GENERAL NOTES:

1. BUILDING CODE: SCBC 2021
2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION AND THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
3. ALL DIMENSIONS TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS.
4. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
5. ALL ASTM SPECIFICATIONS NOTED ON THESE DRAWINGS SHALL BE OF THE LATEST REVISION UNLESS NOTED OTHERWISE.
5. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, UNLESS NOTED OTHERWISE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMEN, OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT LIMITED TO, BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, SHORING FOR EARTH BANKS, FORM SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GINE POLES, ETC. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK, AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES, AS A PART OF HIS RESPONSIBILITY.
7. OPENINGS, POCKETS, ETC. SHALL NOT BE PLACED IN SLABS, DECKS, BEAMS, JOISTS, COLUMNS, WALLS, ETC. UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS. NOTIFY OWNER'S REPRESENTATIVE WHEN OTHER DRAWINGS SHOW OPENINGS, POCKETS, ETC. THAT ARE NOT LIKEWISE SHOWN ON THE STRUCTURAL DRAWINGS. HOLES 3" ROUND OR SQUARE (MAXIMUM) SPACED AT 2'-0" (MINIMUM) IN A FLOOR SLAB, ROOF SLAB (DECK), OR WALL SHALL BE EXEMPT FROM THIS REQUIREMENT.

   SLEEVES, INSERTS AND OTHER ITEMS TO BE CAST IN CONCRETE SHALL BE SET BY THE CONTRACTOR AT LOCATIONS DESIGNATED BY, AND UNDER THE SUPERVISION OF, A REPRESENTATIVE OF EACH TRADE.
8. SEE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:
   A. SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS.
   B. SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS.
   C. SIZE AND LOCATION OF ALL CONCRETE CURBS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, ETC.
   D. SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS.
   E. FLOOR AND ROOF FINISHES.
9. SEE MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR THE FOLLOWING INFORMATION:
   A. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC.
   B. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
   C. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL, OR PLUMBING FIXTURES.
   D. MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS FOR MOTOR MOUNTS.
   E. UNDERGROUND CONCRETE DUCTS, TRENCHES, PITS, OR MANHOLES.
10. ALL HEAVY EQUIPMENT PIECES, SUCH AS COMPUTERS, SAFES, FILE CABINETS, ETC. WITH A UNIT LOAD HIGHER THAN THE DESIGN LOAD SHALL NOT BE PLACED ON ANY FLOOR WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
11. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS OR ROOF. THE CONSTRUCTION MATERIAL LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD FOR EACH PARTICULAR LEVEL.
12. EQUIVALENT MATERIALS SUBSTITUTED AS PER "APPROVED EQUAL" NOTE SHALL BE APPROVED BY ARCHITECT BEFORE USE. ANY MATERIAL DESIGNATED WITH A BRAND NAME MAY BE SUBSTITUTED WITH ITS EQUAL IF THE SO CALLED EQUIVALENT IS FIRST APPROVED BY THE ARCHITECT. THE CONTRACTOR SHALL SUPPLY INFORMATION AS REQUESTED TO VERIFY MATERIAL IS EQUAL TO SPECIFIED MATERIAL.
13. DETAILS LABELED "TYPICAL" SHALL APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SEE DETAIL TITLES FOR APPLICABILITY OF A PARTICULAR DETAIL. TYPICAL DETAILS SHALL APPLY WHETHER OR NOT THEY ARE SPECIFICALLY KEYED AT EACH LOCATION. THE ENGINEER SHALL HAVE FINAL AUTHORITY TO DETERMINE APPLICABILITY OF TYPICAL DETAILS.

FOUNDATION NOTES:

1. FOUNDATION DESIGN BASED ON RECOMMENDATIONS FROM REPORT OF SUPPLEMENTAL GEOTECHNICAL EXPLORATION, CAROLINA FOREST CAMPUS HOSPITAL PARKING DECK, MYRTLE BEACH, SC; PREPARED BY S&ME. PROJECT NUMBER: 25630007 DATED APRIL 8, 2025.
2. DESIGN LATERAL EARTH PRESSURES FOR BELOW-GRADE WALLS:
   A. AT-REST STATE, (Ko) = 54 PCF (EFP)
   B. ACTIVE STATE, (Ka) = 36 PCF (EPF)
   C. PASSIVE STATE, EARTH PRESSURE COEFFICIENT (Kp) = 374 PCF (EFP)
   D. γw SOIL = 115 PCF GRANULAR BACKFILL (SAND)
   E. COEFFICIENT OF FRICTION = 0.36 CONCRETE ON SOIL.
3. WHERE FINISHED GRADES DIFFER ON OPPOSITE SIDES OF WALL, BRACE WALL TO PREVENT MOVEMENT UNTIL ALL ADJACENT FILL, COMPACTION, AND TOP OF WALL BRACING ARE COMPLETED.
4. ALL SLABS AND FOUNDATIONS BEARING ON FILL MATERIAL SHALL BEAR ON APPROVED STRUCTURAL FILL AS SPECIFIED AND AS APPROVED BY THE GEOTECHNICAL TESTING AGENCY.
5. ALL EXCAVATION AND BACKFILLING PROCEDURES AND MATERIALS SHALL CONFORM TO RECOMMENDATIONS OUTLINED BY GEOTECHNICAL INVESTIGATION (NOTE 1). THE TESTING AGENCY SHALL INSPECT AND APPROVE ALL PROCEDURES AND MATERIALS.

DESIGN LOADS:

DESIGN LOADS SHALL CONFORM TO 2021 SOUTH CAROLINA BUILDING CODE AND ASCE7-16, UNO.

ROOF SNOW LOADS:
GROUND SNOW LOAD Pg = 10 PSF
FLAT-ROOF SNOW LOAD, Pf = 7.6 PSF
SNOW EXPOSURE FACTOR, Ce = 0.9
SNOW LOAD IMPORTANCE FACTOR, I = 1.0
THERMAL FACTOR, Ct = 1.2
MINIMUM SNOW LOAD FOR LOW-SLOPE ROOFS, Pm = 10 PSF
SLOPED ROOF SNOW LOADS, Ps = 7.6 PSF

WIND LOADS: PER PROVISIONS OF ASCE 7-16, DIRECTIONAL PROCEDURE: (WIND-BORNE DEBRIS REGION)
BASIC WIND SPEED (V) = 147 MPH
EXPOSURE CATEGORY = C
DIRECTIONALITY FACTOR Kd = 0.85
TOPOGRAPHY FACTOR Kzt = 1.00

BUILDING SEISMIC DESIGN CRITERIA:
A. RISK CATEGORY = II
B. SEISMIC IMPORTANCE FACTOR = 1.0
C. SITE CLASS D PER GEOTECHNICAL REPORT
D. DESIGN SPECTRAL ACCELERATIONS:
  SDS = 0.304g, SD1 = 0.172g
E. SEISMIC DESIGN CATEGORY C
F. SEISMIC-FORCE-RESISTING SYSTEMS:
  1. ORDINARY REINFORCED CONCRETE SHEAR WALLS:
    R = 5, OVERSTRENGTH = 2.5, Cd = 4.5
  2. BASE SHEAR:
    $V_{N\text{-}S}$ = 1104 KIPS, $V_{E\text{-}W}$ = 1104 KIPS
G. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD
H. STRUCTURAL IRREGULARITIES:
  HORIZONTAL STRUCTURAL IRREGULARITIES:
    1b. EXTREME TORSIONAL IRREGULARITIES X-DIRECTION

LIGHT GAUGE STEEL ROOF TRUSSES:
TC SUPERIMPOSED DEAD = 10 PSF (EXCLUDES TRUSS SELF-WEIGHT)
DC SUPERIMPOSED DEAD = 5 PSF (EXCLUDES TRUSS SELF-WEIGHT)
TC LL (ROOF) = 20PSF
TC MIN. SUPERIMPOSED DEAD = 5 PSF (EXCLUDES TRUSS SELF-WEIGHT)
BC MIN. SUPERIMPOSED DEAD = 0 PSF (EXCLUDES TRUSS SELF-WEIGHT)

USE TC MIN. AND BC MIN. WHEN COMPUTING UPLIFT ON TRUSSES.

Sheet List

Sheet Number	Sheet Name	Current Revision	Current Revision Date
S-001	General Notes	A	08.29.2025
S-100	Foundation Plan	A	08.29.2025
S-101	Level P1 & P1.5 Slab Plan	A	08.29.2025
S-102	Level P2 & P2.5 Framing Plan	A	08.29.2025
S-102R	Level P2 & P2.5 Reinforcing Plan	A	08.29.2025
S-103	Level P3 & P3.5 Framing Plan	A	08.29.2025
S-103R	Level P3 & P3.5 Reinforcing Plan		
S-104	Level P4 & P4.5 Framing Plan	A	08.29.2025
S-104R	Level P4 & P4.5 Reinforcing Plan	A	08.29.2025
S-105	Tower Framing Plans	A	08.29.2025
S-300	Foundation Sections & Details	A	08.29.2025
S-500	Shear Wall Schedule & Details	A	08.29.2025
S-501	Retaining Wall Schedules & Details	A	08.29.2025
S-502	Wall Sections		
S-505	Slab Sections	A	08.29.2025
S-506	Girder Schedule, Placing Diagram & Sections	A	08.29.2025
S-507	Beam Schedule & Placing Diagram	A	08.29.2025
S-508	Beam Sections		
S-510	Stair & Tower Sections	A	08.29.2025
S-515	Masonry Schedules, Sections & Details	A	08.29.2025
S-600	Concrete Column Schedule & Details	A	08.29.2025

CONCRETE NOTES:

1. CAST-IN-PLACE CONCRETE MIX DESIGNS SHALL BE AS NOTED:
   4000 PSI FOUNDATIONS
   5000 PSI COLUMNS, SUSPENDED SLAB & FRAMING, SHEAR WALLS, BARRIER COLUMNS AND WALLS
   4000 PSI RETAINING WALLS
   4000 PSI SLAB-ON-GRADE
2. BASE GROUT SHALL BE NON-SHRINK GROUT CONFORMING TO ASTM C1107. USE NON-METALLIC GROUT AT EXPOSED LOCATIONS.
3. NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
4. THE CONTRACTOR SHALL SUBMIT FOR APPROVAL DETAILED DRAWINGS SHOWING THE LOCATION OF ALL CONSTRUCTION JOINTS, CURBS, SLAB DEPRESSIONS, SLEEVES, OPENINGS, ETC. IN ALL CONCRETE WORK. STOP IN WORK SHALL BE MADE AT THE THIRD POINT OF SPAN WITH VERTICAL BULKHEADS. THERE SHALL BE NO HORIZONTAL CONSTRUCTION JOINTS IN SLAB AND BEAMS. PROVIDE KEYS IN SLABS OR BEAMS AT CONSTRUCTION JOINTS. ALL REINFORCING SHOWN ON THE DRAWINGS SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS.
5. ALL CONSTRUCTION JOINTS SHALL BE WIRE BRUSHED, CLEANED, AND COATED WITH CONCRETE BONDING AGENT IMMEDIATELY PRIOR TO PLACING NEW CONCRETE.
6. FORMS SHALL BE CAMBERED AS INDICATED. CAMBER SHALL NOT BE ACHIEVED BY ADDING THICKNESS TO SLAB OR BEAMS.
7. REFER TO ARCHITECTURAL DRAWINGS FOR CLIPS, GROOVES, GROUNDS, ETC. TO BE CAST IN CONCRETE AND FOR CONCRETE FINISHES.
8. HANGER INSERTS IN CONCRETE SLAB SHALL BE PLACED SO THAT 1" CONCRETE COVER OCCURS BETWEEN INSERT AND TOP OF SLAB.
9. CONDUIT OR PIPE SIZE (O.D.) SHALL NOT EXCEED 30% OF SLAB THICKNESS UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS. ALL CONDUITS OR PIPES LARGER THAN 1-3/8" DIAMETER (NOMINAL DIAMETER) OR GREATER THAN 30% OF THE SLAB THICKNESS SHALL BE PLACED UNDER THE SLAB. CONDUITS CAN BE GROUPED IN PAIRS. THE MINIMUM CLEAR DISTANCE BETWEEN SINGLE CONDUITS OR PAIRS SHALL BE 6".
10. SLEEVE PLUMBING OPENINGS IN CONCRETE WALLS AND SLABS BEFORE PLACING CONCRETE. ADJUST REINFORCING AT SLEEVES TO PROVIDE REQUIRED COVER TO REINFORCING. CORING IS NOT PERMITTED IN FLOOR SLABS, ROOF SLABS, COLUMNS, AND WALLS UNLESS PERMITTED BY STRUCTURAL ENGINEER.
11. ALL REINFORCING BARS, ANCHOR BOLTS, AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
12. CONTRACTOR SHALL PROVIDE TESTING AGENCY PROPER STORAGE FACILITIES FOR CONCRETE TEST CYLINDERS TO MAINTAIN CYLINDERS BETWEEN 60° AND 85° F AND IN A MOIST CONDITION.
13. SUBMIT SHOP DRAWINGS FOR FORMWORK SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE WORK WILL BE DONE.
14. PROVIDE 10 MIL VAPOR BARRIER BELOW ALL SLABS-ON-GRADE UNO.
15. AT ALL CONCRETE FOUNDATION WALLS, PROVIDE THE FOLLOWING ITEMS UNO: SELF-EXPANDING WATERSTOP AT ALL CONSTRUCTION JOINTS, DRAINAGE MAT, WATERPROOFING, AND FOUNDATION DRAINS.

REINFORCING STEEL NOTES:

1. REINFORCING BARS SHALL CONFORM TO THE FOLLOWING:
   A. DEFORMED BARS CONFORMING TO ASTM A615 OR A706, GRADE 60.
   B. COLD-ROLLED, DEFORMED BAR ANCHORS CONFORMING TO ASTM A496 WITH TENSILE STRENGTH OF 80,000 PSI AND YIELD STRENGTH OF 70,000 PSI.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 PLAIN WIRE OR A497 DEFORMED WIRE.
3. CONCRETE REINFORCING SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED, AND SPACED IN FORMS, AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318 AND THE LATEST EDITION OF THE "ACI DETAILING MANUAL", ACI SP-66.

