5', UNO. REFERENCE ELEVATION 0' - 0", DATUM.
3. TOP OF FOOTING 2' - 0" BELOW FINISHED FLOOR ELEVATION, UNO. <No> INDICATES TOP OF FOOTING ELEVATION, SEE PLAN.
4. -#- INDICATES STEP IN WALL FOOTING, SEE 1D / S302
5. "F#" INDICATES FOOTING TYPE, SEE S301
6. "WF#" INDICATES WALL FOOTING TYPE, SEE S301
7. "P#" INDICATES CONCRETE PEDESTAL TYPE, SEE S301
8. TOP OF PEDESTAL ELEVATION 0' - 8" BELOW FINISHED FLOOR ELEVATION,UNO. [No] INDICATES TOP OF PEDESTAL ELEVATION, SEE PLAN
9. SEE S301 FOR COLUMN SCHEDULE.
10. SEE S301 FOR TYPICAL SLAB CONSTRUCTION DETAILS.
11. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
12. SEE CIVIL FOR EXTERIOR PAVEMENT.
13. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
14. "ML#" INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
15. C# INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

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Durham, NC 27701
(919) 474-2500
www.littleonline.com

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CORPORATE LICENSE F-0467

NORTH CAROLINA PROFESSIONAL SEAL 47008 ENGINEER CARL SCHUETT

10/06/2025

SCO #24-29053-02A

ISSUE FOR
EARLY PROCUREMENT PACKAGES

ISSUE DATE
10.06.2025

REVISIONS

NO.	REASON	DATE

PROJECT TEAM
PRINCIPAL IN CHARGE
WILLIAM STEWART, AIA
PROJECT MANAGER
ERIC SCHOENAGEL, AIA
DESIGN TEAM
CARL SCHUETT, PE

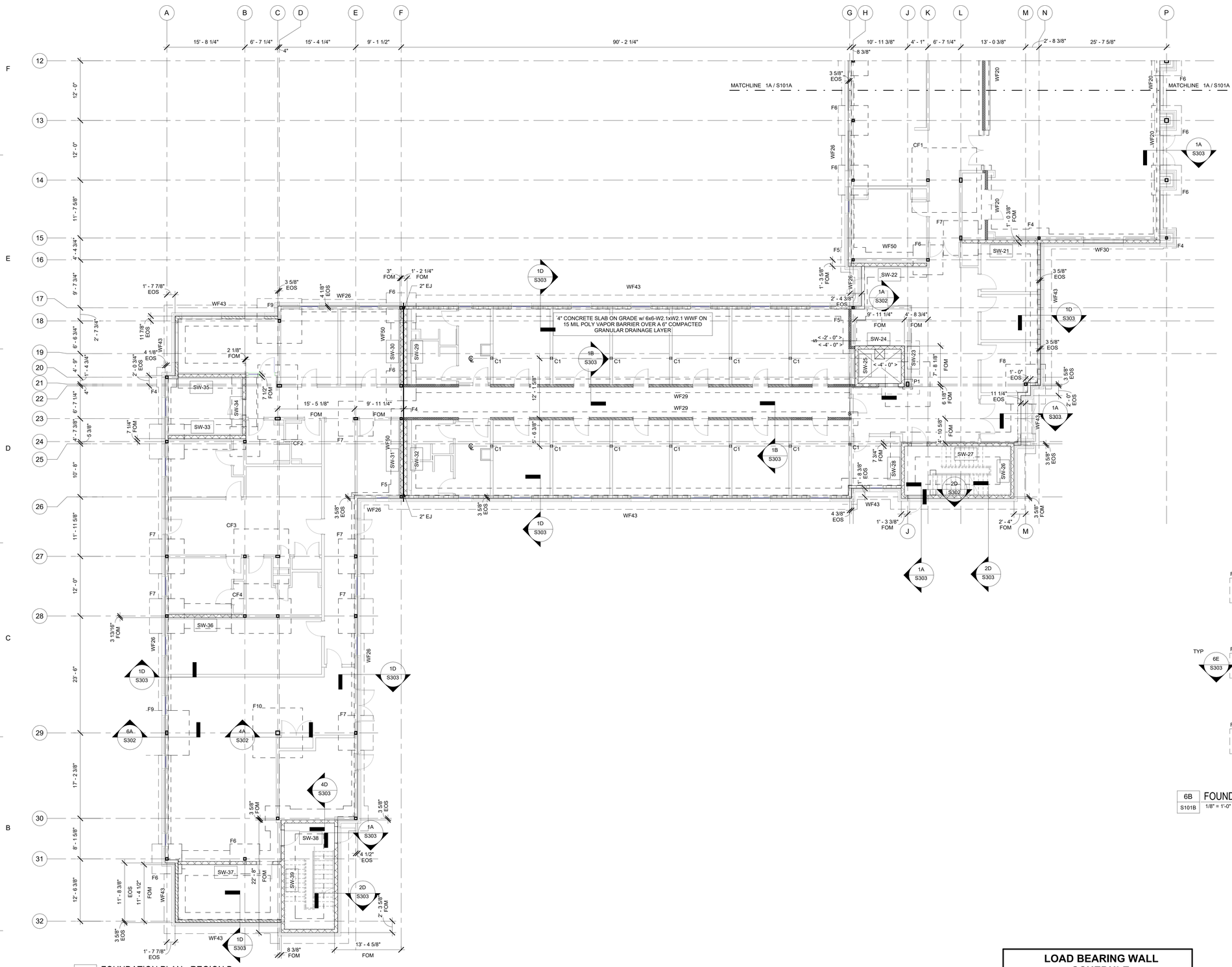
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
FOUNDATION PLAN - REGION A

SHEET NUMBER
S101A

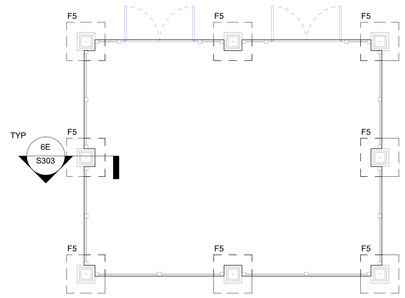


1A S101B FOUNDATION PLAN - REGION B

1/8" = 1'-0"

NOTES:

1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISHED FLOOR ELEVATION 44.5', UNO. REFERENCE ELEVATION 0' - 0", DATUM.
3. TOP OF FOOTING 2' - 0" BELOW FINISHED FLOOR ELEVATION, UNO. <No> INDICATES TOP OF FOOTING ELEVATION, SEE PLAN.
4. ~~0~~ INDICATES STEP IN WALL FOOTING, SEE 1D / S302
5. "F#" INDICATES FOOTING TYPE, SEE S301
6. "WF#" INDICATES WALL FOOTING TYPE, SEE S301
7. "P#" INDICATES CONCRETE PEDESTAL TYPE, SEE S301
8. TOP OF PEDESTAL ELEVATION 0' - 8" BELOW FINISHED FLOOR ELEVATION,UNO. [No] INDICATES TOP OF PEDESTAL ELEVATION, SEE PLAN
9. SEE S301 FOR COLUMN SCHEDULE.
10. SEE S301 FOR TYPICAL SLAB CONSTRUCTION DETAILS.
11. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
12. SEE CIVIL FOR EXTERIOR PAVEMENT.
13. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
14. "ML#" INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
15. C# INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502



6B S101B FOUNDATION PLAN - COOLING TOWER

1/8" = 1'-0"

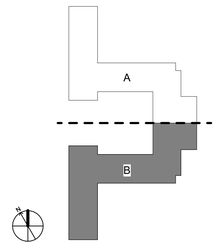
LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:

1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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NORTH CAROLINA PROFESSIONAL SEAL 47008 ENGINEER CARL SCHUETT

10/06/2025

SCO #24-29053-02A

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PROJECT TEAM

PRINCIPAL IN CHARGE: WILLIAM STEWART, AIA

PROJECT MANAGER: ERIC SCHOENAGEL, AIA

DESIGN TEAM: CARL SCHUETT, PE

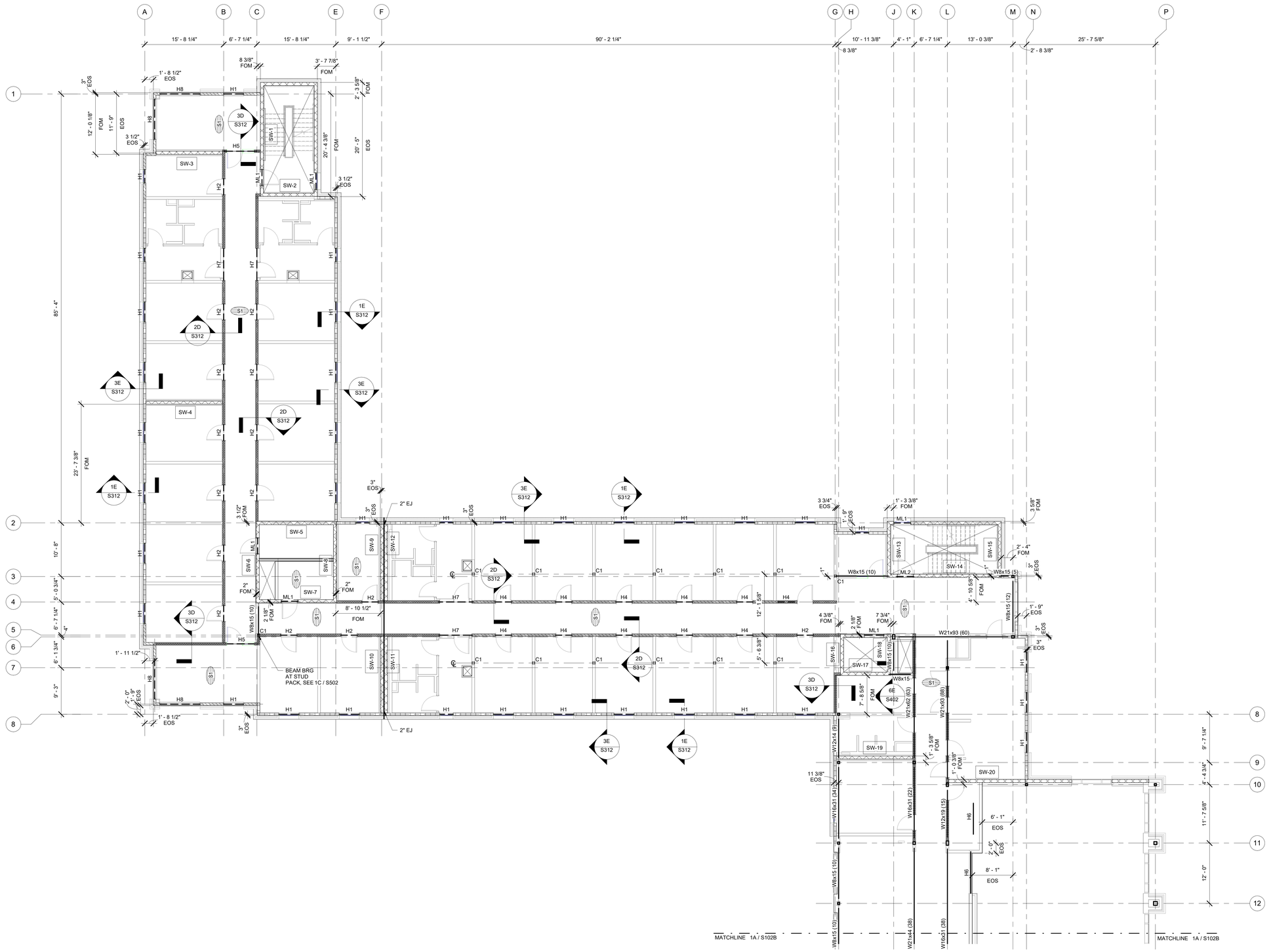
PROJECT NAME: UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.: 514.23018.00