   CHECKED SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING CONCRETE REINFORCING SIZES, SPACING, AND LOCATION SHALL BE SUBMITTED FOR APPROVAL. ALL REINFORCING SHOWN ON THE PLACEMENT DRAWINGS (PLANS, DETAILS, AND ELEVATIONS) SHALL HAVE A UNIQUE MARK AND SHALL BE LISTED SEPARATELY SHOWING LENGTHS, QUANTITY, AND BAR BENDING DETAILS.
4. CONTRACTOR SHALL NOT PLACE ANY REINFORCING UNTIL SHOP DRAWINGS, APPROVED BY THE STRUCTURAL ENGINEER ARE RECEIVED AT THE JOBSITE.
5. ALL WELDED WIRE FABRIC (WWF) SHALL BE LAPPED TWO (2) FULL MESH PANELS AND TIED SECURELY. WELDED WIRE FABRIC SHALL BE PROVIDED IN FLAT SHEETS, NOT ROLLS. SUPPORT WITH CHAIRS AT 4'-0" OC MAX.
6. WHERE VERTICAL WALL REINFORCING IS SPLICED AT TOP OF FOOTING, PROVIDE SPLICE BARS IN FOOTING SAME SIZE, GRADE, AND SPACING AS VERTICAL WALL REINFORCING, UNO. PROVIDE STANDARD HOOK IN FOOTING AND LAP WITH VERTICAL WALL REINFORCING AS NOTED ABOVE.
7. AT COLUMNS AND PIERS, WHERE VERTICAL REINFORCING IS SPLICED AT TOP OF FOOTING, PROVIDE SPLICE BARS IN FOOTING SAME SIZE, GRADE, AND QUANTITY AS VERTICAL COLUMN OR PIER REINFORCING. PROVIDE STANDARD HOOK IN FOOTING AND CLASS B LAP SPLICE WITH COLUMN OR PIER REINFORCING, UNO. MECHANICAL SPLICES ARE REQUIRED FOR #14 AND #18 BARS AND WHERE INDICATED.
8. MINIMUM CLEAR COVERAGE OF CONCRETE OVER OUTER MAIN REINFORCING BARS SHALL BE AS FOLLOWS, UNO:
   A. CONCRETE POURED DIRECTLY AGAINST EARTH: 3"
   B. STRUCTURAL SLABS: 1" TOP AND BOTTOM
   C. CONCRETE FORMED AGAINST EARTH: 2"
   D. WALLS WITH 2 LAYERS OF REINFORCING:
      INTERIOR FACE: 3/4" UNLESS IN CONTACT WITH GROUND
      EXTERIOR FACE: 2"
   E. WALLS WITH ONE LAYER OF REINF: PLACE VERTICAL BAR AT CENTER OF WALL AND HORIZ BAR ON ONE SIDE.
   F. COLUMNS (VERTICAL REINFORCING): 2" OR EQUAL TO THE BAR DIAMETER, WHICHEVER IS GREATER.
   G. BEAMS (TOP AND BOTT REINF): 2" OR EQUAL TO THE BAR DIAMETER, WHICHEVER IS GREATER.
9. FOR BEAMS AND SLABS, THE MINIMUM CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL BE THE DIAMETER OF THE BAR, 1-1/3 TIMES THE MAXIMUM AGGREGATE SIZE, BUT IN NO CASE LESS THAN 1". WHERE TWO OR MORE LAYERS OF REINFORCING ARE USED, PROVIDE #8 SPACERS AT 4'-0" ALONG BEAM. FOR COLUMNS AND WALLS, THE MINIMUM CLEAR DISTANCE BETWEEN BARS SHALL BE 1-1/2" BAR DIAMETERS. BUT IN NO CASE LESS THAN 1-1/2".
10. PLACEMENT OF REINFORCEMENT SHALL BE SUCH THAT ADEQUATE SPACE IS PROVIDED BETWEEN BARS TO ALLOW PASSAGE OF CONCRETE, VIBRATORS, ETC.
11. BARS SHALL BE IN CONTACT WHEN FORMING LAP SPLICES, UNO.
12. ALL REINFORCEMENT SHALL BE BENT COLD, UNO.
13. EQUIPMENT PADS SHALL BE REINFORCED AS FOLLOWS, UNO:
   A. 4" PADS: #3 AT 12" EACH WAY CENTERED IN SLAB THICKNESS.
   B. 6" PADS: #4 AT 12" EACH WAY CENTERED IN SLAB THICKNESS.
   C. 8" PADS: #4 AT 12" EACH WAY CENTERED IN SLAB THICKNESS.
14. ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS AS SHOWN ON DETAILS, ETC.
15. ALL SLAB ON GRADE REINFORCING SHALL BE SUPPORTED BY BAR CHAIRS AT 2'-0" OC MAX.

STRUCTURAL STEEL NOTES:

1. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING (UNO):
   A. ASTM A992, GRADE 50 (Fy = 50 KSI) FOR ALL WIDE FLANGE SECTIONS, CHANNELS.
   B. ASTM A500, GRADE C (Fy=50ksi MINIMUM) FOR ALL STRUCTURAL STEEL TUBING (NOTED HSS...)
   C. ASTM A53, TYPE S, GRADE B (Fy=35ksi) FOR STRUCTURAL STEEL PIPE. (NOTED PIPE)
   D. ASTM A572, GRADE 50 (Fy = 50 KSI) FOR ANGLES.
   E. ASTM A572, GRADE 50 (Fy = 50 KSI) FOR PLATES.

   WHERE STEEL MATERIAL IS NOT INDICATED, OR NOTED ABOVE STEEL SHALL BE A36.
2. ALL BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F3125 GR. A325 OR GR. A490.
   ALL NUTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A563.
   ALL WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F436.
   ALL BOLTS SHALL BE 3/4" DIAMETER UNO.
3. ALL WELDING ELECTRODES SHALL BE E70XX EXCEPT ELECTRODES FOR WELDING METAL DECK SHALL BE E60XX.
4. ALL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO AISC SPECIFICATIONS AND CODES, FOURTEENTH EDITION.
5. ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR'S ENGINEER. DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE GENERAL DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL ONLY AND DO NOT INDICATE THE REQUIRED NUMBER OF BOLTS OR WELD SIZES, UNO.
6. ALL CONNECTIONS SHALL BE SIMPLE SHEAR CONNECTIONS UTILIZING HIGH-STRENGTH BOLTS IN BEARING-TYPE CONNECTIONS WITH THREADS INCLUDED IN THE SHEAR PLANE IN SINGLE SHEAR. THE CAPACITIES SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
   A. CONNECTIONS FOR NON-COMPOSITE BEAMS SHALL BE ADEQUATE TO PROVIDE FOR END REACTIONS DUE TO MAXIMUM UNIFORMLY DISTRIBUTED LOAD FOR THAT SPAN BASED ON THE ALLOWABLE MOMENT CAPACITY OF THE BEAM, UNO. SPAN IS DEFINED AS COLUMN TO COLUMN SUPPORT FOR BRACED FRAMES WITH VERTICAL BRACES AT MID-SPAN.
   B. CONNECTIONS FOR COMPOSITE BEAMS SHALL BE DESIGNED FOR 1.40 TIMES THE CAPACITY FOR NON-COMPOSITE BEAMS, UNO.
   C. CONNECTION DESIGN SHALL BE DONE IN LRFD.
   D. THE MINIMUM CLIP ANGLE THICKNESS AT BEAMS AND GIRDERS ON COLUMN LINES SHALL BE 3/8" THICK. FULLY PRE-TENSION ALL BOLTS ON COLUMN LINES AND SNUG TIGHTEN REMAINDER.
7. THE MINIMUM NUMBER OF BOLTS PER CONNECTION SHALL BE TWO (2).
8. MINIMUM FILLET WELDS SHALL COMPLY WITH AISC BUT SHALL NOT BE LESS THAN 3/16", UNO.
9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS, ESPECIALLY WITH RESPECT TO STRUCTURAL STEEL FRAMING INTO MASONRY OR CONCRETE WALLS, BEAMS, OR COLUMNS.
10. ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS AND SHALL CONFORM TO THE AWS D1.1. "STRUCTURAL WELDING CODE - STEEL", CURRENT CONDITION.
11. ALL BEAMS AND JOISTS SHALL BE FABRICATED WITH A NATURAL CAMBER UP. PROVIDE CAMBERS AS INDICATED ON THE DRAWINGS. DESIGN NATURAL CAMBER = 1/2".
12. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
13. ALL ADDITIONAL STEEL REQUIRED BY THE CONTRACTOR FOR ERECTION PURPOSES AND SITE ACCESS OF STOCKPILED MATERIALS SHALL BE PROVIDED AT NO COST TO THE OWNER. ALL SUCH ADDITIONAL STEEL SHALL BE REMOVED BY THE CONTRACTOR UNLESS APPROVED BY THE OWNER IN WRITING.
14. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED. ALL GALVANIZED HSS MEMBERS SHALL HAVE MOISTURE REMOVED AND ALL VENT HOLES SEALED PRIOR TO BEING TRANSPORTED TO THE JOBSITE UNO. ALL ASSOCIATED GALVANIZED ASSEMBLIES SHALL HAVE MOISTURE REMOVED AND VENT HOLES IN END PLATES, CAP PLATES, AND BASE PLATES PERMANENTLY SEALED PRIOR TO BEING TRANSPORTED TO THE JOBSITE. ALL GALVANIZED FINISHES SHOULD BE REPAIRED AFTER SEALING.
15. FOR THE PURPOSES OF FIRE PROTECTION, ALL BEAMS ARE ASSUMED TO BE RESTRAINED CONSTRUCTION.

MASONRY NOTES:

1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO THE 2021 SC BUILDING CODE, CHAPTER 21 AND ACI 530/ASCE/TMS 402, LATEST EDITION, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
2. CONCRETE MASONRY UNITS SHALL CONFORM TO THE FOLLOWING:
   A. LOAD BEARING AND NON-LOAD BEARING MASONRY WALLS: ASTM C90 HOLLOW UNITS.
   B. CONCRETE BUILDING BRICK: ASTM C55
   C. FACE BRICK ASTM C216
3. MORTAR SHALL CONFORM TO ASTM C270:
   A. TYPE "S" MORTAR, UNO.
   B. TYPE "M" MORTAR, BELOW GRADE.
4. THE MINIMUM COMPRESSIVE STRENGTH FOR ALL MASONRY SHALL BE AS FOLLOWS, UNO: 6", 8" & 12" CMU - f'm = 1500 PSI
5. GROUT FOR ALL MASONRY SHALL CONFORM TO ASTM C476:
   A. PROVIDE FINE GROUT IN GROUT SPACES LESS THAN 2" IN ANY HORIZONTAL DIMENSION OR WHERE CLEARANCE BETWEEN REINFORCING AND MASONRY IS LESS THAN 3/4".
   B. PROVIDE COURSE GROUT IN SPACES 2" OR GREATER IN ALL HORIZONTAL DIMENSIONS PROVIDED THE CLEARANCE BETWEEN REINFORCING AND MASONRY IS NOT LESS THAN 3/4".
   C. THE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS FOR ALL GROUT SHALL BE 3000 PSI, UNO. SUBMIT GROUT MIX DESIGNS INCLUDING MANUFACTURER'S CERTIFICATION FOR MATERIALS USED PRIOR TO THE START OF ANY MASONRY WORK.
6. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS.
7. SPLICE LENGTHS FOR MASONRY REINFORCEMENT SHALL BE AS FOLLOWS, UNO:
   A. #4 BAR - 24"
   B. #5 BAR - 36"
   C. #6 BAR - 45" (SINGLE LAYER REINFORCING)
8. FOR BRICK EXPANSION AND CONTROL JOINTS, SEE ARCHITECTURAL DRAWINGS. FOR CMU EXPANSION AND CONTROL JOINTS, SEE STRUCTURAL DRAWINGS.
9. SEE SECTIONS & DETAILS FOR REINFORCING ARRANGEMENTS AND/OR SPECIAL CONDITIONS.

STEEL ROOF DECK:

1. ROOF DECK SHALL BE AS INDICATED ON DRAWINGS. SEE ROOF PLAN FOR GAUGE AND PROFILE DESIGNATION.
2. ROOF DECK SHALL BE PLACED SO AS TO COVER AT LEAST TWO SPANS. NO SINGLE SPAN CONDITIONS SHALL BE USED.
3. DECK SHALL BE FABRICATED SO THAT DECK RUNS CONTINUOUSLY OVER OPENINGS. THE OPENINGS IN THE DECK SHALL NOT BE CUT UNTIL THE OPENING IS NEEDED (PER OSHA).
4. STEEL DECK SHALL CONFORM TO ASTM A653 SQ GRADE 33 (Fy = 33,000 PSI) UNO.
5. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A924, WITH COATING DESIGNATION G60 UNLESS NOTED OTHERWISE.
6. SEE ROOF PLAN AND DETAILS FOR ROOF DECK ATTACHMENT. SEE SPECIFICATIONS FOR INSPECTION AND REPORTING REQUIRED ON ROOF DECK ATTACHMENT.
7. PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS. END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS.
8. THE CONTRACTOR SHALL COORDINATE ALL TRADE REQUIREMENTS AND CONFIRM THE SIZE AND LOCATION OF ALL OPENINGS. OPENINGS LARGER THAN 12", AND AS DETAILED, SHALL HAVE STEEL. FRAMING SUPPORTING ALL EDGES. SEE TYPICAL FRAMING DETAILS.
9. STEEL MEMBERS SUPPORTING STEEL DECK AT THE PERIMETER OF THE BUILDING SHALL BE CONTINUOUS, BUTT WELD PIECES WHERE SPLICES OCCUR.