SHEET TITLE: FOUNDATION PLAN - REGION B

SHEET NUMBER: S101B

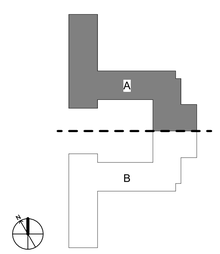


LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



NOT TO SCALE

1A / S102A

LEVEL 02 FRAMING PLAN- REGION A

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +13' - 4" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" - INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
9. "ML#" - INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF. SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

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NORTH CAROLINA PROFESSIONAL SEAL 47006 ENGINEER CARL SCHUETT

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PROJECT NAME
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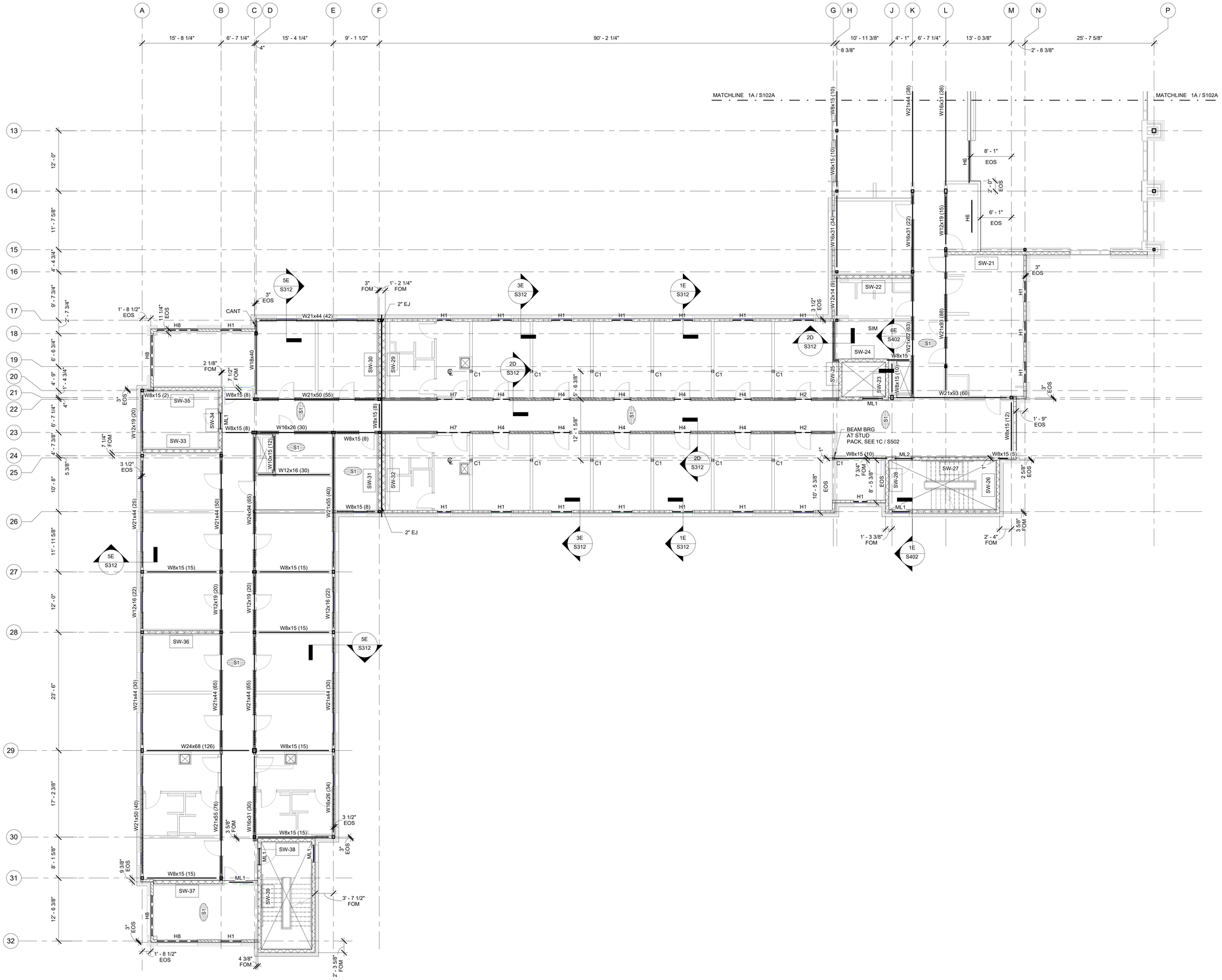
PROJECT NO.
514.23018.00

SHEET TITLE
LEVEL 02 FRAMING PLAN - REGION A

SHEET NUMBER
S102A



1A / S102B LEVEL 02 FRAMING PLAN - REGION B

1/8" = 1'-0"

NOTES:

1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +13' - 4" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL. SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" - INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
9. "ML#" - INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

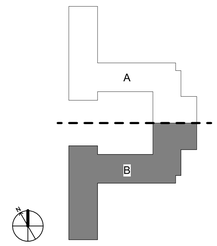
S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF, SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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NORTH CAROLINA PROFESSIONAL SEAL 47006 ENGINEER CARL SCHUETT

10/06/2025

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DESIGN TEAM
CARL SCHUETT, PE

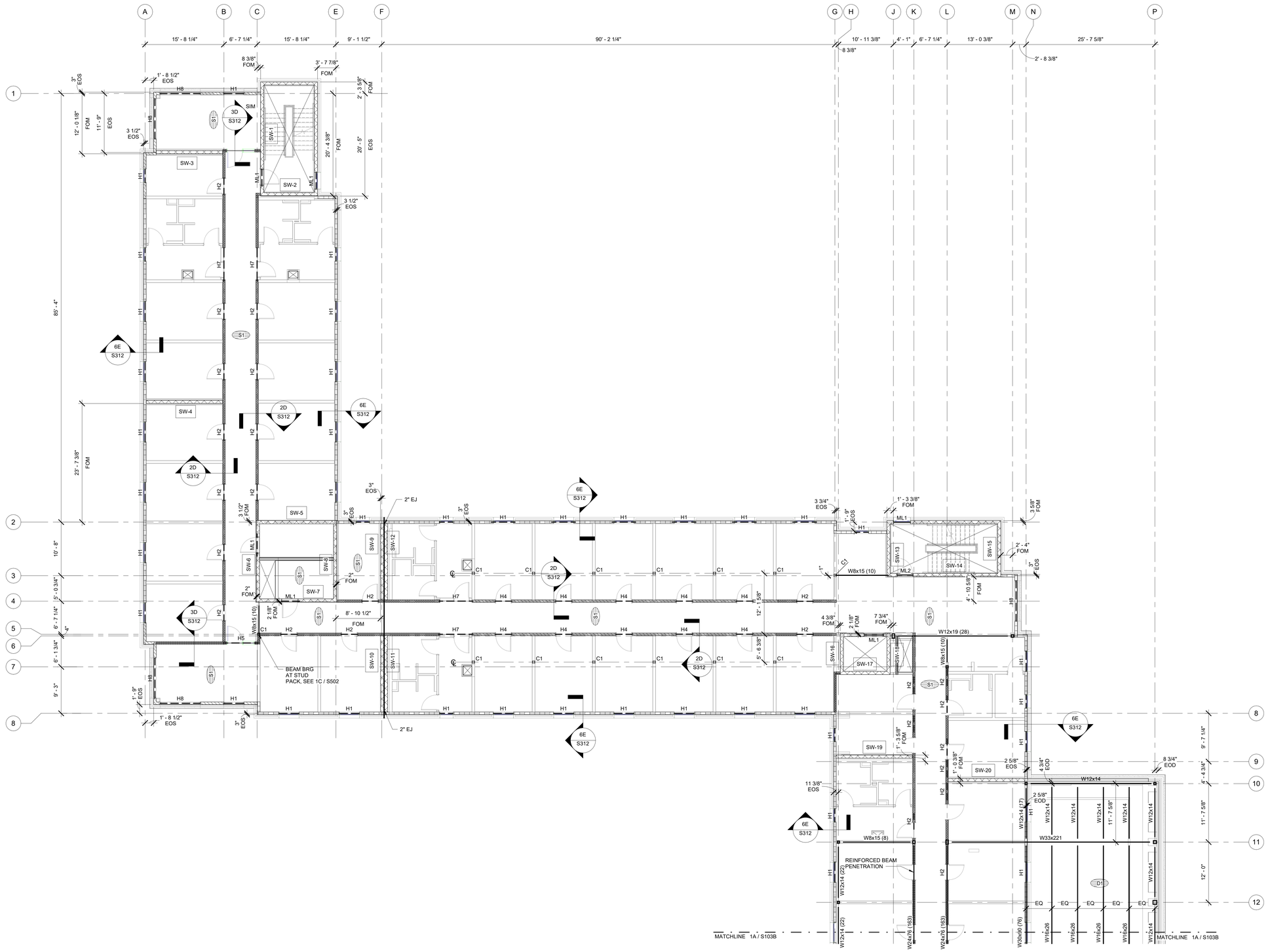
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
LEVEL 02 FRAMING PLAN - REGION B

SHEET NUMBER
S102B

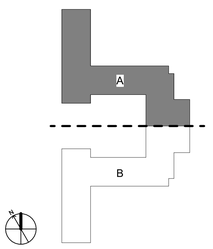


LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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1A / S103A LEVEL 03 FRAMING PLAN - REGION A

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +24' - 8" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
9. "ML#" INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF. SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

D1 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 20 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

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DESIGN TEAM: CARL SCHUETT, PE

PROJECT NAME: UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.: 514.23018.00

SHEET TITLE: LEVEL 03 FRAMING PLAN - REGION A

SHEET NUMBER: S103A

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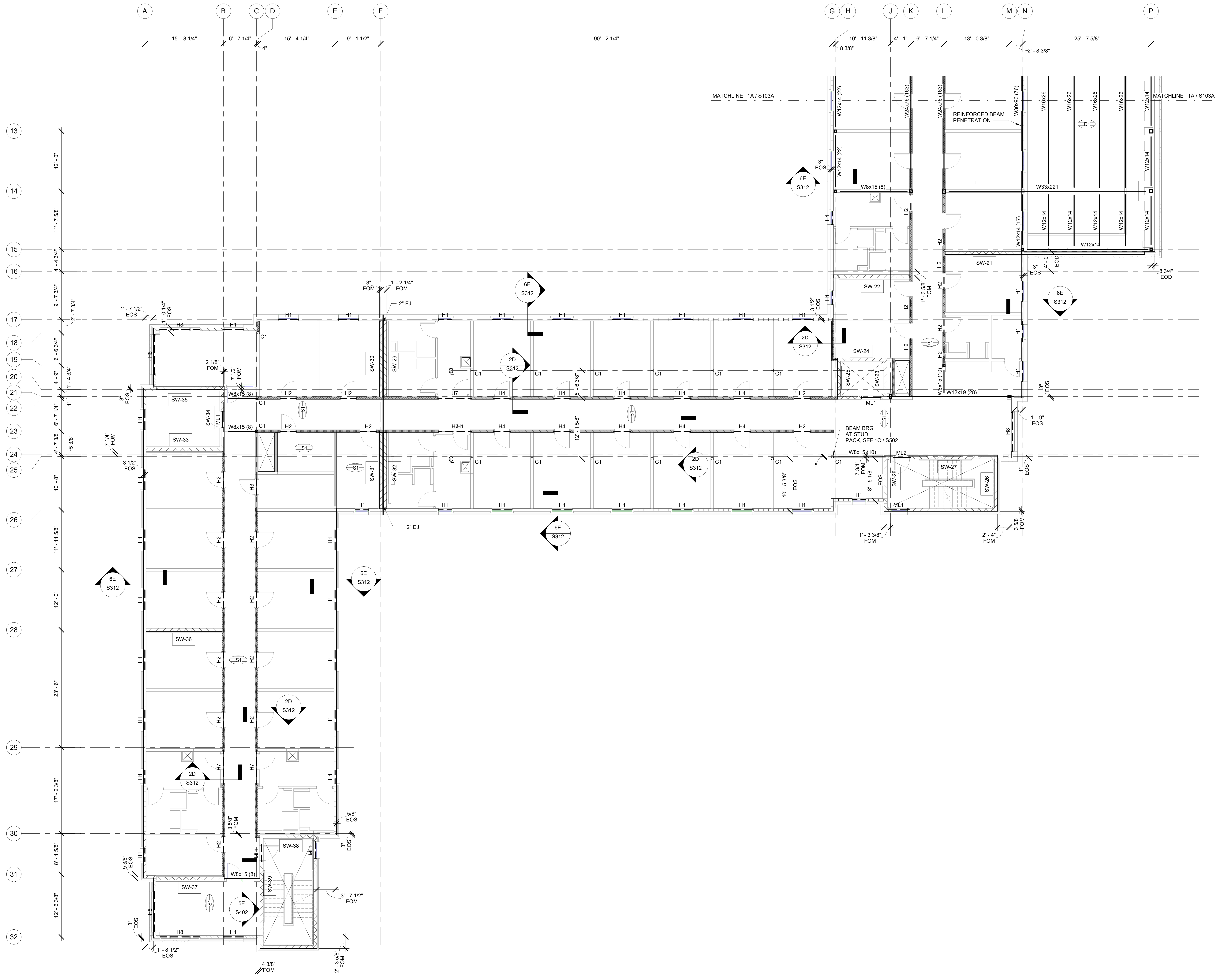


1A LEVEL 03 FRAMING PLAN - REGION B

S103B
1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +24' - 8" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
9. "ML#" INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF, SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

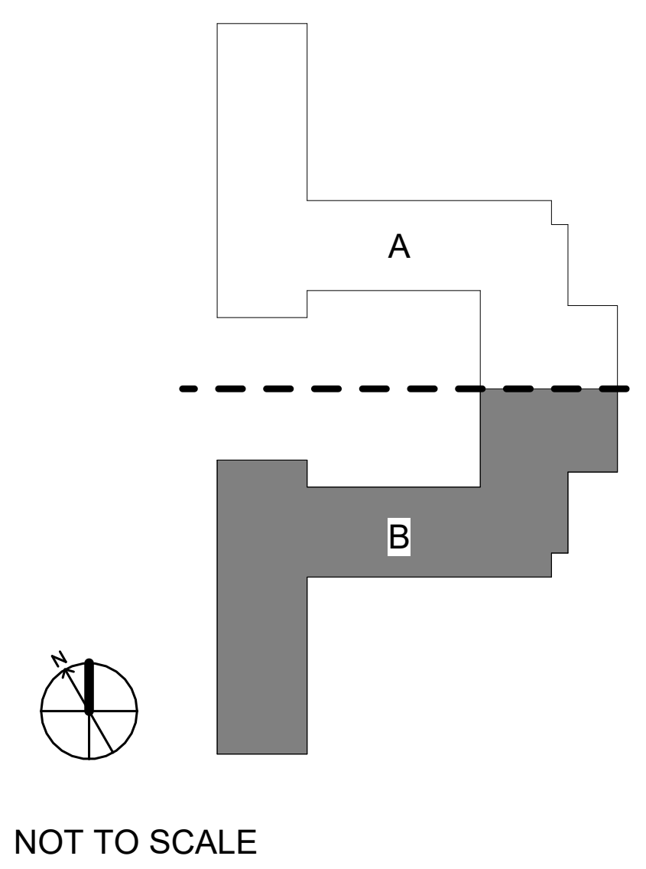
D1 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 20 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
▨	ZONE-5 EXTERIOR WALL STUDS
▥	ZONE-4 EXTERIOR WALL STUDS
▩	TYPICAL CORRIDOR WALL STUDS
▬	CORRIDOR WALL STUDS AT STORAGE
▧	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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CARL SCHUETT 47008
10/06/2025

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DESIGN TEAM
CARL SCHUETT, PE

PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
LEVEL 03 FRAMING PLAN - REGION B

SHEET NUMBER
S103B

Autodesk Docs://UNC Wilmington - Housing Village Phase 3/5142301800_Housing Village Phase 3_S_v24.rvt
W:\StudioFolder\Subfolder\########## JobName\2 Design\4 Drawings\Revit\

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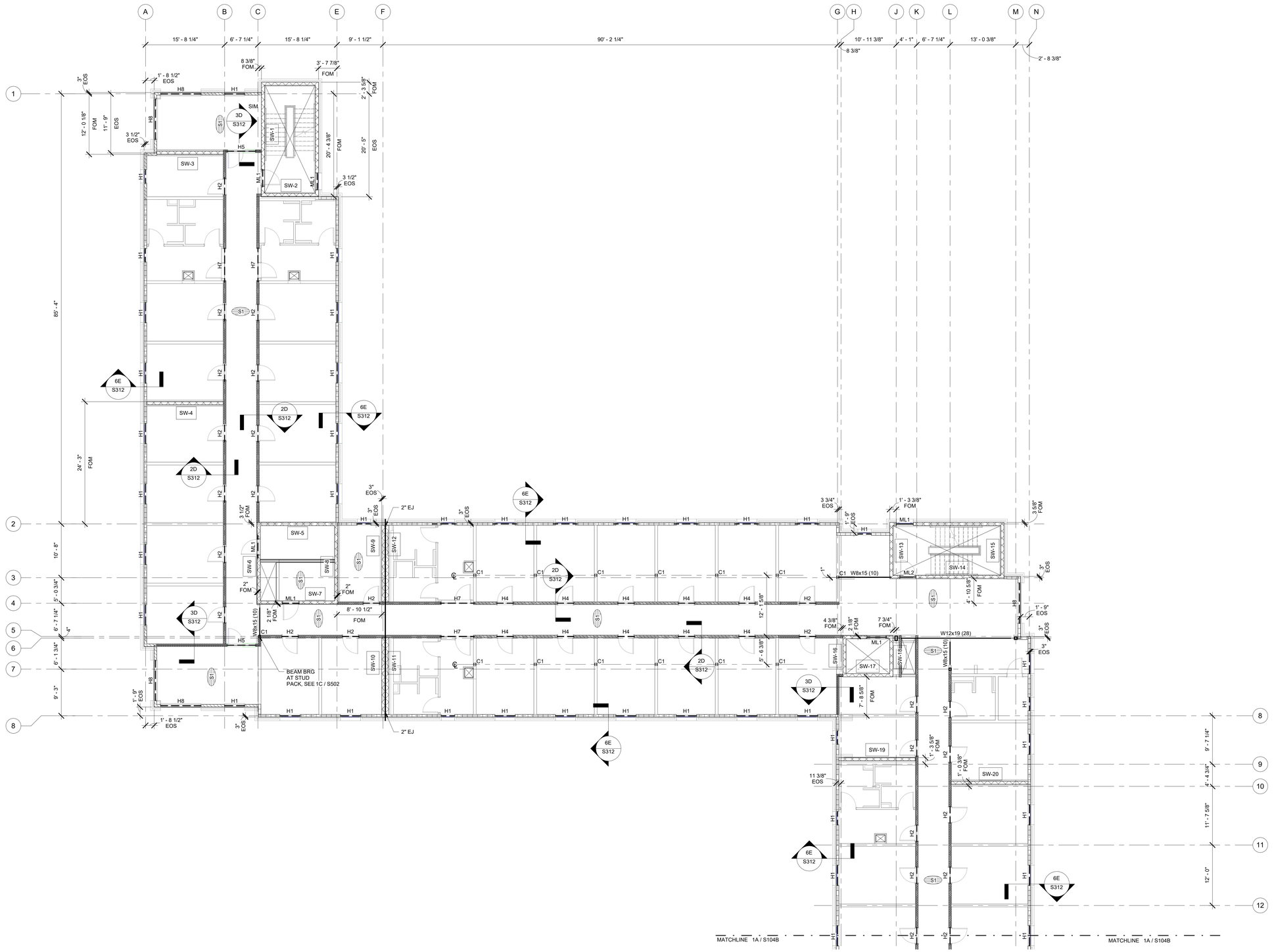
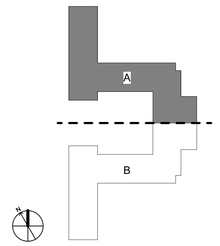


LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



NOT TO SCALE

1A / S104A — LEVEL 04 FRAMING PLAN - REGION A

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +36' - 0" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
9. "ML#" INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF. SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

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SHEET NUMBER
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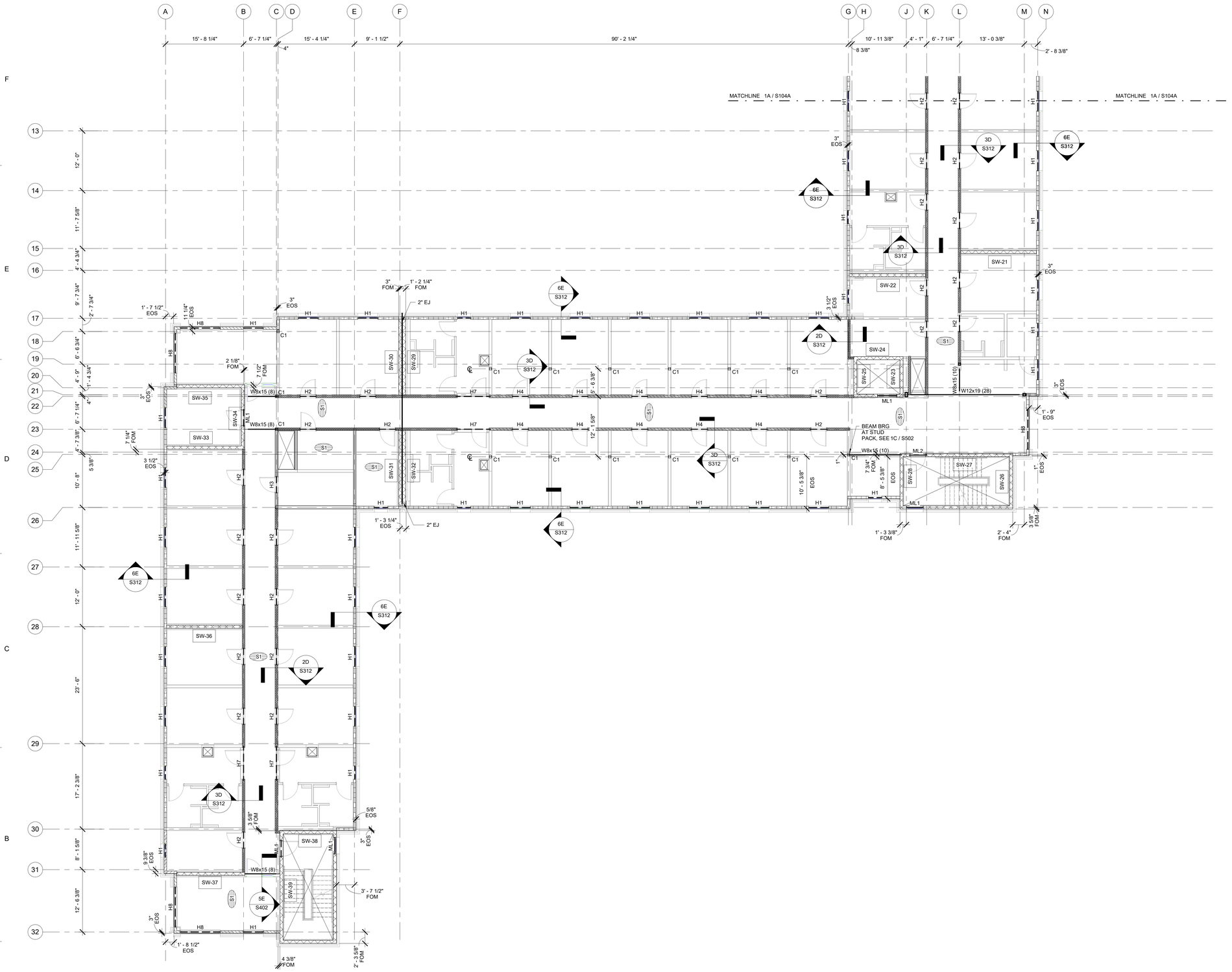