SUBMITTAL NOTES:

1. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL. SUBMITTAL WITHOUT CONTRACTOR REVIEW WILL RESULT IN DELAYS. THE CONTRACTOR SHALL CONFIRM THAT SHOP DRAWINGS HAVE BEEN COMPLETED AND CHECKED BY THE SUPPLIER PRIOR TO SUBMISSION.
2. CONTRACTOR IS TO PROVIDE 3 COPIES OF SHOP DRAWINGS. 1 COPY EACH WILL BE RETAINED BY THE ARCHITECT AND ENGINEER AND THE REMAINDER WILL BE RETURNED TO THE CONTRACTOR. ADDITIONAL COPIES WILL BE RETURNED UNMARKED. CONTRACTOR SHALL MAKE ANY OTHER ADDITIONAL REQUIRED COPIES FROM THE MARKED-UP DRAWINGS. PREFERRED ALTERNATE: PROVIDE ELECTRONIC COPIES (PDF) OF CONTRACTOR REVIEWED AND STAMPED SHOP DRAWINGS FOR A/E REVIEW AND PROCESSING (IF APPROVED BY A/E).
3. SHOP DRAWING SUBMITTAL REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STRUCTURAL SPECIFICATIONS. CHANGES OR ADDITIONS MADE ON RESUBMITTED SHOP DRAWINGS SHALL BE CLEARLY INDICATED, AND THE PURPOSE OF THE RESUBMITTAL SHALL BE NOTED ON THE TRANSMITTAL. REVIEW OF RESUBMITTED SHOP DRAWINGS SHALL BE LIMITED SPECIFICALLY TO THE ITEMS NOTED FOR CORRECTION ON THE PREVIOUS SUBMITTAL.
4. THE GENERAL CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER SHOP DRAWINGS FOR THE FOLLOWING ITEMS:
   CONCRETE MIX DESIGNS
   CURTAIN WALL (a, c)
   COLD FORMED METAL FRAMING (a)
   REINFORCING STEEL
   STEEL ROOF DECK
   STEEL STAIRS/LADDERS (a, c)
   STRUCTURAL STEEL

   THE NOTATIONS FOLLOWING SUBMITTAL ITEMS INDICATE THE FOLLOWING:
   (a) CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA.
   (b) INCLUDE A CERTIFICATE OF COMPLIANCE WITH CONTRACT DOCUMENTS SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED AND RESPONSIBLE FOR THE DESIGN.
   (c) SUBMIT ONE COPY FOR INFORMATION AND RECORD ONLY.
5. MANUFACTURER'S LITERATURE: SUBMIT TWO COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION OF PROJECT.
6. THE ENGINEER'S REVIEW OF SHOP DRAWINGS IS FOR GENERAL CONFORMANCE OF THE DESIGN CONCEPT. CONTRACTOR SHALL SUBMIT A SCHEDULE OF SHOP DRAWING SUBMITTALS THAT IS ACCEPTABLE TO BOTH CONTRACTOR AND ENGINEER. AFTER THE CONTRACTOR HAS REVIEWED THE SHOP DRAWINGS, PROMPT REVIEW REVIEW BY THE ENGINEER WILL BE MADE OF ALL SUBMITTALS. FOR LARGE SUBMITTALS, REASONABLE REVIEW TIME SHALL BE ALLOWED AND MAY EXCEED TWO WEEKS. THE CONCURRENT SUBMITTAL OF MULTIPLE SHOP DRAWINGS ("DUMPING") WILL FURTHER EXTEND THE REVIEW PROCESS AND TIME FRAME NECESSARY TO PROPERLY REVIEW EACH SUBMITTAL.
7. REPRODUCTION OF THESE CONTRACT DOCUMENTS BY ANYONE FOR USE IN SHOP DRAWINGS SHALL SIGNIFY THEIR ACCEPTANCE OF ALL INFORMATION SHOWN AS BEING CORRECT. DESIGNSTRATEGIES, LLC SHALL BE INDEMNIFIED AND HELD HARMLESS FROM ALL CLAIMS, DAMAGES, LOSSES, EXPENSES OR LIABILITIES OF ANY KIND, INCLUDING ATTORNEY'S FEES. THE CONTRACTOR IS RESPONSIBLE FOR PROPER CHECKING AND COORDINATING OF DETAILS, DIMENSIONS, SIZES AND QUANTITIES AS REQUIRED TO FACILITATE COMPLETE AND ACCURATE FABRICATION AND ERECTION.

STRUCTURAL ABBREVIATIONS:

&.	AND
@	AT
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ANCH	ANCHOR, ANCHORS
ARCH	ARCHITECTURAL
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
B	BOTTOM
B/L	BUILDING LINE
BLDG	BUILDING
BM	BEAM
BOTT	BOTTOM
BRG	BEARING
CC	CENTER-TO-CENTER
CFMF	COLD-FORMED METAL FRAMING
CIV	CIVIL
CJ	CONTROL AND/OR CONSTRUCTION JOINT
CL	CL
C/L	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUE, CONTINUOUS
CONTR	CONTRACTOR
CTR	CENTER
DIA	DIAMETER
DP	DEEP
DT	DOUBLE TEE
DWG	DRAWING
DWGS	DRAWINGS
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMBED	EMBEDDED, EMBEDMENT
ENG	ENGINEER
EOD	EDGE OF DECK
EOS	EDGE OF SLAB
EOW	EDGE OF WALL
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EACH SIDE
ETC	ET CETERA
ETF	ELEVATION, TOP OF FOOTING
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
FAB	FABRICATOR
FDN	FOUNDATION
FIN	FINISH, FINISHED
FLR	FLOOR
FOM	FACE OF MASONRY
FS	FAR SIDE
FTG	FOOTING
FV	FIELD VERIFY
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
HDG	HOT-DIP GALVANIZED
HK	HOOK
HORIZ	HORIZONTAL
ID	INSIDE DIAMETER
IF	INSIDE FACE
IN.2	SQUARE INCHES
IN.3	INCHES CUBED
IN.4	INCHES TO THE FOURTH POWER
INFO	INFORMATION
INT	INTERIOR
JST	JOIST
JT	JOINT
KIPS	X 1000 POUNDS
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
LG	LONG
LW	LONG WAY
MATL	MATERIAL
MAX	MAXIMUM
MECH	MECHANICAL
MIN	MINIMUM
MTL	METAL
NF	NEAR FACE
NO	NUMBER
NS	NEAR SIDE
OC	ON CENTER
OPNG	OPENING
OD	OUTSIDE DIAMETER
PC	PRECAST
PCF	POUNDS PER CUBIC FOOT
PERP	PERPENDICULAR
PL	PLATE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST-TENSIONED
REF	REFERENCE, REFERENCED
REINF	REINFORCED, REINFORCING
REQ	REQUIRE, REQUIRED
REQ'D	REQUIRE, REQUIRED
SCHED	SCHEDULE
SHT	SHEET
SIM	SIMILAR
SL	SLOPE
SP	SPACE, SPACES
SQ	SQUARE
STAG	STAGGER
STD	STANDARD
STL	STEEL
SW	SHORT WAY
T	TOP
T/	TOP OF
THK	THICK
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/	WITH
WD	WOOD
WP	WORKING POINT

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CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN

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General Notes

S-001

Foundation Schedule

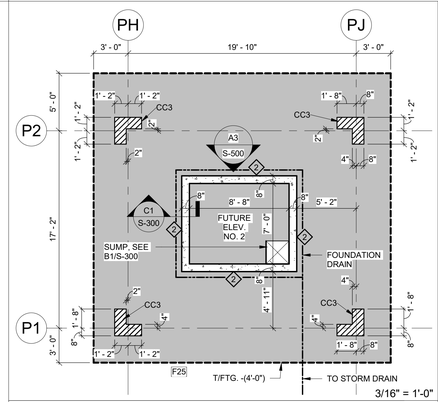
Mark	Type	Count	Bottom Reinf. Each Way	Top Reinf. Each Way	Comments
F4	4'-0" x 2'-0"" x 1'-0"	3	3-#5 Long Way 5-#5 Short Way		Foundation Designed for NET Bearing Pressure of 2500 PSF, Without Ground Improvements
F5.5	5'-6" x 5'-6" x 1'-3"	5	6-#5		
F6.5	6'-6" x 6'-6" x 1'-4"	3	8-#5		
F7	7'-0" x 7'-0" x1'-7"	6	7-#6		
F8	8'-0" x 8'-0" x 1'-7"	1	8-#6		
F10.5	10'-6" x 10'-6" x 2'-4"	7	11-#7		
F11	11'-0" x 11'-0" x 2'-6"	5	12-#7		
F12	12'-0" x 12'-0" x 2'-10"	4	12-#8		
F13	13'-0" x 13'-0" x 3'-0"	2	13-#8		
F25	25'-10" x 25'-2" x 3'-3"	2	#11@10"	#9@10"	180 Degree Hook Ends
F30.5	30'-6" x 16'-0" x 3'-6"	4	19-#11 Long Way #11@12" Short Way	19-#8 Long Way #8@10" Short Way	Hook Ends (90 DEGREES OR 180 DEGREES)
F45.75	45'-9" x 18'-0" x 3'-0"	2	16-#11 Long Way #11@9" Short Way	14-#9 Long Way #9@12" Short Way	Hook Ends (90 DEGREES OR 180 DEGREES)

NOTES:

1. CENTER ALL FOOTINGS ON COLUMN CENTER LINES, UNLESS NOTED OTHERWISE.

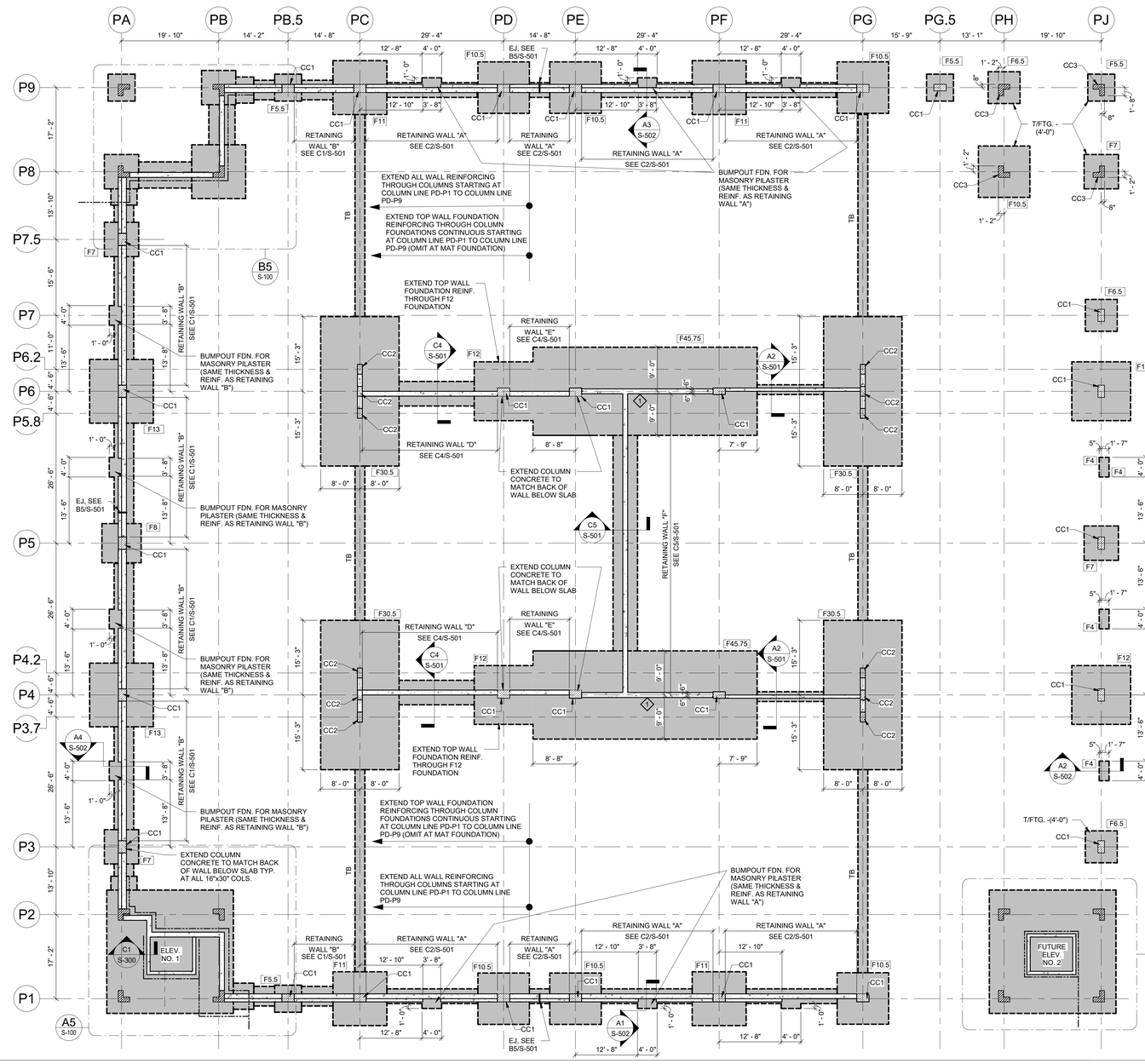
FOUNDATION PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. [X] INDICATES FOOTING MARK, SEE THIS DWG.
3. ALL PLUMBING, ETC. TO BE ABOVE TOP OF FOOTING, UNO.
4. ⟨X⟩ DENOTES SHEAR WALL TYPE, SEE DRAWING S-500.
5. T/FTG. EL. = -(2'-0") UNO.
   T/TB EL. SHALL MATCH T/FTG. ELEVATION THAT TIE BEAM IS FRAMED TO U.N.O.
6. TOP OF FOOTING ELEVATIONS REFERENCED FROM TOP OF P1 LEVEL SLAB-ON-GRADE HIGH POINT @ 0'-0". (ELEV. 31.5')
7. CCx DENOTES CONCRETE COLUMN, SEE DRAWINGS S-600.
8. TB DENOTES TIE BEAM, SEE A2 AND A4 ON S-300. NO GROUND IMPROVEMENT REQUIRED.
9. FOUNDATIONS ARE DESIGNED TO BEAR ON EXISTING SOILS REINFORCED WITH GROUND IMPROVEMENT, AND ARE DESIGNED FOR A NET ALLOWABLE BEARING PRESSURE OF 5000 PSF, WITH A FRICTION COEFFICIENT OF 0.36.
10. PRIOR TO INSTALLATION OF GROUND IMPROVEMENT, THE GROUND IMPROVEMENT DESIGNER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA.
11. THE FOUNDATIONS ARE DESIGNED TO RESIST UPLIFT AND OVERTURNING USING DEAD LOAD OF FOUNDATION ELEMENTS AND WEIGHT OF BACKFILL ABOVE. THERE ARE NO REQUIREMENTS FOR GROUND IMPROVEMENT ELEMENTS TO RESIST NET UPLIFT.
12. GROUND IMPROVEMENT ELEMENTS SHALL BE DESIGNED TO LIMIT SETTLEMENT AS FOLLOWS:
    MAX. TOTAL SETTLEMENT = 11/2"
    MAX. DIFFERENTIAL SETTLEMENT = 3/4"
13. TREAT SOIL WITH TERMITICIDE PER SPECIFICATION SECTION 313116-TERMITE CONTROL.



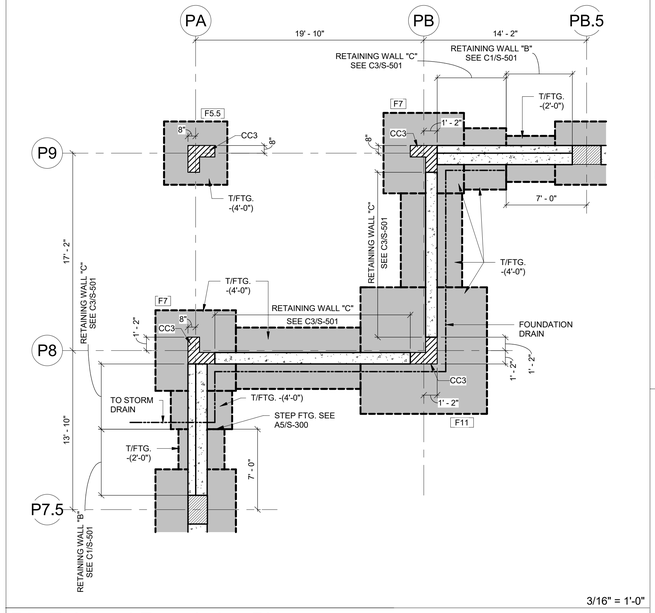
D5 Enlarged Foundation Plan - Stair #2

3/16" = 1'-0"



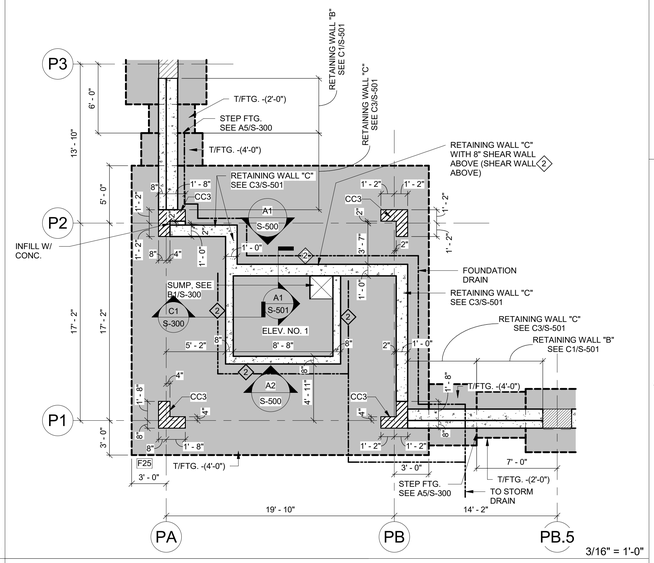
A1 Foundation Plan

3/32" = 1'-0"



B5 Enlarged Foundation Plan

3/16" = 1'-0"



A5 Enlarged Foundation Plan - Stair #1

3/16" = 1'-0"

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CAROLINA FOREST HOSPITAL

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN

TRUE NORTH PLAN NORTH

PROJECT NO: 0470.40

DATE: 11.21.2023

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Foundation Plan

S-100

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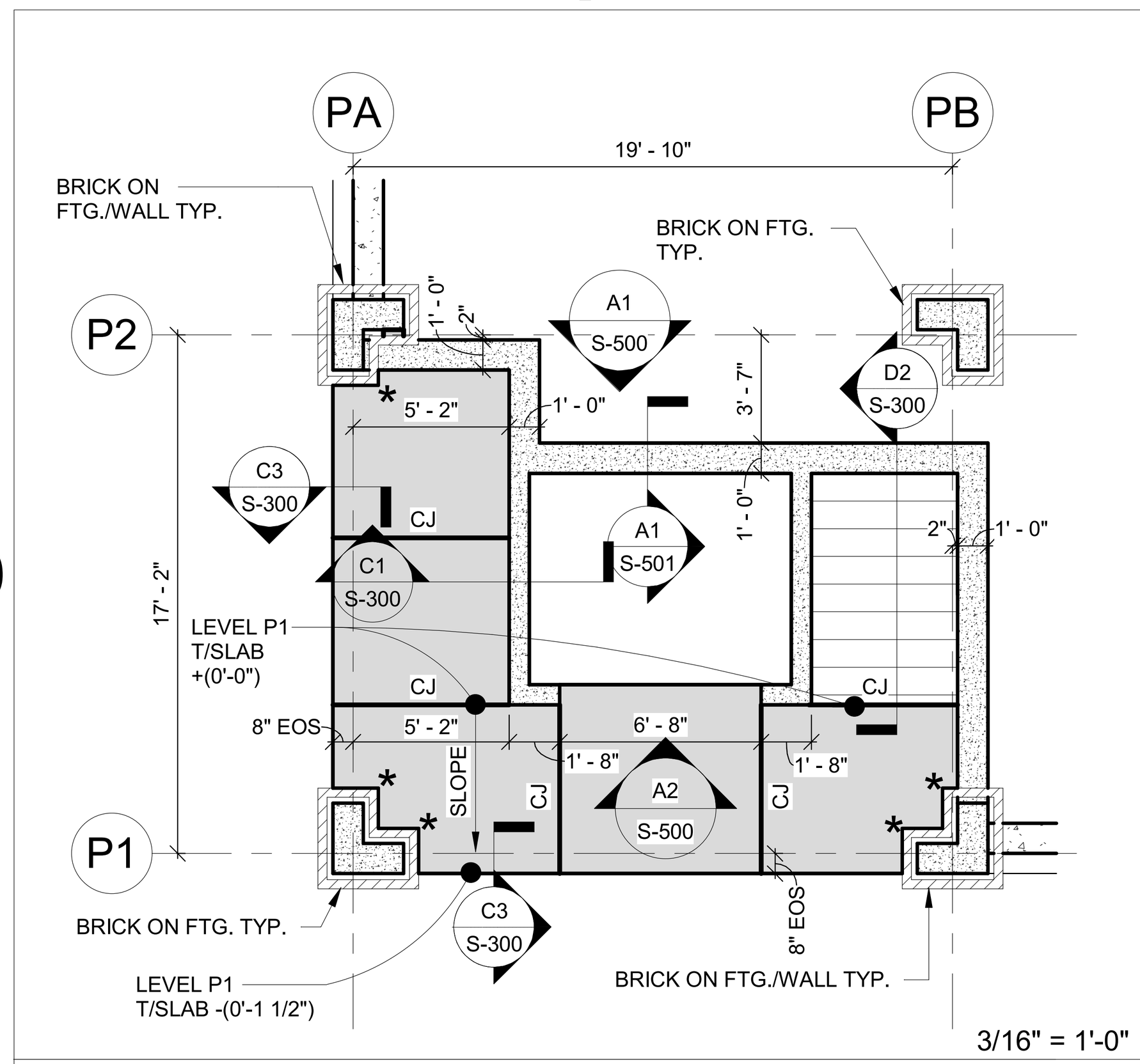
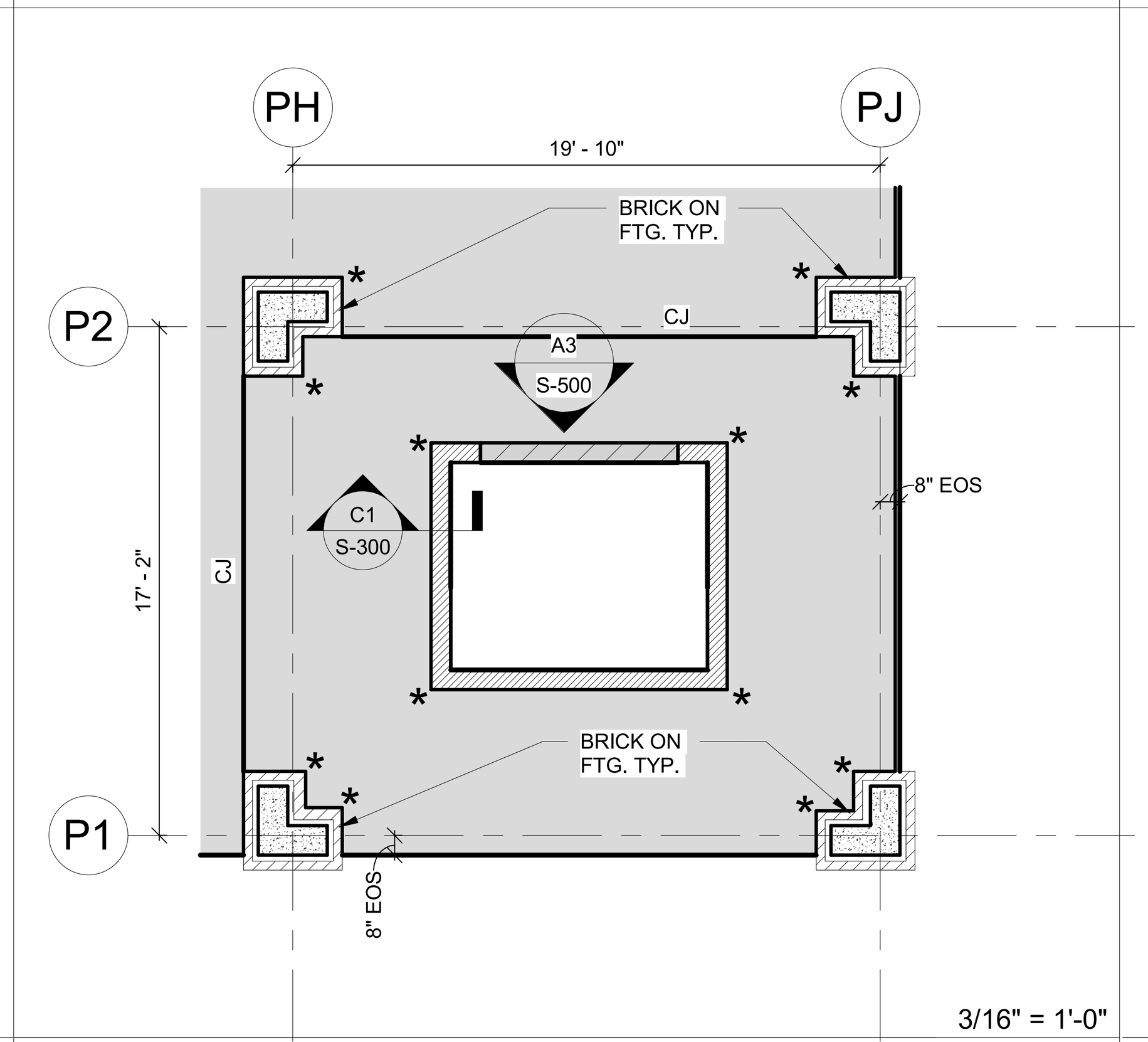
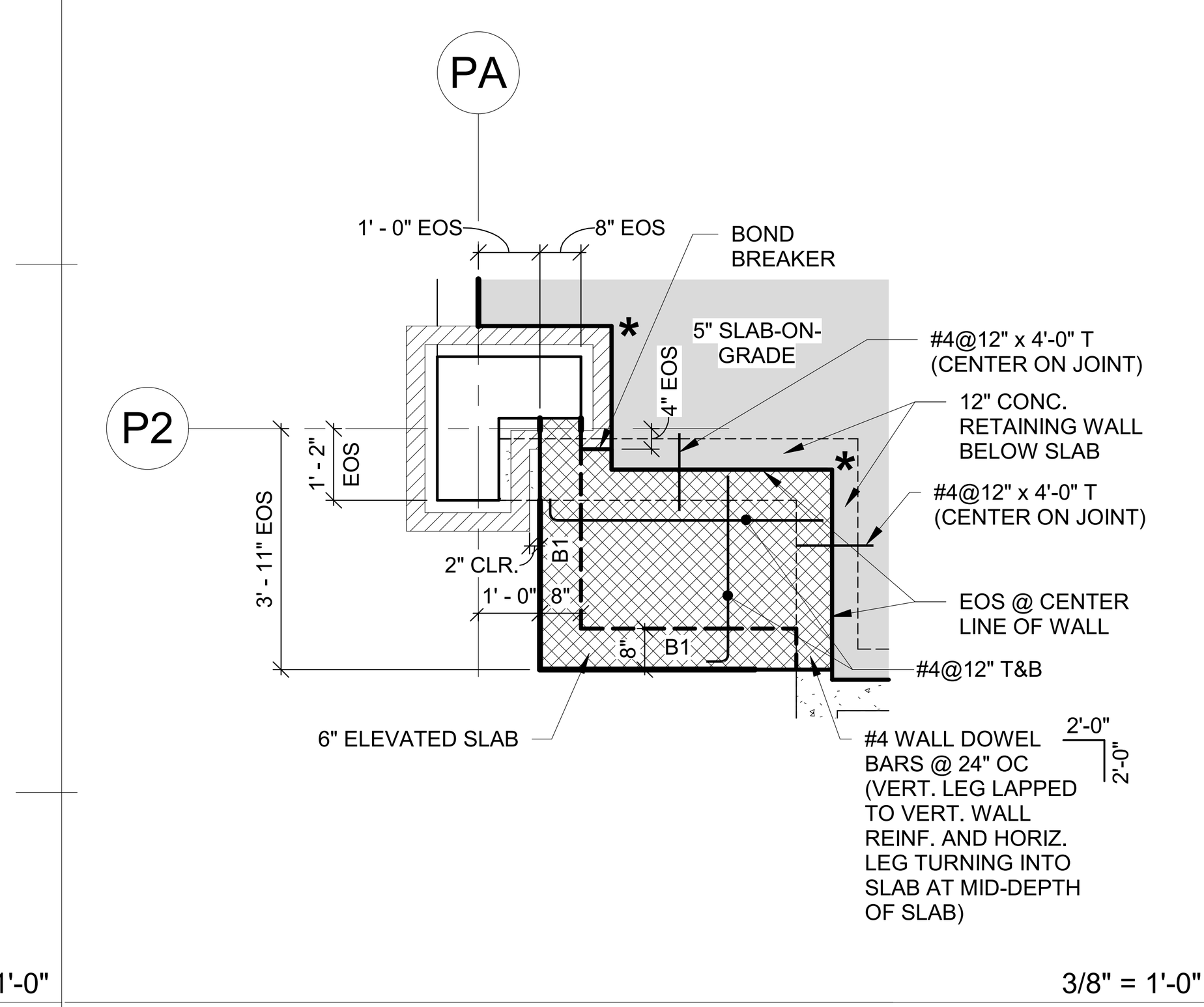