1A / S104B LEVEL 04 FRAMING PLAN - REGION B

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +36' - 0" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
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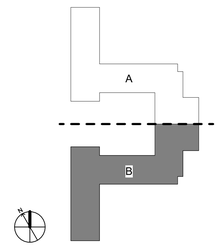
LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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10.06.2025

REVISIONS

NO.	REASON	DATE

PROJECT TEAM
PRINCIPAL IN CHARGE
WILLIAM STEWART, AIA
PROJECT MANAGER
ERIC SCHOENAGEL, AIA
DESIGN TEAM
CARL SCHUETT, PE

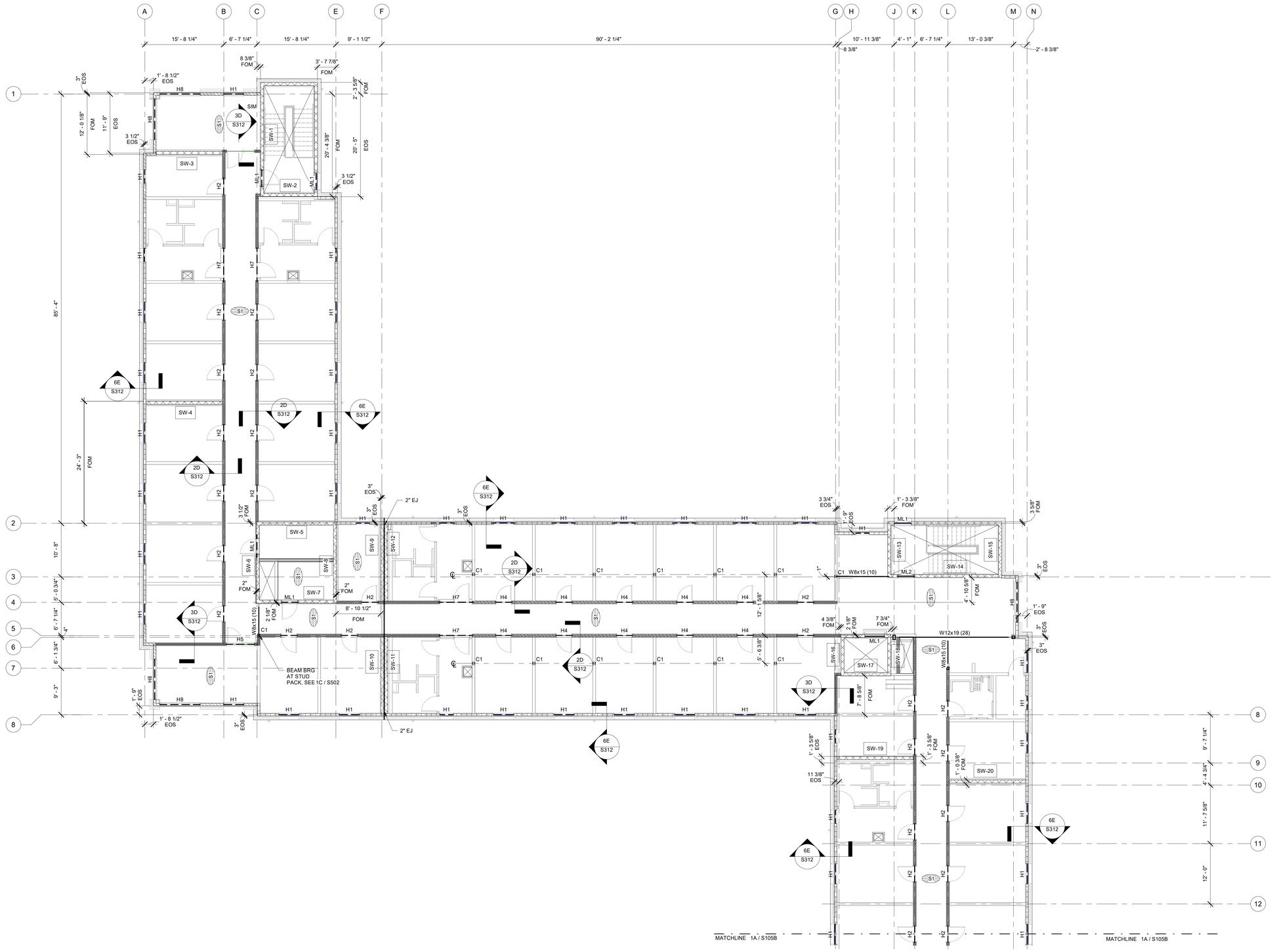
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
LEVEL 04 FRAMING PLAN - REGION B

SHEET NUMBER
S104B



1A / S105A LEVEL 05 FRAMING PLAN - REGION A

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +47' - 4" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
9. "ML#" INDICATES CMU LINTEL BELOW SLAB, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

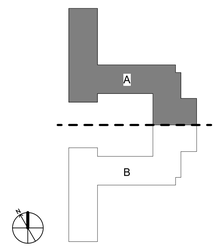
S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF, SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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NORTH CAROLINA PROFESSIONAL SEAL 47006 ENGINEER CARL SCHUETT

10/06/2025

SCO #24-29053-02A

ISSUE FOR: EARLY PROCUREMENT PACKAGES

ISSUE DATE: 10.06.2025

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PROJECT MANAGER: ERIC SCHOENAGEL, AIA
DESIGN TEAM: CARL SCHUETT, PE

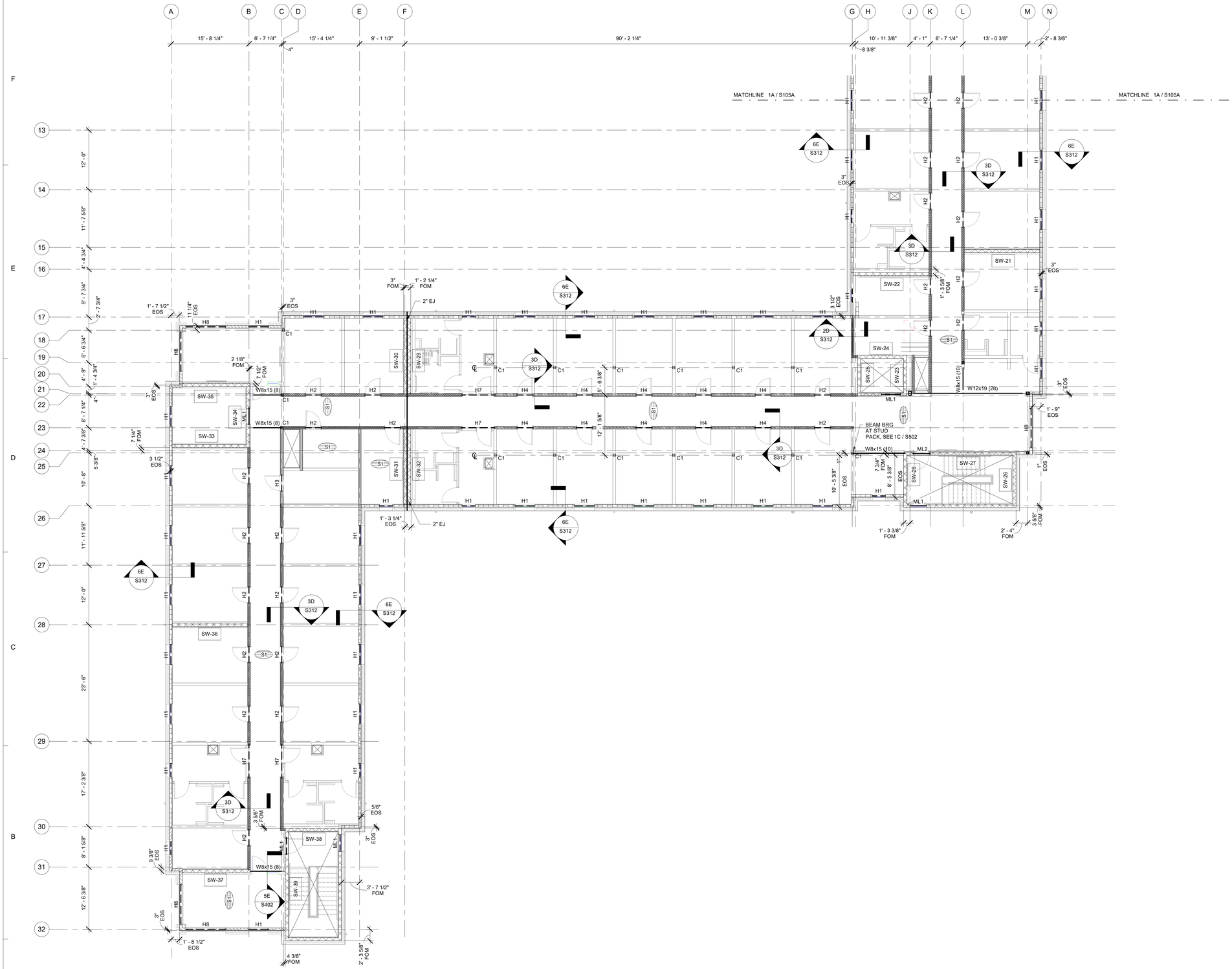
PROJECT NAME: UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.: 514.23018.00

SHEET TITLE: LEVEL 05 FRAMING PLAN - REGION A

SHEET NUMBER: S105A



1A / S105B — LEVEL 05 FRAMING PLAN - REGION B

1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +47' - 4" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
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7. SEE S301 FOR COLUMN SCHEDULE.
8. "H#" INDICATES LIGHT GAGE HEADER BELOW SLAB, SEE 4A / S501
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10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502

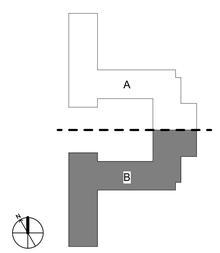
S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF, SEE TYPICAL DETAILS ON S311. PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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NORTH CAROLINA PROFESSIONAL SEAL 27006 ENGINEER CARL SCHUETT

10/06/2025

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DESIGN TEAM
CARL SCHUETT, PE

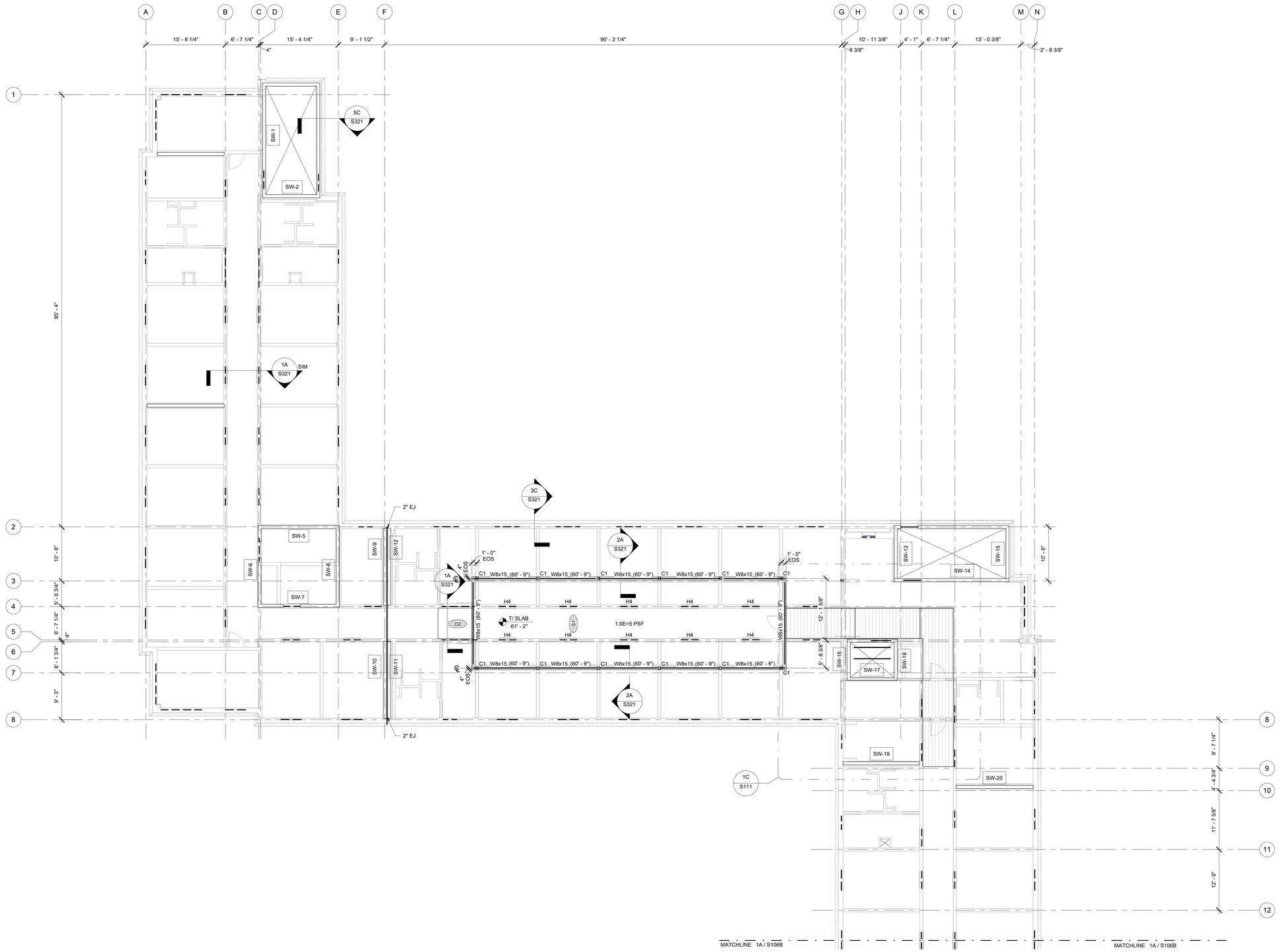
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
LEVEL 05 FRAMING PLAN - REGION B