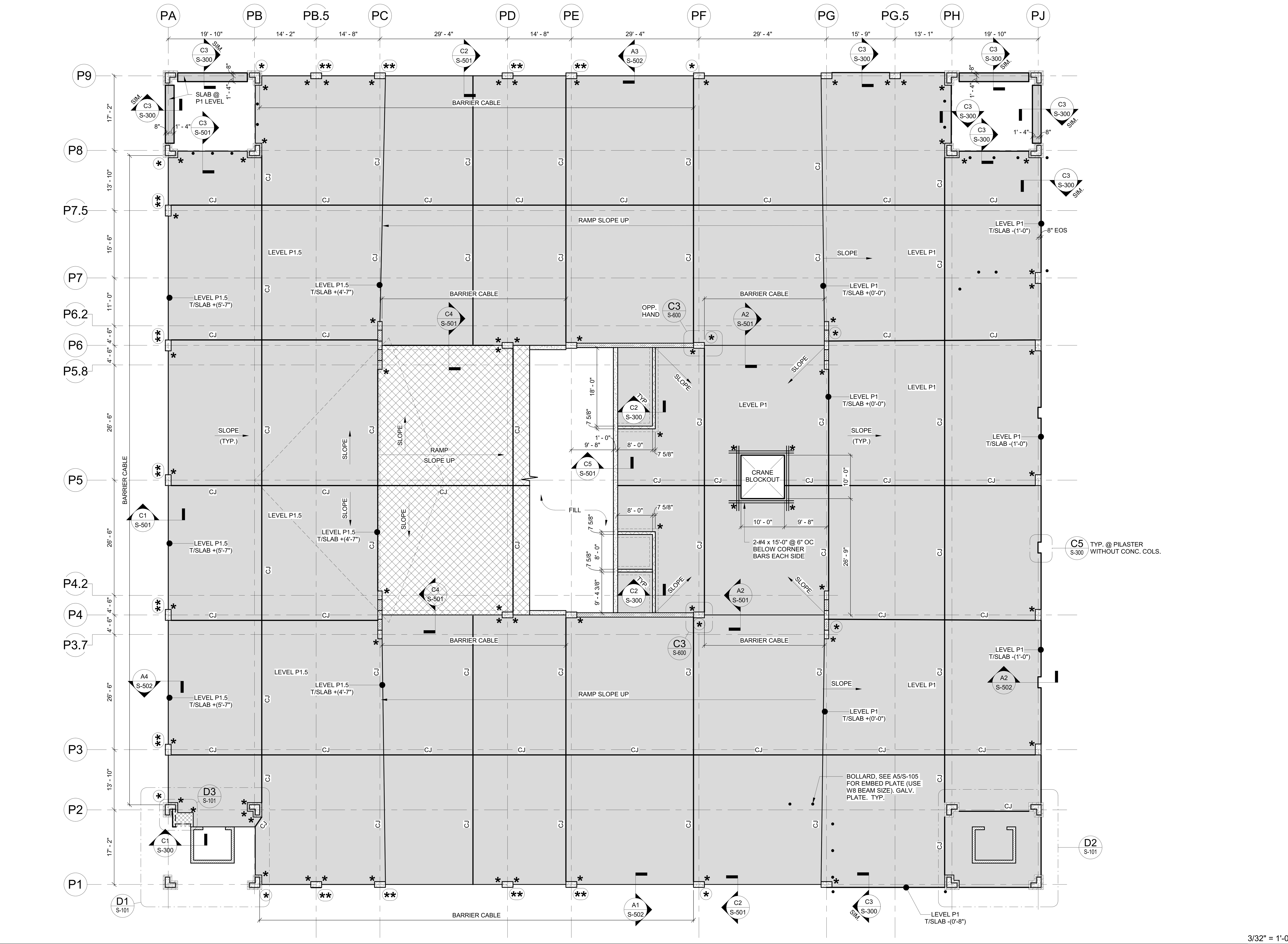
D1 Level P1 Slab Plan



D2 Level P1 Slab Plan



D3 Level P1.5 Slab Plan Enlarged



A1 Level P1 & P1.5 Slab Plan

SLAB PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. * DENOTES 2-#4 x 3'-0" CORNER BARS @ 1 1/2" FROM T/ SLAB.
3. REFERENCE ELEVATION +(0'-0") @ P1 LEVEL HIGH POINT (ELEV. 31.5'), SEE ARCHITECTURAL DRAWINGS FOR SLAB SLOPES.
4. SLAB-ON-GRADE CONSTRUCTION:
   DENOTES 5" NW CONC. SLAB (UNO) REINF. W/ #4@15" EACH WAY, 1 1/2" CLR. FROM T/SLAB. PLACE SLAB ON 4" OF COMPACTED GRADED AGGREGATE BASE COURSE (GABC) THAT MEETS THE MATERIAL & GRADATION REQUIREMENTS DEFINED BY SECTION 305 OF SCDOT STD SPECIFICATION FOR HWY CONSTRUCTION (2007).
5. CJ DENOTES CONTROL JOINT OR CONSTRUCTION JOINT. SEE DETAIL B3 & B4 ON S-300.
6. COORDINATE FLOOR DRAINS W/ ARCH., MECHANICAL & CIVIL DWGS.
7. REINF. ALL CONCRETE CURBING W/ WWF6x6-W2.9xW2.9 1" CLR. FROM T/CONC. USE LW CONCRETE AND PROVIDE BONDING AGENT. SEE ARCH. DWGS. FOR LOCATIONS.
8. CMU WALLS: REFER TO S-515 FOR MASONRY SCHEDULES, REINFORCING, AND DETAILS.
9. BARRIER CABLE NOTES:
   (*) DENOTES END ANCHORAGE LOCATION.
   (**) DENOTES COLUMN SLEEVES FOR CABLES. SEE (1) ON A4-S-600.

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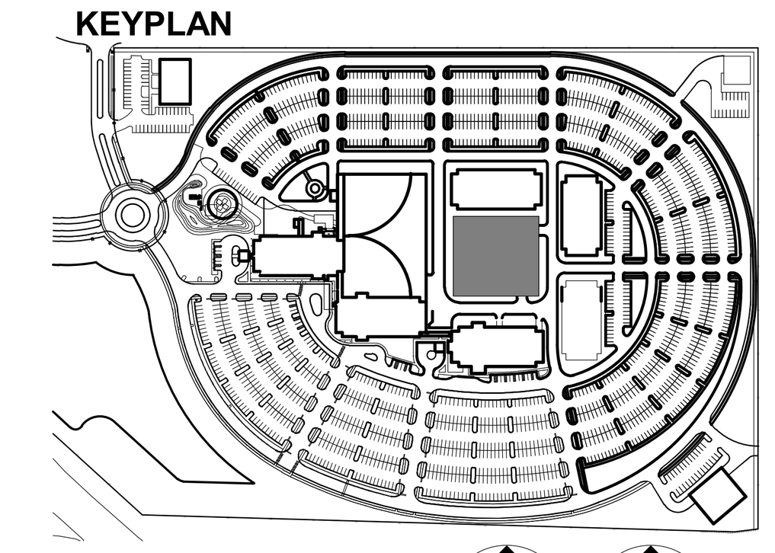
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



TRUE NORTH PLAN NORTH

SCALE: 1/8" = 1'-0"
UNLESS NOTED OTHERWISE

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Level P1 & P1.5 Slab Plan

S-101

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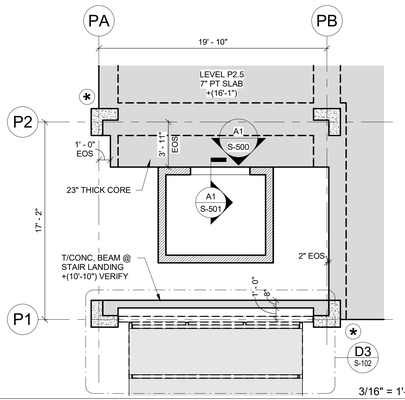
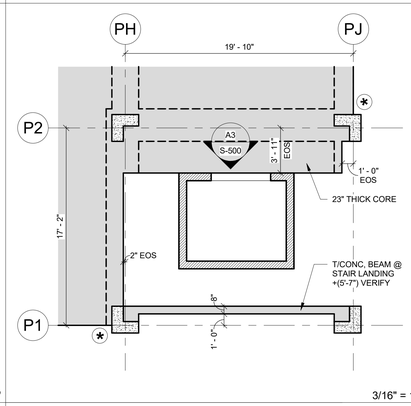


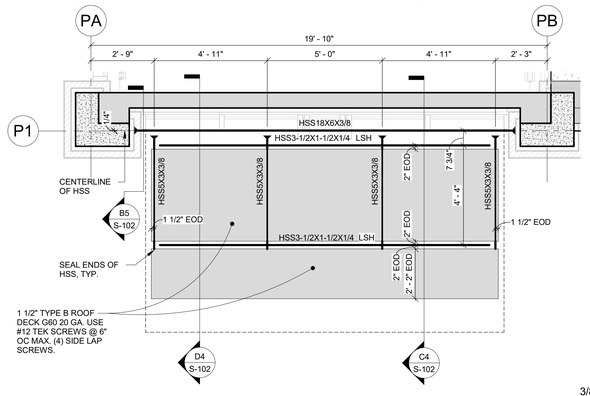
3/16" = 1'-0"

D1 Level P2.5 Framing Plan



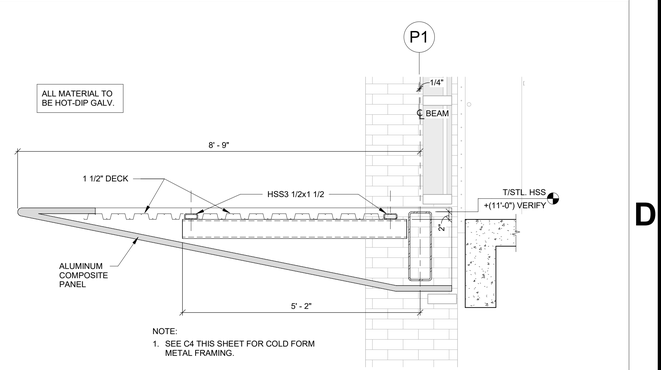
3/16" = 1'-0"

D2 Level P2 Framing Plan



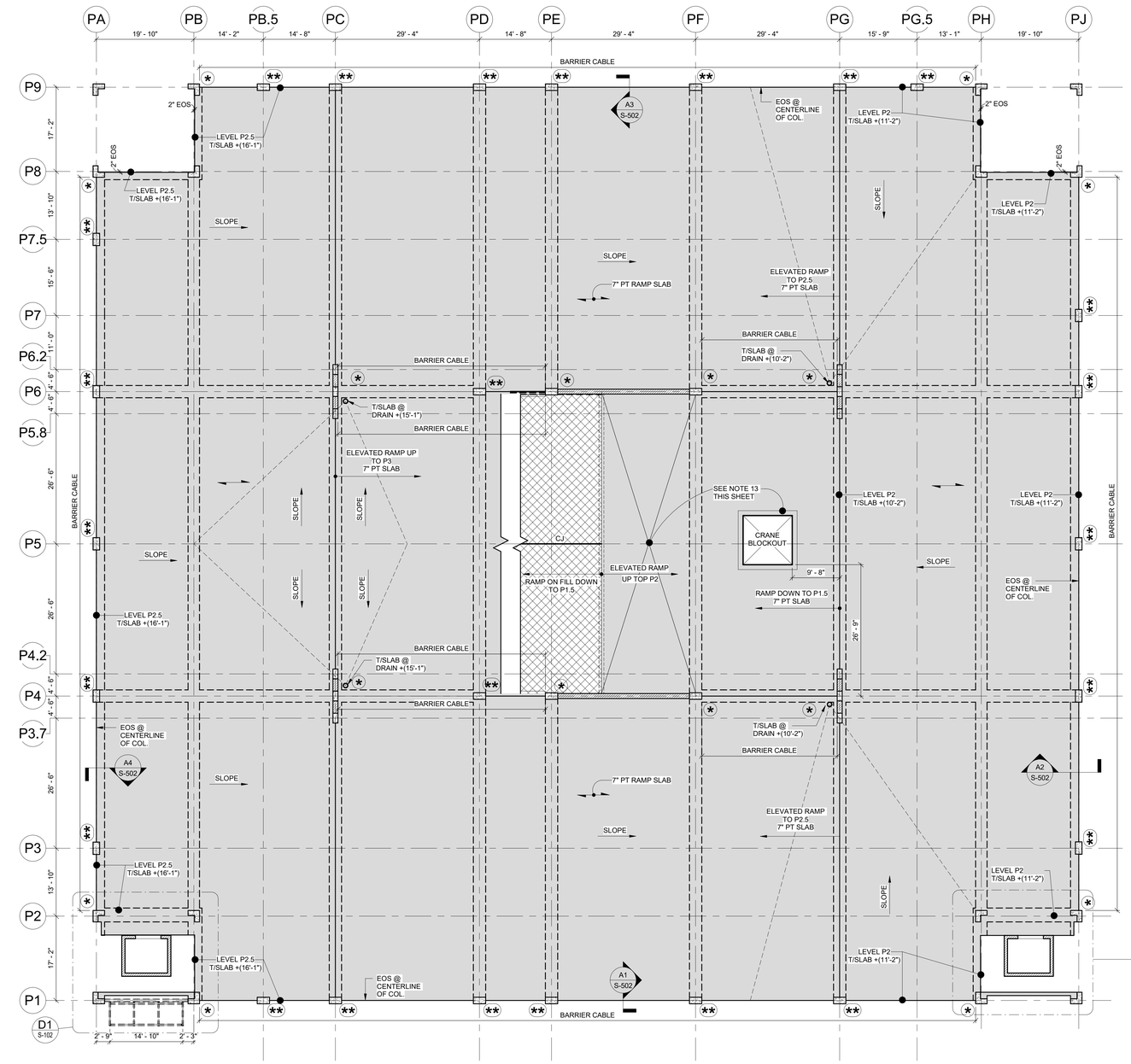
3/8" = 1'-0"

D3 Side Canopy Framing Plan



3/4" = 1'-0"

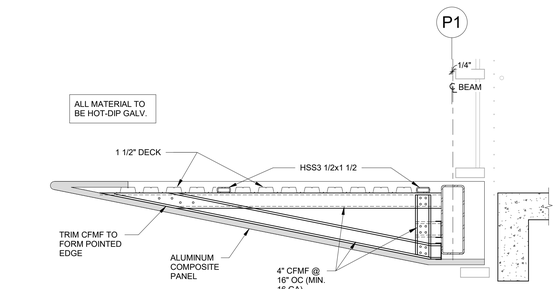
D4 Section At Canopy



3/32" = 1'-0"

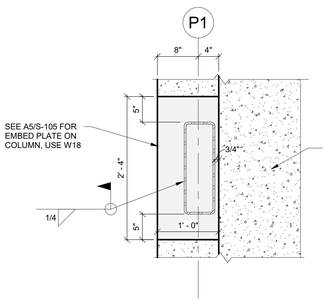
A1 Level P2 & P2.5 Framing Plan

NOTE: PROVIDE FLAT SURFACE AT DRAIN LOCATIONS TO ALLOW LEVEL INSTALLATION OF DECK DRAINS.



3/4" = 1'-0"

C4 Section At Canopy



1" = 1'-0"

B5 Section At HSS Embed Plate

LEVEL P2 & P2.5 FRAMING PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. DENOTES ONE-WAY POST-TENSIONED BEAM-SUPPORTED SLAB CONCRETE SYSTEM. SEE REINFORCING PLAN ON DRAWING S-102R.
3. DENOTES SPAN DIRECTION.
4. COORDINATE ALL DRAINS, SLOPES, DEPRESSIONS, ETC. FOR SUSPENDED CONCRETE FRMG W/ ARCH AND MECH DWGS.
5. CJ DENOTES CONTROL JOINT OR CONSTRUCTION JOINT. SEE DETAIL B3 & B4 ON S-300.
6. SLAB-ON-GRADE CONSTRUCTION:
   DENOTES 5" NW CONCRETE SLAB (UNO) REINFORCE WITH #4@15" EACH WAY, 1 1/2" CLR. FROM T/SLAB. PLACE SLAB ON 4" OF COMPACTED GRADED AGGREGATE BASE COURSE (GABC) THAT MEETS THE MATERIAL AND GRADATION REQUIREMENTS DEFINED BY SECTION 305 OF SCDOT STD SPECIFICATION FOR HIGHWAY CONSTRUCTION (2007).
7. REFERENCE ELEVATION +(0'-0") @ P1 LEVEL HIGH POINT
8. REINF. ALL CONCRETE CURBING W/ WWF6x6-W2.9xW2.9 1" CLR. FROM T/CONC. USE LW CONCRETE AND PROVIDE BONDING AGENT. SEE ARCH. DWGS. FOR LOCATIONS.
9. PLEASE NOTE THAT DUE TO RESTRAINT BY THE SHEARWALLS AND OTHER WALLS, THE POST-TENSIONED SLABS ARE VERY LIKELY TO EXHIBIT SOME SHORTENING AND/OR RESTRAINT CRACKING WHICH SHOULD NOT IMPAIR THE SLAB'S STRUCTURAL INTEGRITY. RESTRAINT CRACKS ARE ALSO VERY LIKELY TO OCCUR IN SHEAR WALLS & OTHER WALLS.
10. BARRIER CABLE NOTES:
    (*) DENOTES END COLUMN ANCHORAGE LOCATION.
    (**) DENOTES COLUMN SLEEVED FOR CABLES - SEE (1) ON A4/S-600.
11. CONDUITS EMBEDED IN SUSPENDED SLABS OR WALLS SHALL SATISFY THE FOLLOWING:
    A. CONDUIT OUTSIDE DIMENSION SHALL NOT BE LARGER THAN 1/3 THE OVERALL THICKNESS OF SLAB, WALL, OR BEAM IN WHICH THEY ARE EMBEDED.
    B. CONDUIT SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.
    C. CONCRETE COVER SHALL BE 1 1/2".
    D. INSTALLATION OF CONDUIT SHALL NOT REQUIRE REINFORCEMENT TO BE CUT, BENT OR DISPLACED FROM PROPER LOCATION.
12. TOWER CRANE BLOCKOUT SIZE TO BE PROVIDED BY CONTRACTOR. DESIGN IS BASED ON 10' SQUARE OPENINGS.
13. PROVIDE WATERPROOFING MEMBRANE BY TREMCO, VULKEM 350/950NF/950NF (1-800-321-7906). EXTEND 1'-0" MINIMUM EACH SIDE OF CRANE BLOCKOUT.

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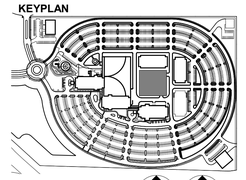
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



TRUE NORTH PLAN NORTH

0 4' 8' 16'
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Level P2 & P2.5 Framing Plan

S-102

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SLAB MISC REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
E1	ADD 4-#5 (2 TOP + 2 BOTTOM) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E2	ADD 2-#5 (2 TOP) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E3	ADD 3-#5 (3 TOP) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E4	ADD 6-#5 (3 TOP + 3 BOTTOM) PLACE AT ALL EDGES OF 23" SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E5	4-#4 BUNDLED BARS x 20'-0" LAP 2'-0" WITH HORIZONTAL SHEAR WALL REINF.
H1	#4 BENT-BAR @14" (3'-0" BENT LEG TOP, 1'-6" LEG BOTTOM, 8" LEG VERTICAL)
H2	#4 BENT-BAR @14" (3'-0" BENT LEG TOP, 8" HOOK VERTICAL)
H3	#5 U-BAR @12" (10'-0" LEG TOP, 10'-0" LEG BOTTOM, 5" LEG VERTICAL) PLACE AROUND VERTICAL SHEAR WALL REINF.
H4	#4 BENT-BAR @14" (3'-6" BENT LEG TOP, 8" HOOK VERTICAL)

SLAB TOP REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
T1	#4 x 16'-0" @ 12" TOP
T2	#4 x 6'-0" @ 12" TOP (HOOK)
T3	#4 x 13'-6" @ 12" TOP
T4	#4 x 7'-6" @ 12" TOP (HOOK)
T5	#5 x 13'-6" @ 7" TOP
T6	#7 x 11'-0" @ 14" TOP (HOOK)
T7	#6 x 23'-0" @ 12" TOP (BEND AT RAMP START/END)
T8	(6) #4 x 12'-0" @ 6" O.C. TOP
T9	#4 x 6'-0" @ 6" TOP (HOOK)
T10	#4 x 22'-0" @ 6" TOP (BEND AT RAMP START/END)
T11	#4 x 13'-6" @ 8" TOP O.C.
T12	#4 x 11'-0" @ 6" O.C. TOP (HOOK)

SLAB BOTTOM REBAR SCHEDULE

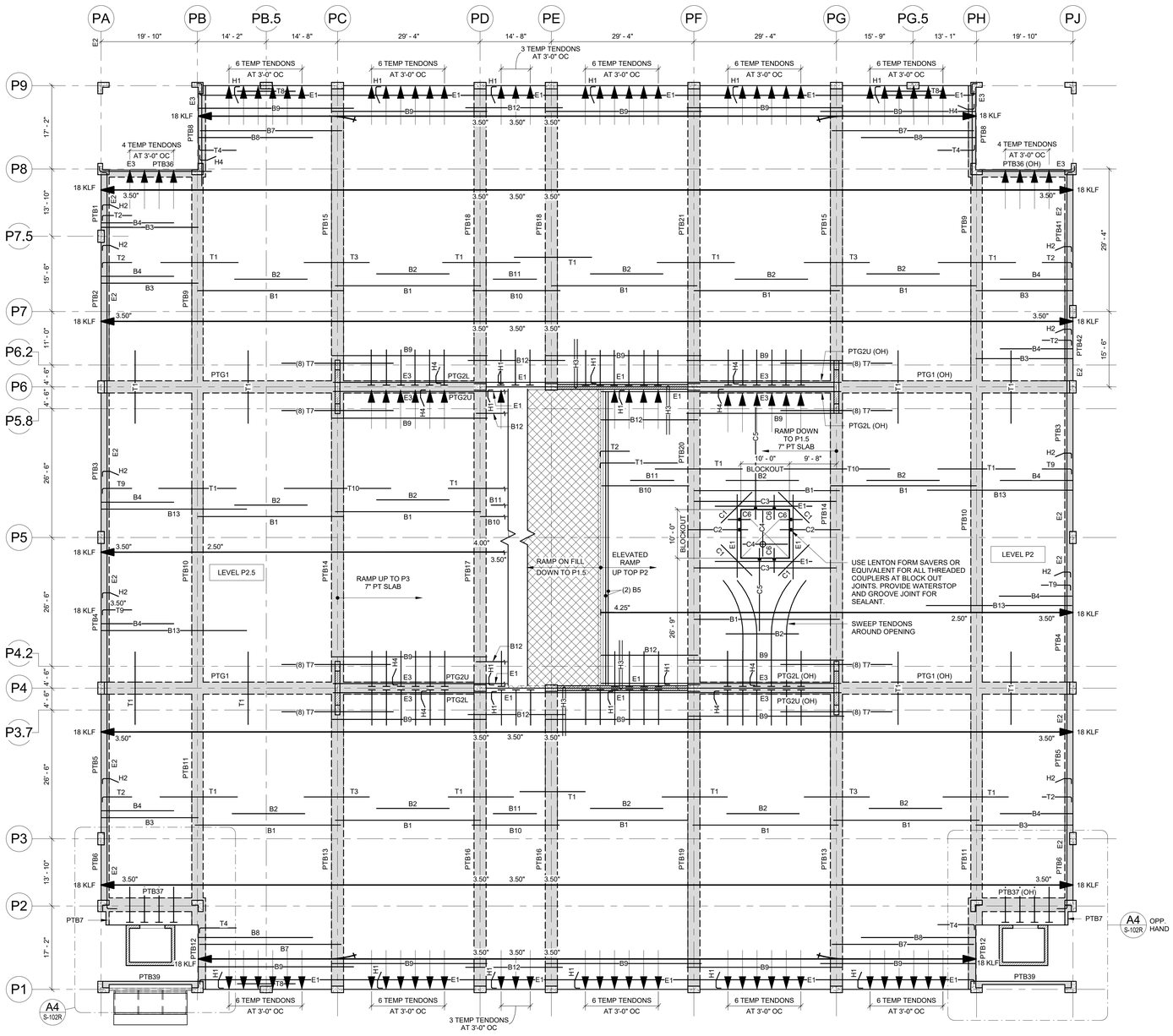
MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
B1	#4 x 30'-0" @ 36" BOTTOM (LONG)
B2	2-#4 x 15'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B3	#4 x 20'-0" @ 36" BOTTOM (LONG)
B4	2-#4 x 15'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B5	#4 x CONT. @ 14" BOTTOM
B6	#7 x CONT. @ 14" BOTTOM (EACH WAY WITHIN 23" THICK SLAB, HOOK AT SHEARWALLS)
B7	#4 x 31'-0" @ 36" BOTTOM (LONG)
B8	2-#4 x 23'-6" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B9	10-#4 x 32'-0" @ 12" BOTTOM (PLACE ALONG RAMP EDGES IN LIEU OF 'B1' & 'B2')
B10	#4 x 18'-0" @ 36" BOTTOM LONG
B11	2-#4 x 9'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 14")
B12	10-#4 x 20'-0" @ 12" BOTTOM (PLACE ALONG RAMP EDGES IN LIEU OF 'B10' & 'B11')
B13	#4 x 30'-0" @ 12" BOTTOM (LONG)

CRANE REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
C1	4-#4 x 8'-0" @ 7" MID-DEPTH (PLACE DIAGONALLY AT CORNERS SHOWN)
C2	#4 x 13'-0" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)
C3	5-#4 x 32'-0" @ 7" BOTTOM (PLACE ALONG OPENING EDGE IN LIEU OF 'B1' & 'B2')
C4	#4 x 6'-3" @ 7" BOTTOM (PLACE EACH WAY IN CRANE OPENING INFILL)
C5	#4 x 19'-3" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)
C6	#4 x 6'-3" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)

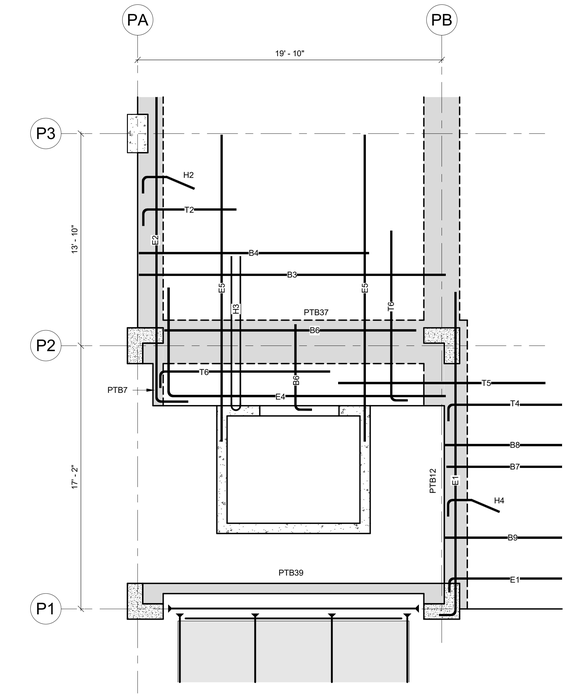
REINFORCING NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. TYPICAL SLAB SHALL BE 7" THICK POST-TENSIONED CONCRETE SLAB EXCEPT 23" THICK CORE AREAS. CONCRETE STRENGTH SHALL BE 3000 PSI AT TIME OF STRESSING AND 5000 PSI MINIMUM AT 28 DAYS. TYPICAL 3" CONCRETE WASH AT SLAB EDGES PER ARCHITECT ARE ADDED MONOLITHICALLY TO SLAB THICKNESSES GIVEN.
3. ELEVATED POST-TENSIONED SLABS SHALL BE AN ENCAPSULATED SYSTEM WITH POST-TENSIONED TENDONS THAT CONSIST OF 1/2" DIA. 270 KSI UNBONDED WIRE TENDONS. SLAB POST-TENSIONING FORCES GIVEN ARE EFFECTIVE PRESTRESS FORCES AFTER ALL SHORT AND LONG-TERM LOSSES AS SUBSTANTIATED BY CALCULATIONS PROVIDED BY THE PT SUPPLIER'S ENGINEER. IN NO CASE SHALL GREATER THAN 27 KIPS PER TENDON BE USED TO DETERMINE TENDON QUANTITIES PROVIDED. ADDED TENDONS EXTEND 6'-0" PAST SUPPORT CENTERLINES. ALL PT TENDONS MUST BE STRESSED WITHIN 96 HOURS OF CASTING CONCRETE.
4. UNLESS NOTED OTHERWISE ON PLAN, FOR UNIFORM SLAB TENDONS, PROVIDE SIMPLE-PARABOLIC TENDON PROFILES WITH HIGHPOINTS OF 5.75" AT EACH FACE OF SUPPORTING BEAM OR GIRDER, LOWPOINTS OF 1.00" AT MIDSPAN, AND ENDPOINTS OF 3.50" AT E.O.S. AND FACE OF EXTERIOR BEAM (IF ANY). FOR TEMPERATURE SLAB TENDONS, PROVIDE A STRAIGHT PROFILE WITHIN THE MIDDLE-THIRD OF THE SLAB DEPTH, AND ENDPOINTS OF 3.50" AT E.O.S. AND FACE OF EXTERIOR BEAMS (IF ANY).
5. REBAR SHALL BE GRADE 60 KSI WITH STANDARD CLASS B LAPS AND STANDARD HOOKS PLACED 2" CLEAR OF EDGE OF SLAB U.N.O. USE STIRRUP BENDS FOR BEAM STIRRUPS AND SMALL SECTION BENDS.
6. FOR INTERIOR SPANS, CENTER SLAB TOP BARS ON BEAM CENTER AND CENTER SLAB BOTTOM BARS ON MIDSPAN UNLESS SHOWN OTHERWISE. FOR EXTERIOR SPANS, START SLAB TOP REBAR HOOKS AND SLAB BOTTOM BARS AT 2" OFF E.O.S.
7. ALL #4 SLAB TOP BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS AT MIDSPAN. ALL #4 SLAB BOTTOM BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS AT BEAM CENTERS: ALL #4 TEMPERATURE BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS.
8. ALL SLAB REBAR COVER SHALL BE 1" CLEAR TOP & 3/4" CLEAR BOTTOM.
9. SEE BEAM & GIRDER REBAR SCHEDULE SHEETS FOR ALL BEAM & GIRDER DETAILS, SCHEDULE AND NOTES THAT CORRESPOND TO BEAM & GIRDER MARKS THIS SHEET.