SHEET NUMBER
S105B



1A S106A MECHANICAL PLATFORM FRAMING PLAN - REGION A

1/8" = 1'-0"

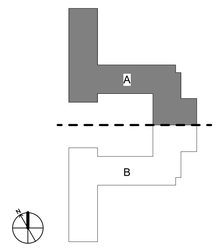
NOTES:

1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +61' - 2" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
4. MINIMUM COMPOSITE BEAM REACTION SHALL BE 10k, UNO.
5. "SW-#" INDICATES CMU SHEAR WALL, SEE S200 SERIES.
6. SEE S311 FOR TYPICAL FLOOR FRAMING DETAILS.
7. SEE S301 FOR POST SCHEDULE.
8. "H#" INDICATES LIGHT GAGE HEADER, SEE 4A / S501
9. "M-#" INDICATES CMU LINTEL, SEE 2A / S401
10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502
11. "1.0E = #PSF" INDICATES (LRFD) SEISMIC DEAD LOAD. TRUSS SUPPLIER TO DESIGN TRUSS BRACING TO TRANSFER LOAD TO SHEAR WALLS.

S1 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 3 1/2" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF, SEE TYPICAL DETAILS ON S311, PROVIDE TOP #5 x 6'-0" LG AT 12" OC OVER SUPPORT AND BOTT #5 x 6'-0" LG AT 12" OC MID-SPAN.

D2 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 22 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

KEY PLAN



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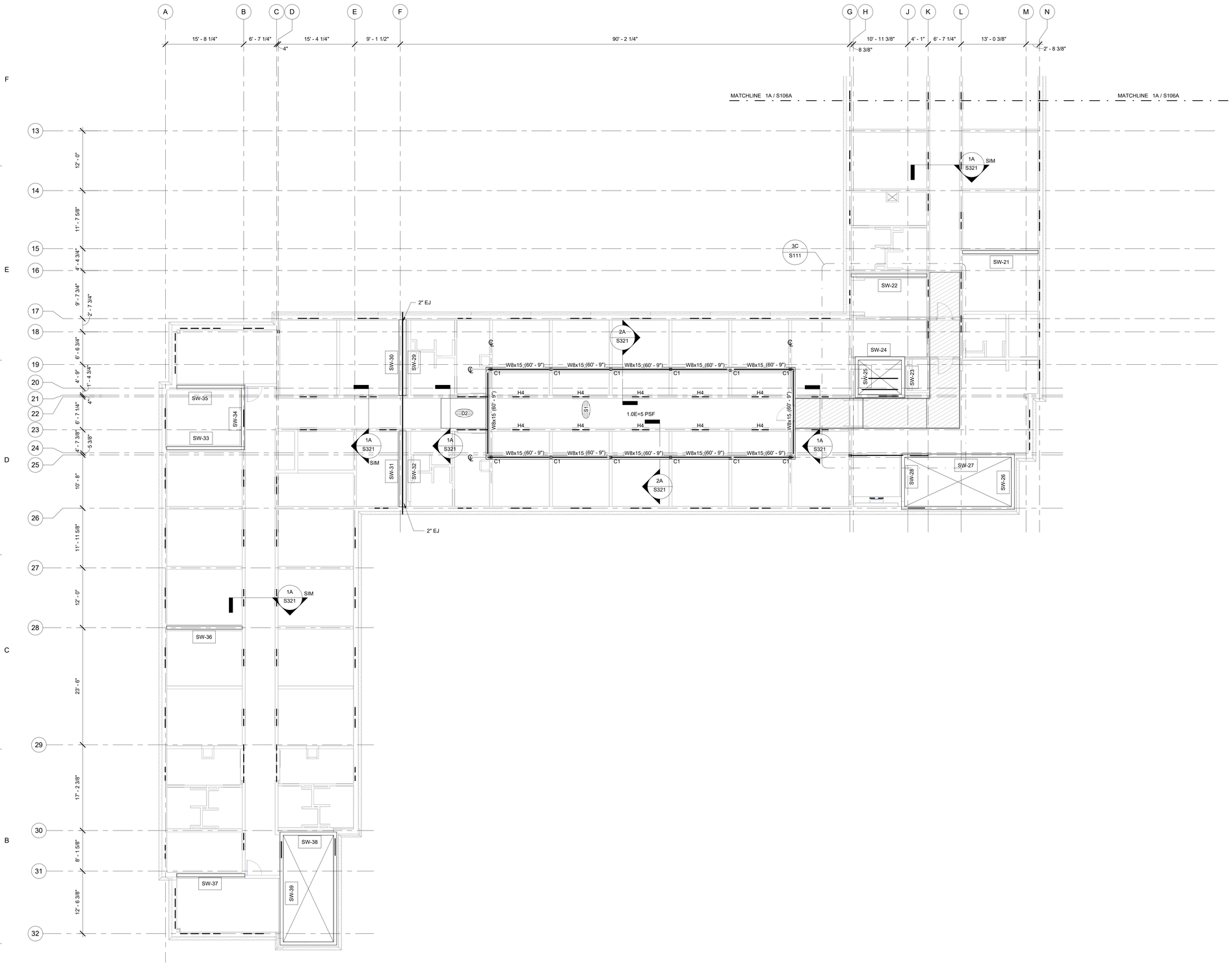
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
MECHANICAL PLATFORM FRAMING PLAN - REGION A

SHEET NUMBER
S106A



1A / S106B — MECHANICAL PLATFORM FRAMING PLAN - REGION B

1/8" = 1'-0"

NOTES:

1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. FINISH FLOOR ELEVATION +61' - 2" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. TOP OF STEEL ELEVATION 5" BELOW FINISHED FLOOR ELEVATION, UNO. (No) INDICATES TOP OF STEEL ELEVATION.
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8. "H#" INDICATES LIGHT GAGE HEADER, SEE 4A / S501
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10. "C#" - INDICATES LIGHT GAGE STUD PACK, SEE 1A / S502
11. "1.0E = #PSF" INDICATES (LRFD) SEISMIC DEAD LOAD. TRUSS SUPPLIER TO DESIGN TRUSS BRACING TO TRANSFER LOAD TO SHEAR WALLS.

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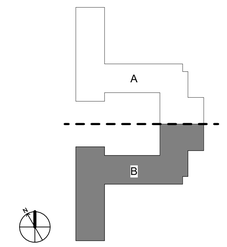
D2 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 22 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

KEY PLAN



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NORTH CAROLINA PROFESSIONAL SEAL 47006 ENGINEER CARL SCHUETT

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DESIGN TEAM
CARL SCHUETT, PE

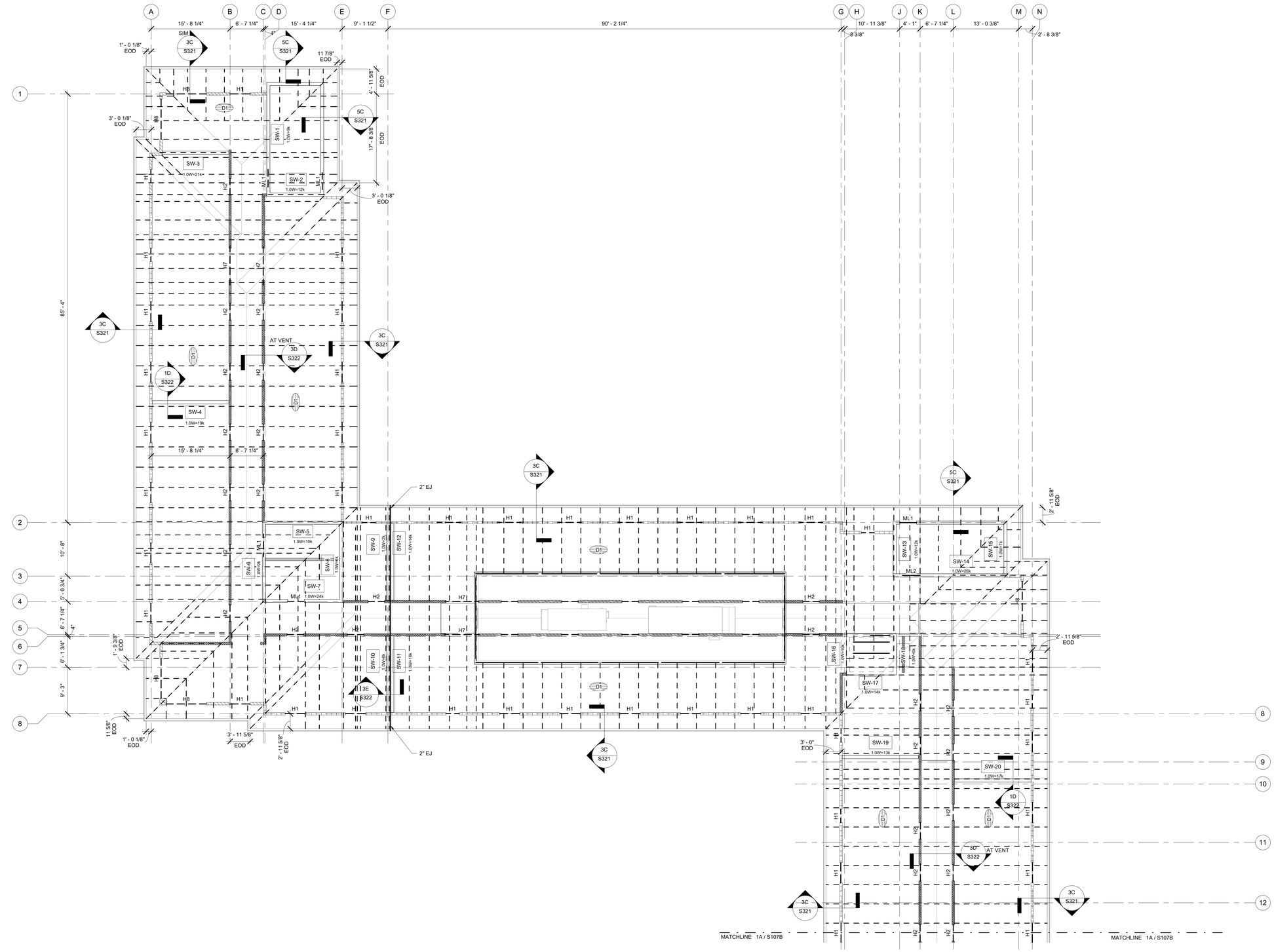
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
MECHANICAL PLATFORM FRAMING PLAN - REGION B

SHEET NUMBER
S106B



1A ROOF FRAMING PLAN- REGION A

S107A 1/8" = 1'-0"

NOTES:

1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. TRUSS BEARING +58' - 8" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. SEE S321 FOR TYPICAL ROOF FRAMING DETAILS.
4. — — — INDICATES LIGHT GAGE METAL ROOF TRUSS AT 4'-0" MAX OC.
5. "H#" INDICATES LIGHT GAGE HEADER; SEE 4A / S501
6. "ML#" INDICATES CMU LINTEL, SEE 2A / S401
7. "1.0W=##k" INDICATES (LRFD) SHEAR WALL COLLECTOR LOADS. TRUSS SUPPLIER TO DESIGN TRUSS CONNECTIONS & DRAG TRUSSES TO TRANSFER INDICATED LOADS TO SHEAR WALLS.
8. TOP OF CMU SHEAR WALLS = 58' - 8" UNO.
9. TRUSS SUPPLIER TO DESIGN AND PROVIDE TRUSS CONNECTIONS FOR TRUSS UPLIFT AT TRUSS BEARING LOCATIONS.

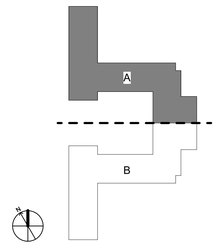
D1 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 20 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



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NORTH CAROLINA PROFESSIONAL SEAL 47006 ENGINEER CARL SCHUETT

10/06/2025

SCO #24-29053-02A

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PRINCIPAL IN CHARGE: WILLIAM STEWART, AIA

PROJECT MANAGER: ERIC SCHOENAGEL, AIA

DESIGN TEAM: CARL SCHUETT, PE

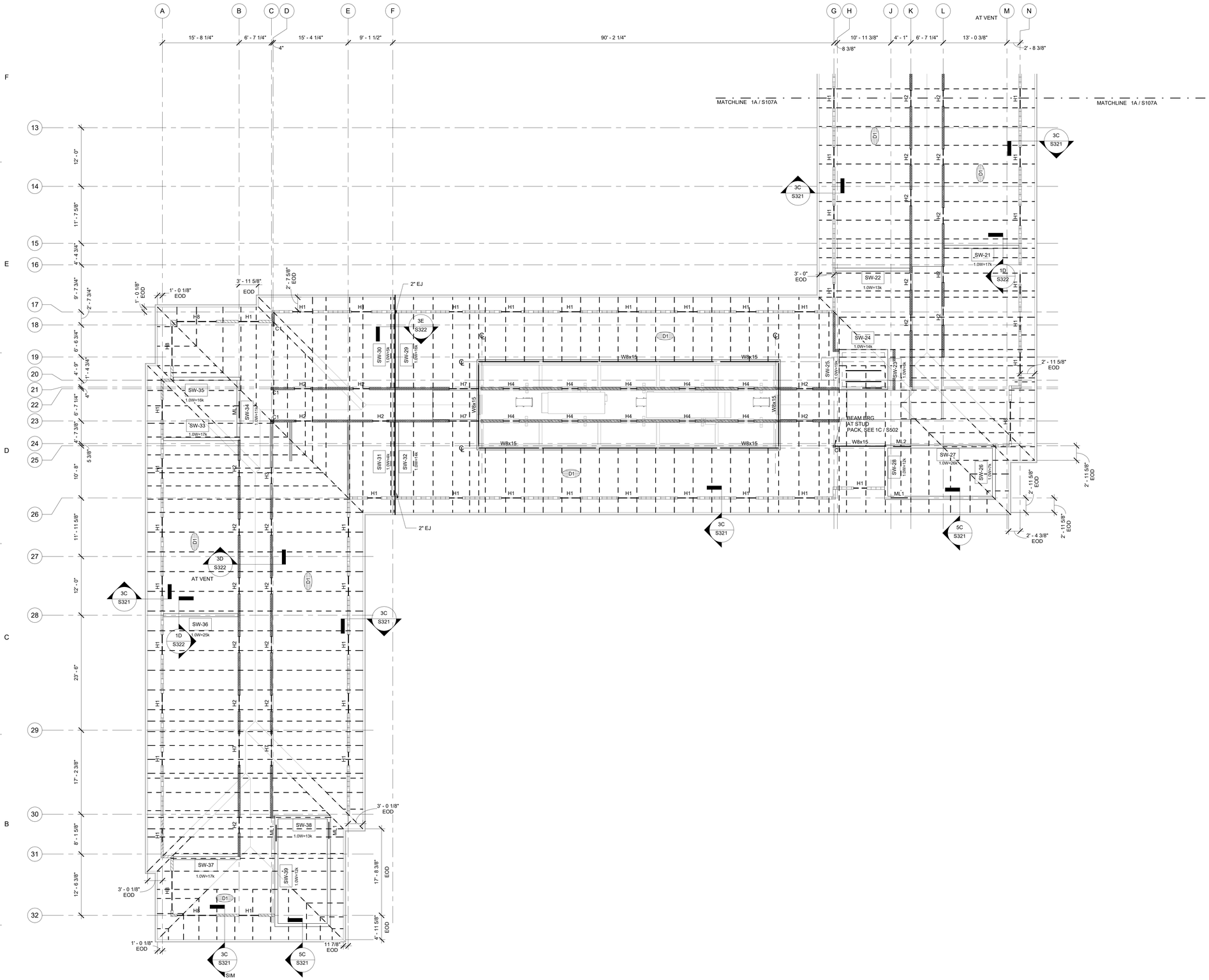
PROJECT NAME: UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO: 514.23018.00

SHEET TITLE: ROOF FRAMING PLAN - REGION A

SHEET NUMBER: S107A

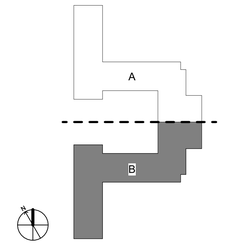


LOAD BEARING WALL SCHEDULE

PATTERN	LOCATION
	ZONE-5 EXTERIOR WALL STUDS
	ZONE-4 EXTERIOR WALL STUDS
	TYPICAL CORRIDOR WALL STUDS
	CORRIDOR WALL STUDS AT STORAGE
	CORRIDOR WALL STUDS AT MECHANICAL PLATFORM

NOTES:
1. SEE 1E / S501 FOR ADDITIONAL INFORMATION.

KEY PLAN



NOT TO SCALE

1A ROOF FRAMING PLAN- REGION B

S107B 1/8" = 1'-0"

NOTES:
1. SEE S001 FOR GENERAL NOTES AND ABBREVIATIONS.
2. TRUSS BEARING +58' - 8" ABOVE REFERENCE DATUM ELEVATION, UNO.
3. SEE S321 FOR TYPICAL ROOF FRAMING DETAILS.
4. — — — INDICATES LIGHT GAGE METAL ROOF TRUSS AT 4'-0" MAX OC.
5. "H#" INDICATES LIGHT GAGE HEADER, SEE 4A / S501
6. "ML#" INDICATES CMU LINTEL, SEE 2A / S401
7. "1.0W=##k" INDICATES (LRFD) SHEAR WALL COLLECTOR LOADS. TRUSS SUPPLIER TO DESIGN TRUSS CONNECTIONS & DRAG TRUSSES TO TRANSFER INDICATED LOADS TO SHEAR WALLS.
8. TOP OF CMU SHEAR WALLS = 58' - 8" UNO.
9. TRUSS SUPPLIER TO DESIGN AND PROVIDE TRUSS CONNECTIONS FOR TRUSS UPLIFT AT TRUSS BEARING LOCATIONS.

D1 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 20 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

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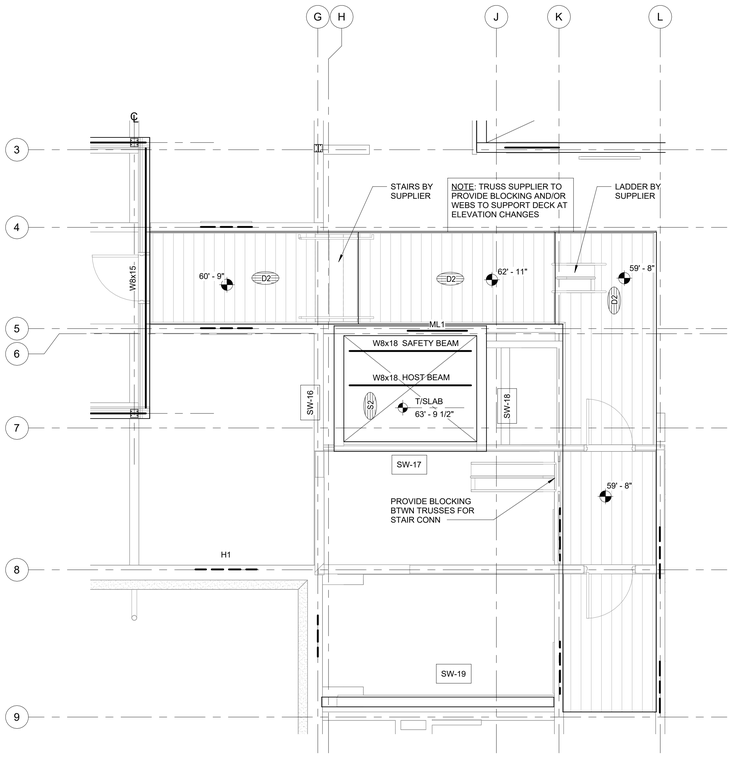
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

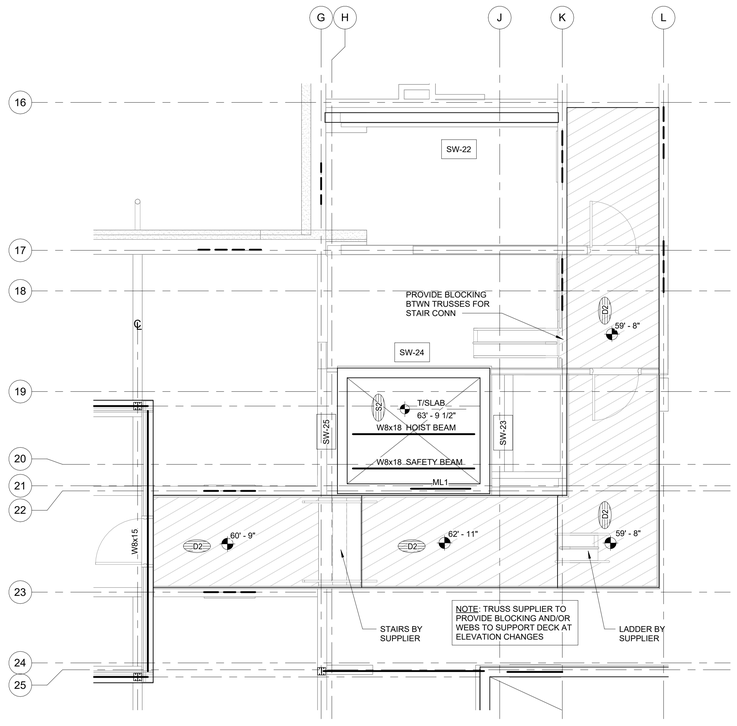
SHEET TITLE
ROOF FRAMING PLAN - REGION B

SHEET NUMBER
S107B



1C | S111 ENLARGED FLOOR PLAN - MECH PLATFORM CATWALK - AREA A

1/4" = 1'-0"



3C | S111 ENLARGED FLOOR PLAN - MECH PLATFORM CATWALK - AREA B

1/4" = 1'-0"

S2 INDICATES SPAN DIRECTION OF 1 1/2" VLR - 20 GA COMPOSITE STEEL DECK, GALV G60 FINISH, w/ 4" NORMAL WEIGHT CONCRETE REINFORCED w/ 6x6-W2.1xW2.1 WWF

D2 INDICATES SPAN DIRECTION OF METAL ROOF DECK, 1 1/2" TYPE "B", 22 GA, GALV G60 FINISH, SEE S321 FOR ATTACHMENT

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PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

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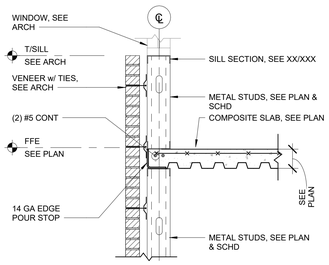
PROJECT NO.
514.23018.00

SHEET TITLE
ENLARGED FLOOR PLANS

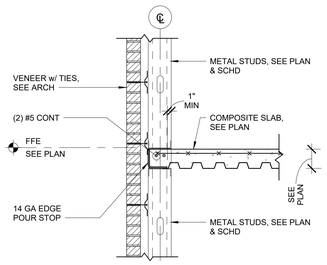
SHEET NUMBER
S111



1E S312 EXTERIOR BEARING DETAIL AT WINDOW

3/4" = 1'-0"

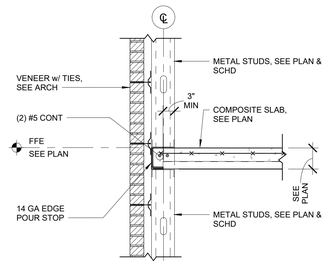
NOTES:
1. SEE PLAN FOR BOTTOM REINF.