A1 Level P2 & P2.5 Reinforcing Plan

3/32" = 1'-0"



A4 P2 Level Reinforcing Plan Enlarged

1/4" = 1'-0"

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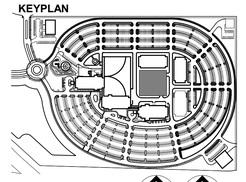
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



TRUE NORTH PLAN NORTH

PROJECT NO: 0470.62

DATE: 00.00.2025

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Level P2 & P2.5 Reinforcing Plan

S-102R

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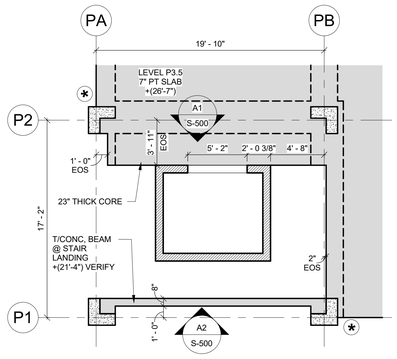
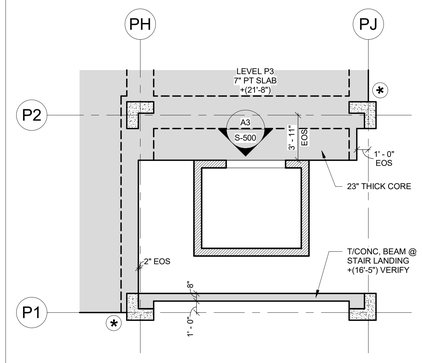


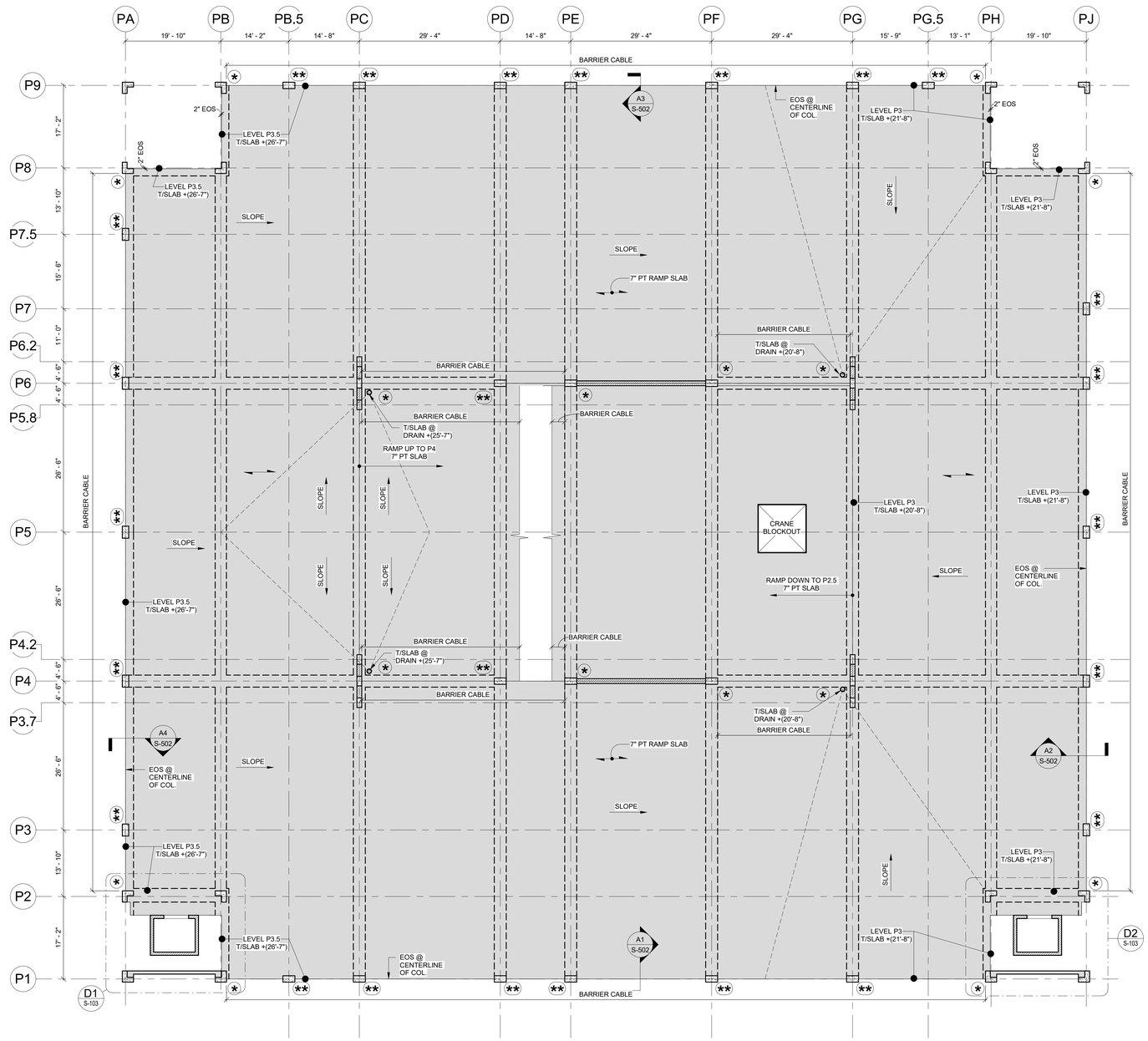
3/16" = 1'-0"

D1 Level P3.5 Framing Plan



3/16" = 1'-0"

D2 Level P3 Framing Plan



3/32" = 1'-0"

A1 Level P3 & P3.5 Framing Plan

NOTE: PROVIDE FLAT SURFACE AT DRAIN LOCATIONS TO ALLOW LEVEL INSTALLATION OF DECK DRAINS.

LEVEL P3 & P3.5 FRAMING PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. DENOTES ONE-WAY POST-TENSIONED BEAM-SUPPORTED SLAB CONCRETE SYSTEM. SEE REINFORCING PLAN ON DRAWING S-103R.
3. DENOTES SPAN DIRECTION.
4. COORDINATE ALL DRAINS, SLOPES, DEPRESSIONS, ETC. FOR SUSPENDED CONCRETE FRMG W/ ARCH AND MECH DWGS.
5. REFERENCE ELEVATION +(0'-0") @ P1 LEVEL HIGH POINT
6. REINF. ALL CONCRETE CURBING W/ WWF6x6-W2.9xW2.9 1" CLR. FROM T/CONC. USE LW CONCRETE AND PROVIDE BONDING AGENT. SEE ARCH. DWGS. FOR LOCATIONS.
7. PLEASE NOTE THAT DUE TO RESTRAINT BY THE SHEARWALLS AND OTHER WALLS, THE POST-TENSIONED SLABS ARE VERY LIKELY TO EXHIBIT SOME SHORTENING AND/OR RESTRAINT CRACKING WHICH SHOULD NOT IMPAIR THE SLAB'S STRUCTURAL INTEGRITY. RESTRAINT CRACKS ARE ALSO VERY LIKELY TO OCCUR IN SHEAR WALLS & OTHER WALLS.
8. BARRIER CABLE NOTES:
   - (*) DENOTES END COLUMN ANCHORAGE LOCATION.
   - (**) DENOTES COLUMN SLEEVED FOR CABLES - SEE (1) ON A4/S-600.
9. CONDUITS EMBEDED IN SUSPENDED SLABS OR WALLS SHALL SATISFY THE FOLLOWING:
   - A. CONDUIT OUTSIDE DIMENSION SHALL NOT BE LARGER THAN 1/3 THE OVERALL THICKNESS OF SLAB, WALL, OR BEAM IN WHICH THEY ARE EMBEDED.
   - B. CONDUIT SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.
   - C. CONCRETE COVER SHALL BE 1 1/2".
   - D. INSTALLATION OF CONDUIT SHALL NOT REQUIRE REINFORCEMENT TO BE CUT, BENT OR DISPLACED FROM PROPER LOCATION.
10. TOWER CRANE BLOCKOUT SIZE TO BE PROVIDED BY CONTRACTOR. DESIGN IS BASED ON 10' SQUARE OPENINGS.

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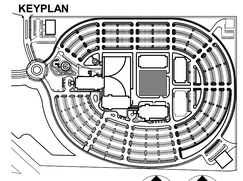
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



TRUE NORTH PLAN NORTH

0 4' 8' 16'
SCALE: 1/8" = 1'-0"
UNLESS NOTED OTHERWISE

PROJECT NO: 0470.62

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Level P3 & P3.5 Framing Plan

S-103

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SLAB MISC REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
E1	ADD 4-#5 (2 TOP + 2 BOTTOM) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E2	ADD 2-#5 (2 TOP) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E3	ADD 3-#5 (3 TOP) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E4	ADD 6-#5 (3 TOP + 3 BOTTOM) PLACE AT ALL EDGES OF 23" SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E5	4-#4 BUNDLED BARS x 20'-0" LAP 2'-0" WITH HORIZONTAL SHEAR WALL REINF.
H1	#4 BENT-BAR @14" (3'-0" BENT LEG TOP, 1'-6" LEG BOTTOM, 8" LEG VERTICAL)
H2	#4 BENT-BAR @14" (3'-0" BENT LEG TOP, 8" HOOK VERTICAL)
H3	#5 U-BAR @12" (10'-0" LEG TOP, 10'-0" LEG BOTTOM, 5" LEG VERTICAL) PLACE AROUND VERTICAL SHEAR WALL REINF.
H4	#4 BENT-BAR @14" (3'-6" BENT LEG TOP, 8" HOOK VERTICAL)

SLAB TOP REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
T1	#4 x 16'-0" @ 12" TOP
T2	#4 x 6'-0" @ 12" TOP (HOOK)
T3	#4 x 13'-6" @ 12" TOP
T4	#4 x 7'-6" @ 12" TOP (HOOK)
T5	#5 x 13'-6" @ 7" TOP
T6	#7 x 11'-0" @ 14" TOP (HOOK)
T7	#6 x 23'-0" @ 12" TOP (BEND AT RAMP START/END)
T8	(6) #4 x 12'-0" @ 6" O.C. TOP
T9	#4 x 6'-0" @ 6" TOP (HOOK)
T10	#4 x 22'-0" @ 6" TOP (BEND AT RAMP START/END)
T11	#4 x 13'-6" @ 8" TOP O.C.
T12	#4 x 11'-0" @ 6" O.C. TOP (HOOK)

SLAB BOTTOM REBAR SCHEDULE

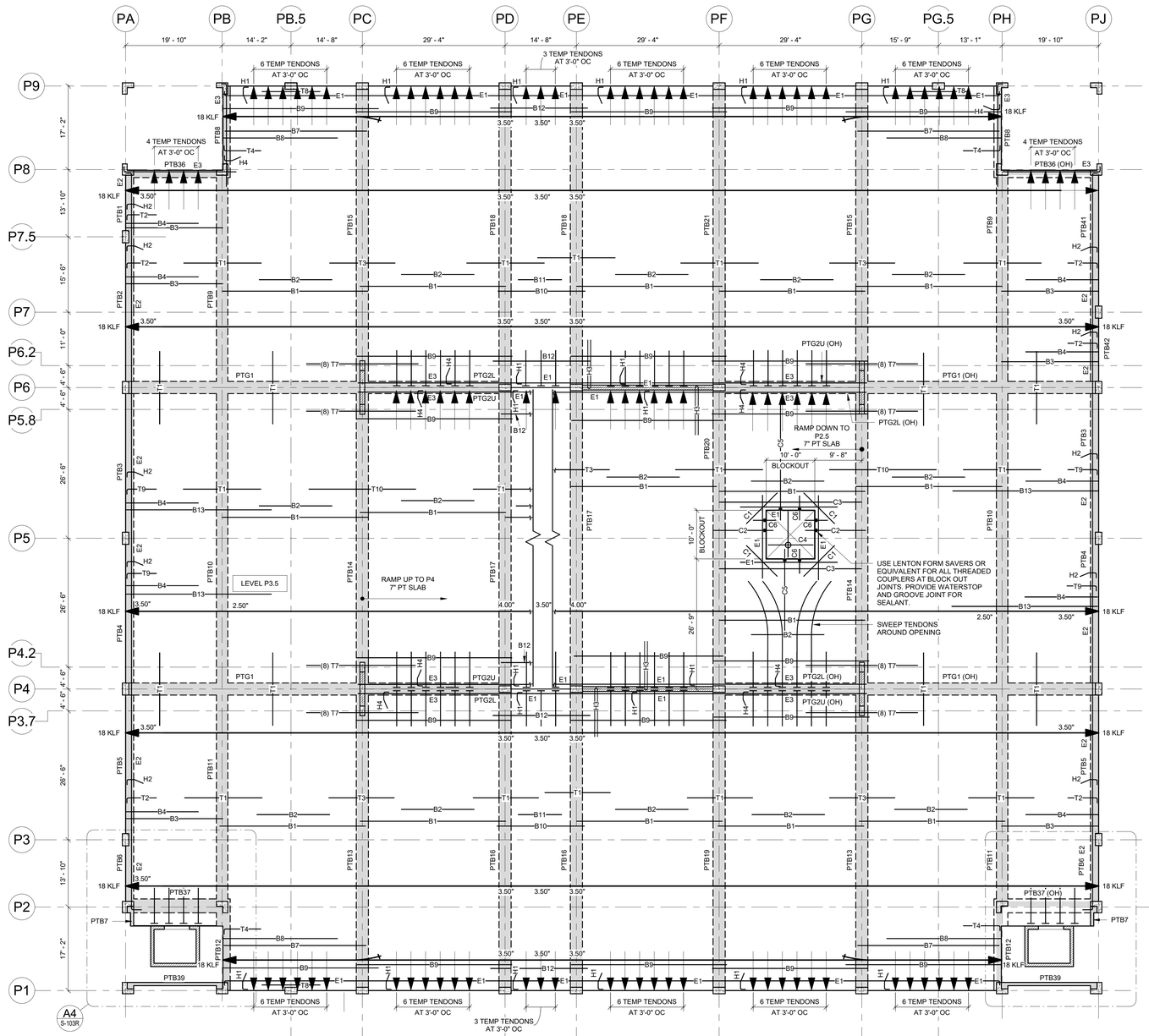
MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
B1	#4 x 30'-0" @ 36" BOTTOM (LONG)
B2	2-#4 x 15'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B3	#4 x 20'-0" @ 36" BOTTOM (LONG)
B4	2-#4 x 15'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B5	#4 x CONT. @ 14" BOTTOM
B6	#7 x CONT. @ 14" BOTTOM (EACH WAY WITHIN 23" THICK SLAB, HOOK AT SHEARWALLS)
B7	#4 x 31'-0" @ 36" BOTTOM (LONG)
B8	2-#4 x 23'-6" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B9	10-#4 x 32'-0" @ 12" BOTTOM (PLACE ALONG RAMP EDGES IN LIEU OF 'B1' & 'B2')
B10	#4 x 18'-0" @ 36" BOTTOM LONG
B11	2-#4 x 9'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 14")
B12	10-#4 x 20'-0" @ 12" BOTTOM (PLACE ALONG RAMP EDGES IN LIEU OF 'B10' & 'B11')
B13	#4 x 30'-0" @ 12" BOTTOM (LONG)

CRANE REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
C1	4-#4 x 8'-0" @ 7" MID-DEPTH (PLACE DIAGONALLY AT CORNERS SHOWN)
C2	#4 x 13'-0" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)
C3	5-#4 x 32'-0" @ 7" BOTTOM (PLACE ALONG OPENING EDGE IN LIEU OF 'B1' & 'B2')
C4	#4 x 6'-3" @ 7" BOTTOM (PLACE EACH WAY IN CRANE OPENING INFILL)
C5	#4 x 19'-3" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)
C6	#4 x 6'-3" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)