2E S312 EXTERIOR BEARING DETAIL PARALLEL TO DECK

3/4" = 1'-0"

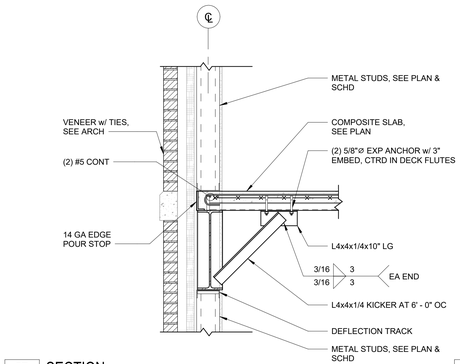
NOTES:
1. SEE PLAN FOR BOTTOM REINF.



3E S312 EXTERIOR BEARING DETAIL PERPENDICULAR TO DECK

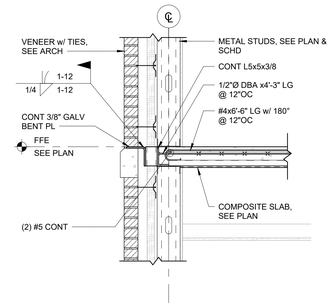
3/4" = 1'-0"

NOTES:
1. SEE PLAN FOR BOTTOM REINF.



5E S312 SECTION

3/4" = 1'-0"



6E S312 SECTION

3/4" = 1'-0"

1D S312 INTERIOR BRG DETAIL PARALLEL TO DECK

3/4" = 1'-0"

NOTES:
1. SEE PLAN FOR BOTTOM REINF.

2D S312 INTERIOR BRG DETAIL PERPENDICULAR TO DECK

3/4" = 1'-0"

NOTES:
1. SEE PLAN FOR BOTTOM REINF.

3D S312 DECK DIRECTION CHANGE AT INTERIOR BEARING WALL

3/4" = 1'-0"

NOTES:
1. AT SIMILAR CONDITION, SUPPORT OF COMPOSITE SLAB VARIES, SEE PLAN.

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NORTH CAROLINA PROFESSIONAL SEAL 47006 ENGINEER CARL SCHUETT

10/06/2025

SCO #24-29053-02A

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NO.	REASON	DATE

PROJECT TEAM

PRINCIPAL IN CHARGE
WILLIAM STEWART, AIA

PROJECT MANAGER
ERIC SCHOENAGEL, AIA

DESIGN TEAM
CARL SCHUETT, PE

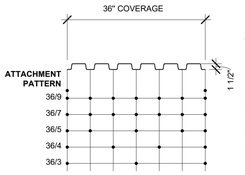
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
TYPICAL FLOOR FRAMING DETAILS

SHEET NUMBER
S312



DECKING ATTACHMENT D2

DECK TYPE.....B	Ip =	0.155 IN⁴/FT
DECK GAGE.....22 GA	In =	0.183 IN⁴/FT
DECK FINISH.....GALVANIZED	Sp =	0.186 IN³/FT
	Fy =	50 KSI

PROVIDE 36/4 ATTACHMENT PATTERN AT ALL SUPPORTS AND AT ENDS WITH #12 TEK SCREWS

PROVIDE SIDELAP CONNECTORS (AT 12" OC) PER DECK SPAN BETWEEN SUPPORTS WITH #10 TEK SCREWS

PROVIDE #12 TEK SCREWS AT 6" OC AT EDGE SUPPORTS

DECKING ATTACHMENT D1

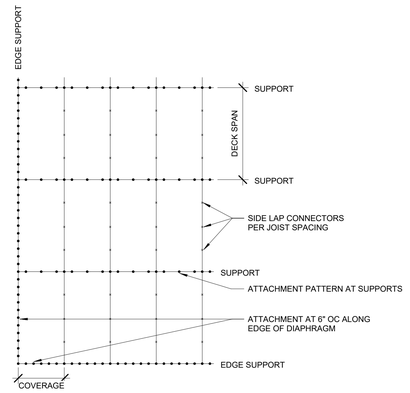
DECK TYPE.....B	Ip =	0.201 IN⁴/FT
DECK GAGE.....20 GA	In =	0.222 IN⁴/FT
DECK FINISH.....GALVANIZED	Sp =	0.234 IN³/FT
	Fy =	50 KSI

PROVIDE 36/9 ATTACHMENT PATTERN AT ALL SUPPORTS AND AT ENDS WITH #12 TEK SCREWS

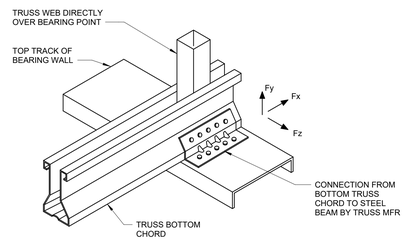
PROVIDE SIDELAP CONNECTORS (AT 6" OC) PER DECK SPAN BETWEEN SUPPORTS WITH #10 TEK SCREWS

PROVIDE #12 TEK SCREWS AT 6" OC AT EDGE SUPPORTS

1E ROOF DECK ATTACHMENT SCHEDULE
S321 NOT TO SCALE

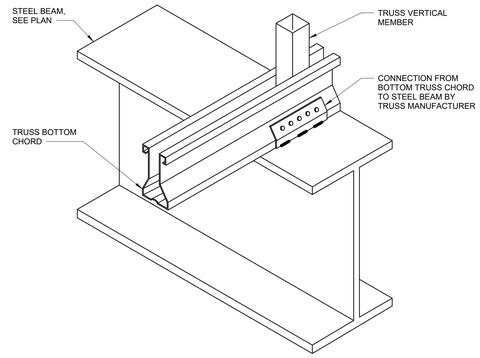


3E PLAN VIEW OF DECK ATTACHMENT
S321 NOT TO SCALE



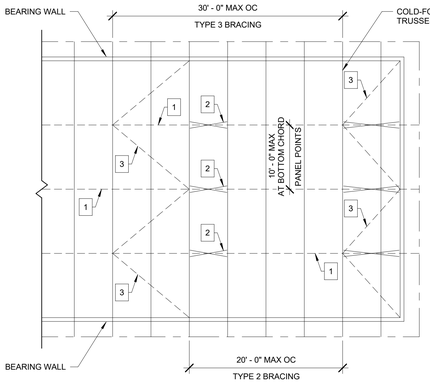
4E TYPICAL TRUSS-TO BEARING AT CFS WALL
S321 NOT TO SCALE

NOTES:
1. TRUSS MANUFACTURER SHALL DESIGN TRUSS-TO-BEARING CONNECTIONS FOR UPLIFT AS WELL AS MIN 500 LB MIN IN-PLANE (Fx) SHEAR AT EACH TRUSS-TO-BEARING CONNECTION ALONG EXTERIOR SUPPORTS.



6E TYPICAL TRUSS-TO BEARING AT STRUCTURAL STEEL
S321 NOT TO SCALE

NOTES:
1. TRUSS MANUFACTURER SHALL DESIGN TRUSS-TO-BEARING CONNECTIONS FOR UPLIFT AS WELL AS MIN 500 LB MIN IN-PLANE (Fx) SHEAR AT EACH TRUSS-TO-BEARING CONNECTION ALONG EXTERIOR SUPPORTS.



1 = CONTINUOUS 362S162-43 STEEL STUD BRACING FOR COLD-FORMED STEEL TRUSS BOTTOM CHORD (SPACE 10'-0" OC MAX AT BOTTOM CHORD PANEL POINTS)

2 = "VERT" 362S162-43 STEEL STUD CROSS BRACING OF WEB MEMBERS AT EACH ROW OF BOTTOM CHORD BRACING. LOCATE AT ENDS OF THE BOTTOM CHORD BRACING AND REPEAT AT 20'-0" OC (MAX) INTERVALS

3 = 362S162-43 STEEL STUD BOTTOM CHORD DIAGONAL BRACING. LOCATE AT END OF BUILDING AND REPEAT AT 30'-0" OC (MAX) INTERVALS

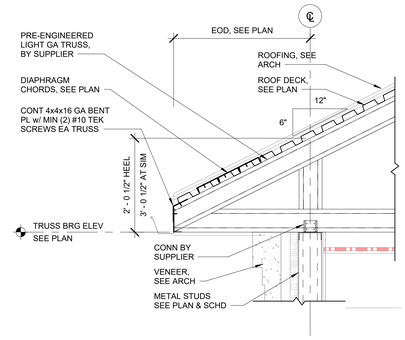
PROVIDE 362S162-43 STEEL STUD CROSS BRACING AT 20'-0" OC AT PERMANENT CONTINUOUS LATERAL BRACING AS SPECIFIED BY THE TRUSS ENGINEER

LAP ALL LATERAL BOTTOM CHORD BRACES OVER A MINIMUM OF 2 TRUSSES

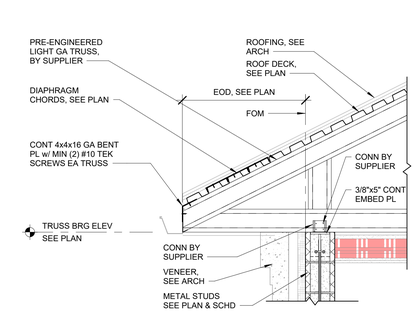
ALL CONNECTIONS SHOULD BE MADE WITH (3) #12 SELF DRILLING SCREWS FOR DIAGONAL BRACING, AND (2) #12 SELF DRILLING SCREWS FOR LATERAL BRACING

1C MINIMUM COLD-FORMED STEEL TRUSS BRACING DIAGRAM
S321 3/4" = 1'-0"

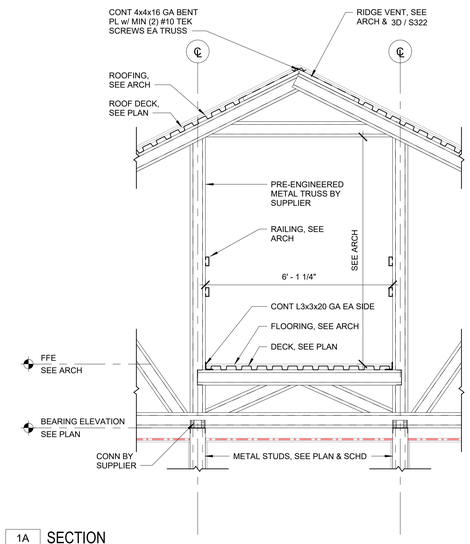
NOTES:
1. TRUSS MANUFACTURER SHALL DESIGN COMPLETE BRACING SYSTEM AND PROVIDE PLANS AND DETAILS OF BRACING AS PART OF SHOP DRAWING SUBMITTAL.



3C TYPICAL TRUSS BEARING AT EXTERIOR WALL
S321 3/4" = 1'-0"

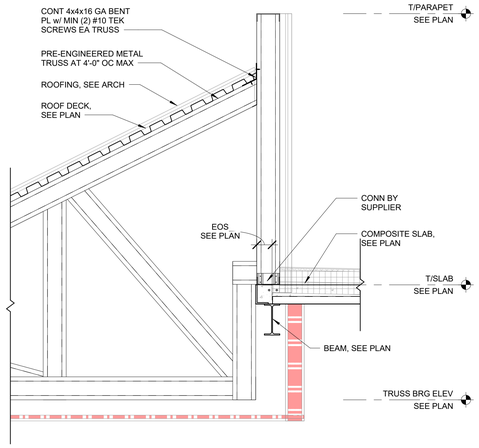


5C TYPICAL TRUSS BEARING AT CMU WALL
S321 3/4" = 1'-0"



1A SECTION
S321 1/2" = 1'-0"

NOTES:
1. AT SIMILAR SECTION, OPENING FOR CATWALK SHALL REMAIN, DECKING IS OMITTED.



2A SECTION
S321 3/4" = 1'-0"

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10/06/2025

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DESIGN TEAM
CARL SCHUETT, PE

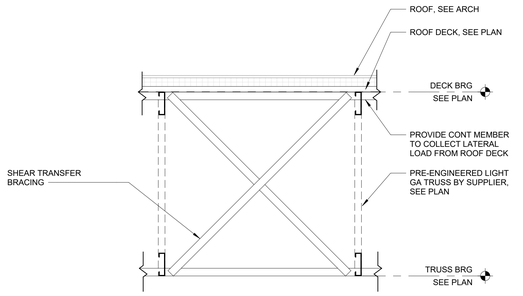
PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

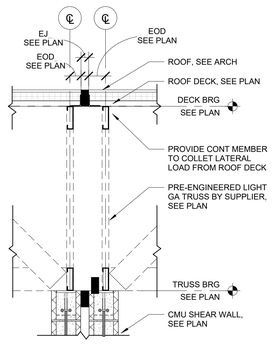
PROJECT NO.
514.23018.00

SHEET TITLE
ROOF FRAMING DETAILS

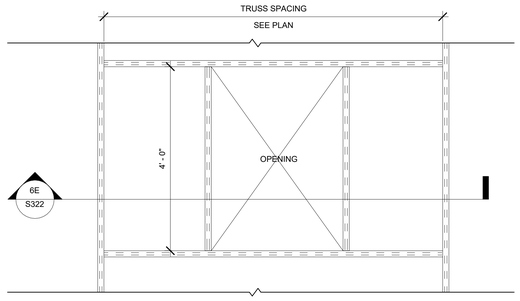
SHEET NUMBER
S321



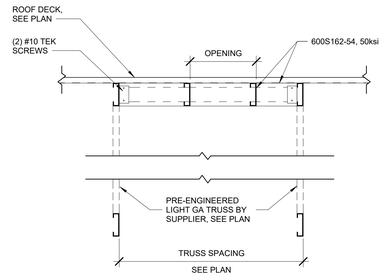
1E **TRUSS SHEAR TRANSFER**
S322 3/4" = 1'-0"



3E **SECTION AT EXPANSION JT**
S322 3/4" = 1'-0"



5E **TYP DECK SUPPORT AT ROOF OPENINGS**
S322 3/4" = 1'-0"



6E **SECTION A-A**
S322 3/4" = 1'-0"

1D **DRAG TRUSS BEARING**
S322 3/4" = 1'-0"

3D **SECTION AT RIDGE VENT**
S322 3/4" = 1'-0"

NOTES:
1. SEE ARCHITECTURAL DRAWINGS FOR EXTENT OF RIDGE VENT.

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ERIC SCHOENAGEL, AIA
DESIGN TEAM
CARL SCHUETT, PE

PROJECT NAME
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UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
ROOF TRUSS FRAMING DETAILS

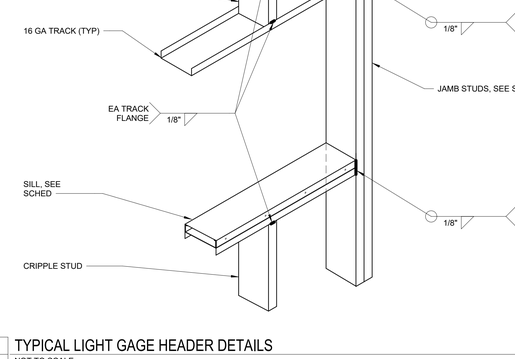
SHEET NUMBER
S322

LOAD BEARING WALL SCHEDULE

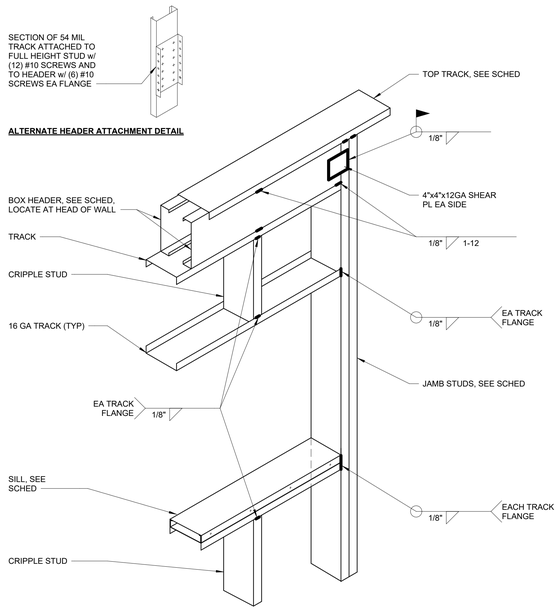
	ZONE-5 EXT	ZONE-4 EXT	CORRIDOR	CORRIDOR (STORAGE)	CORRIDOR (MECH PLATFORM)
ROOF					
5TH	600S200-54 (50KSI) AT 16"	600S200-43 (50KSI) AT 16"	600S162-43 (50KSI) AT 16"	600S162-43 (50KSI) AT 16"	600S162-43 (50KSI) AT 16"
4TH	600S200-54 (50KSI) AT 16"	600S200-43 (50KSI) AT 16"	600S162-43 (50KSI) AT 16"	600S162-43 (50KSI) AT 16"	600S162-43 (50KSI) AT 16"
3RD	600S200-54 (50KSI) AT 16"	600S200-43 (50KSI) AT 16"	600S200-43 (50KSI) AT 16"	600S250-54 (50KSI) AT 16"	600S200-54 (50KSI) AT 16"
2ND	600S200-68 (50KSI) AT 16"	600S200-54 (50KSI) AT 16"	600S250-54 (50KSI) AT 16"	600S250-68 (50KSI) AT 16"	600S200-68 (50KSI) AT 16"
1ST	600S200-68 (50KSI) AT 16"	600S200-68 (50KSI) AT 16"	600S250-68 (50KSI) AT 16"	600S350-68 (50KSI) AT 16"	600S250-68 (50KSI) AT 16"

1E LOAD BEARING WALL SCHEDULE
S501 NOT TO SCALE

NOTES:
1. CONNECTION OF STUD TO BOTTOM TRACK, SEE 1D / S501



1D STUD TO TRACK CONNECTION
S501 NOT TO SCALE



1A TYPICAL LIGHT GAGE HEADER DETAILS
S501 NOT TO SCALE

OPENING TYPE

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CRIPPLE STUD	SILL CONNECTION TO JAMB
"H1"	LEVEL 05	600S350-54 (50KSI)	1A / S501	1D / S501		(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501		600T125-33 (33KSI)	600S162-43 CRIPPLE STUD w/ (2) #12 SCREW EA FLANGE	1A / S501
	LEVEL 04	600S350-54 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)			600T125-33 (33KSI)	600S162-43 CRIPPLE STUD w/ (2) #12 SCREW EA FLANGE	
	LEVEL 03	600S350-68 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)			600T125-33 (33KSI)	600S162-43 CRIPPLE STUD w/ (2) #12 SCREW EA FLANGE	
	LEVEL 02	600S250-97 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)			600T125-33 (33KSI)	600S162-43 CRIPPLE STUD w/ (2) #12 SCREW EA FLANGE	
	LEVEL 01	600S250-118 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)			600T125-33 (33KSI)	600S162-43 CRIPPLE STUD w/ (2) #12 SCREW EA FLANGE	

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H2"	LEVEL 05	600S250-54 (50KSI)	1A / S501	1D / S501		(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501				
	LEVEL 04	600S250-54 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 03	600S350-68 (50KSI)				(2) 600S162-54(50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 02	600S250-97 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 01	600S250-118 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H3"	LEVEL 05	600S200-54 (50KSI)	1A / S501	1D / S501		(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501				
	LEVEL 04	600S350-54 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 03	600S250-97 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 02	600S350-97 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 01	600S350-118 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H4"	LEVEL 05	600S200-54 (50KSI)	1A / S501	1D / S501		(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501				
	LEVEL 04	600S200-68 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 03	600S350-68 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 02	600S300-97 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 01	600S250-118 (50KSI)				(2) 600S162-54 (50KSI) +(2) 600T125-33 (33KSI)					

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H5"	LEVEL 05	600S200-54 (50KSI)	1A / S501	1D / S501		(2) 600S250-97 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501				
	LEVEL 04	600S350-54 (50KSI)				(2) 600S250-97 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 03	600S250-97 (50KSI)				(2) 600S250-97 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 02	600S250-118 (50KSI)				(2) 600S250-97 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 01	600S350-118(50KSI)				(2) 600S250-97 (50KSI) +(2) 600T125-33 (33KSI)					

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H6"	LEVEL 01	600S200-43 (50KSI)				(2) 600S250-54 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501				

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H7"	LEVEL 05	600S200-54 (50KSI)	1A / S501	1D / S501		(2) 600S200-68 (50KSI) +(2) 600T125-33 (33KSI)	1A / S501				
	LEVEL 04	600S350-68 (50KSI)				(2) 600S300-97 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 03	600S250-118 (50KSI)				(2) 600S300-97 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 02	600S350-118 (50KSI)				(2) 600S300-97 (50KSI) +(2) 600T125-33 (33KSI)					
	LEVEL 01	600S300-97(50KSI)+ 600T125-33(33KSI)				(2) 600S300-97 (50KSI) +(2) 600T125-33 (33KSI)					

LOCATION	FLOOR	JAMB SIZE	JAMB CONN TO STRUCT AT TOP	JAMB CONN TO STRUCT AT BOTT	JAMB REF	HEADER SIZE	HEADER CONNECTION TO JAMB	HEADER REF	SILL SIZE	SILL CONNECTION TO JAMB	SILL CONNECTION TO JAMB
"H8"	LEVEL 05	600S250-118 (50KSI)	1A / S501	1D / S501		(2) 600S250-118 (50KSI) +(2) 600T150-68 (50KSI) +(2) 600T150-43 (33KSI)	1A / S501				
	LEVEL 04	600S250-97 (50KSI)				(2) 600S300-118 (50KSI) +(2) 600T200-33 (33KSI)					
	LEVEL 03	600S250-118 (50KSI)				(2) 600S300-118 (50KSI) +(2) 600T200-33 (33KSI)					
	LEVEL 02	600S350-118 (50KSI)				(2) 600S300-118 (50KSI) +(2) 600T200-33 (33KSI)					
	LEVEL 01	600S350-188(50KSI)+ 600T200-33(33KSI)				(2) 600S300-118 (50KSI) +(2) 600T200-33 (33KSI)					

4A HEADER/JAMB SCHEDULE
S501 NOT TO SCALE

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CARL SCHUETT, PE

PROJECT NAME
UNIVERSITY OF NORTH CAROLINA AT WILMINGTON HOUSING VILLAGE PHASE 3

UNC WILMINGTON, NC

PROJECT NO.
514.23018.00

SHEET TITLE
TYPICAL LIGHT GAGE FRAMING DETAILS

SHEET NUMBER
S501