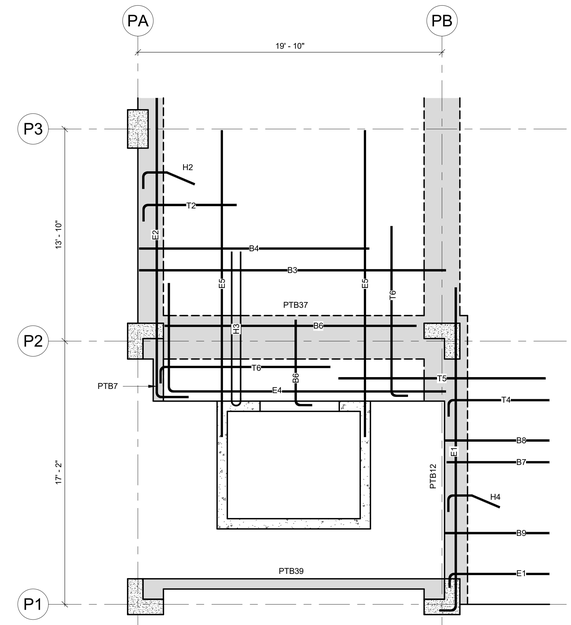
REINFORCING NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. TYPICAL SLAB SHALL BE 7" THICK POST-TENSIONED CONCRETE SLAB EXCEPT 23" THICK CORE AREAS. CONCRETE STRENGTH SHALL BE 3000 PSI AT TIME OF STRESSING AND 5000 PSI MINIMUM AT 28 DAYS. TYPICAL 3" CONCRETE WASH AT SLAB EDGES PER ARCHITECT ARE ADDED MONOLITHICALLY TO SLAB THICKNESSES GIVEN.
3. ELEVATED POST-TENSIONED SLABS SHALL BE AN ENCAPSULATED SYSTEM WITH POST-TENSIONED TENDONS THAT CONSIST OF 1/2" DIA. 270 KSI UNBONDED WIRE TENDONS. SLAB POST-TENSIONING FORCES GIVEN ARE EFFECTIVE PRESTRESS FORCES AFTER ALL SHORT AND LONG-TERM LOSSES AS SUBSTANTIATED BY CALCULATIONS PROVIDED BY THE PT SUPPLIER'S ENGINEER. IN NO CASE SHALL GREATER THAN 27 KIPS PER TENDON BE USED TO DETERMINE TENDON QUANTITIES PROVIDED. ADDED TENDONS EXTEND 6'-0" PAST SUPPORT CENTERLINES. ALL PT TENDONS MUST BE STRESSED WITHIN 96 HOURS OF CASTING CONCRETE.
4. UNLESS NOTED OTHERWISE ON PLAN, FOR UNIFORM SLAB TENDONS, PROVIDE SIMPLE-PARABOLIC TENDON PROFILES WITH HIGHPOINTS OF 5.75" AT EACH FACE OF SUPPORTING BEAM OR GIRDER, LOWPOINTS OF 1.00" AT MIDSPAN, AND ENDPOINTS OF 3.50" AT E.O.S. AND FACE OF EXTERIOR BEAM (IF ANY). FOR TEMPERATURE SLAB TENDONS, PROVIDE A STRAIGHT PROFILE WITHIN THE MIDDLE-THIRD OF THE SLAB DEPTH, AND ENDPOINTS OF 3.50" AT E.O.S. AND FACE OF EXTERIOR BEAMS (IF ANY).
5. REBAR SHALL BE GRADE 60 KSI WITH STANDARD CLASS B LAPS AND STANDARD HOOKS PLACED 2" CLEAR OF EDGE OF SLAB U.N.O. USE STIRRUP BENDS FOR BEAM STIRRUPS AND SMALL SECTION BENDS.
6. FOR INTERIOR SPANS, CENTER SLAB TOP BARS ON BEAM CENTER AND CENTER SLAB BOTTOM BARS ON MIDSPAN UNLESS SHOWN OTHERWISE. FOR EXTERIOR SPANS, START SLAB TOP REBAR HOOKS AND SLAB BOTTOM BARS AT 2" OFF E.O.S.
7. ALL #4 SLAB TOP BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS AT MIDSPAN. ALL #4 SLAB BOTTOM BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS AT BEAM CENTERS. ALL #4 TEMPERATURE BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS.
8. ALL SLAB REBAR COVER SHALL BE 1" CLEAR TOP & 3/4" CLEAR BOTTOM.
9. SEE BEAM & GIRDER REBAR SCHEDULE SHEETS FOR ALL BEAM & GIRDER DETAILS, SCHEDULE AND NOTES THAT CORRESPOND TO BEAM & GIRDER MARKS THIS SHEET.



3/32" = 1'-0"

A1 Level P3 & P3.5 Reinforcing Plan



1/4" = 1'-0"

A4 P3 Level Reinforcing Plan Enlarged

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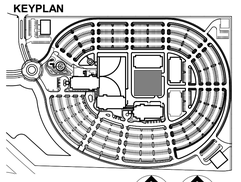
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



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Level P3 & P3.5 Reinforcing Plan

S-103R

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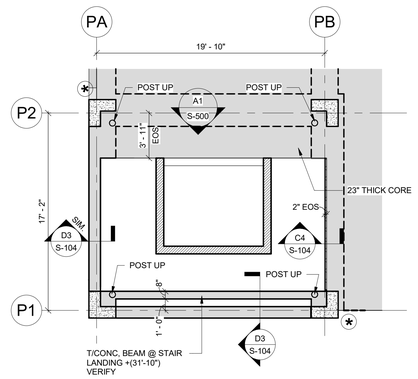
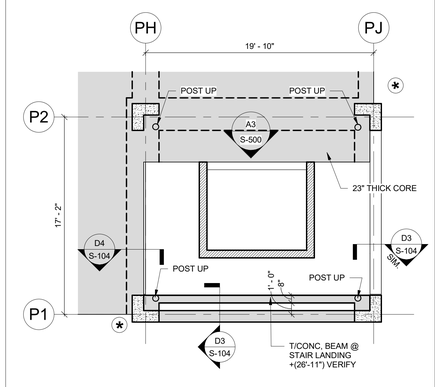


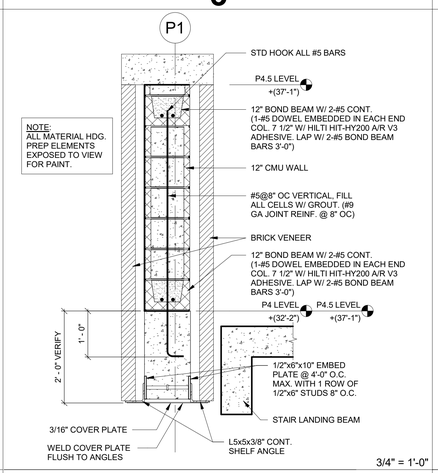
3/16" = 1'-0"

D1 Level P4.5 Framing Plan



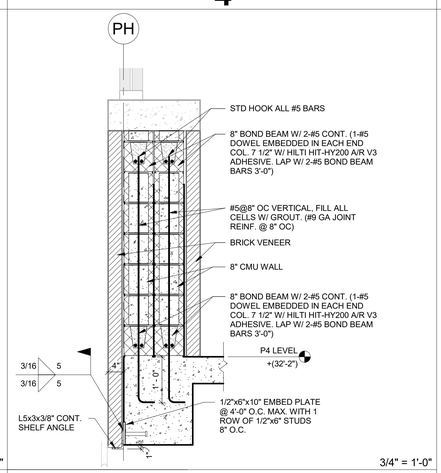
3/16" = 1'-0"

D2 Level P4 Framing Plan



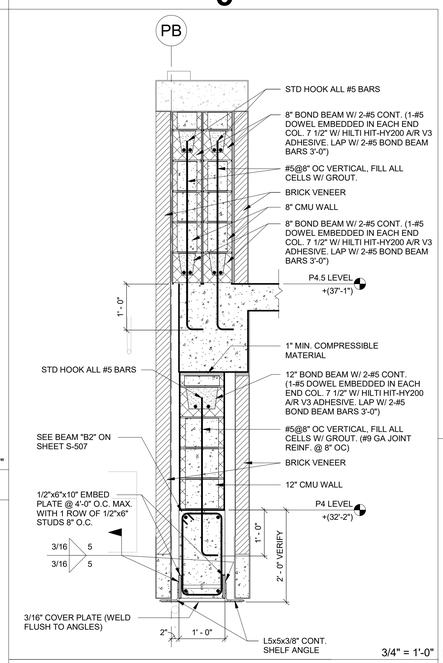
3/4" = 1'-0"

D3 Tower Section



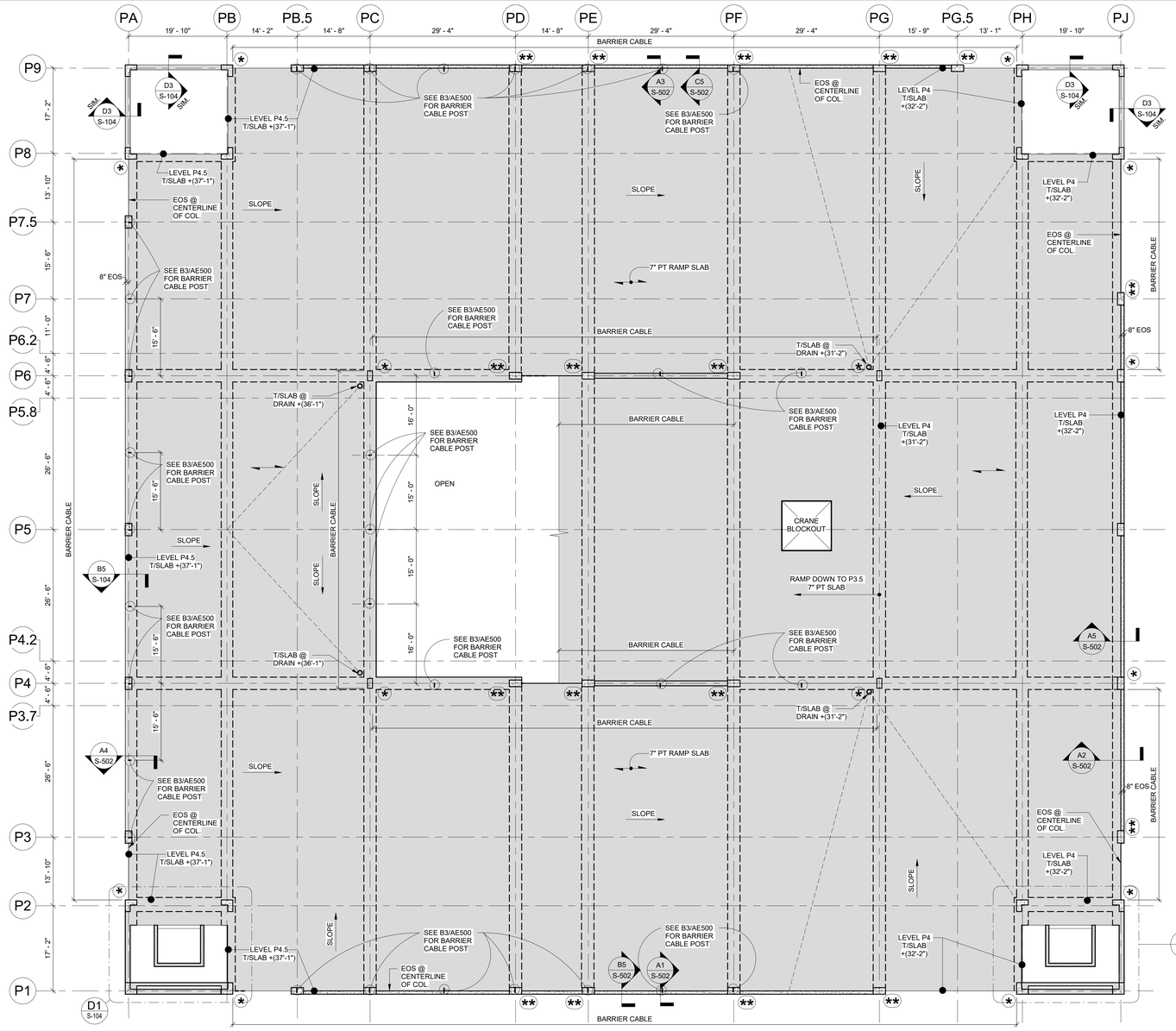
3/4" = 1'-0"

D4 Tower Section



3/4" = 1'-0"

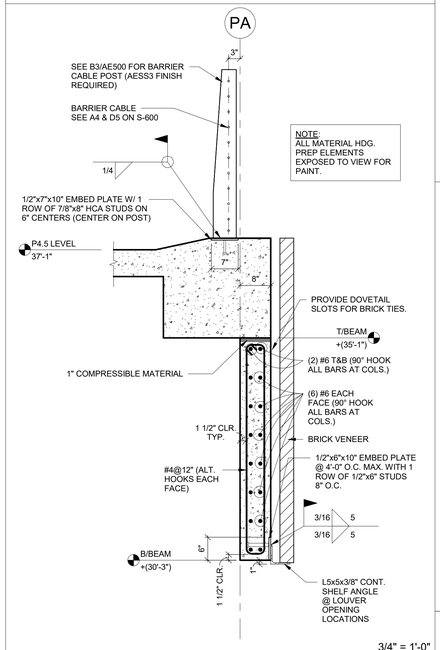
C4 Tower Section



3/32" = 1'-0"

A1 Level P4 & P4.5 Framing Plan

NOTE: PROVIDE FLAT SURFACE AT DRAIN LOCATIONS TO ALLOW LEVEL INSTALLATION OF DECK DRAINS.



3/4" = 1'-0"

B5 Section At Brick support Beam (West)

LEVEL P4 & P4.5 FRAMING PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. DENOTES ONE-WAY POST-TENSIONED BEAM-SUPPORTED SLAB CONCRETE SYSTEM. SEE REINFORCING PLAN ON DRAWING S-104R.
3. DENOTES SPAN DIRECTION.
4. COORDINATE ALL DRAINS, SLOPES, DEPRESSIONS, ETC. FOR SUSPENDED CONCRETE FRMG W/ ARCH AND MECH DWGS.
5. REFERENCE ELEVATION +(0'-0") @ P1 LEVEL HIGH POINT
6. REINF. ALL CONCRETE CURBING W/ WWF6x6-W2.9xW2.9 1" CLR. FROM T/CONC. USE LW CONCRETE AND PROVIDE BONDING AGENT. SEE ARCH. DWGS. FOR LOCATIONS.
7. PLEASE NOTE THAT DUE TO RESTRAINT BY THE SHEARWALLS AND OTHER WALLS, THE POST-TENSIONED SLABS ARE VERY LIKELY TO EXHIBIT SOME SHORTENING AND/OR RESTRAINT CRACKING WHICH SHOULD NOT IMPAIR THE SLAB'S STRUCTURAL INTEGRITY. RESTRAINT CRACKS ARE ALSO VERY LIKELY TO OCCUR IN SHEAR WALLS & OTHER WALLS.
8. BARRIER CABLE NOTES:
   - (*) DENOTES END COLUMN ANCHORAGE LOCATION.
   - (**) DENOTES COLUMN SLEEVED FOR CABLES - SEE (1) ON A4/S-600.
9. CONDUITS EMBEDED IN SUSPENDED SLABS OR WALLS SHALL SATISFY THE FOLLOWING:
   - A. CONDUIT OUTSIDE DIMENSION SHALL NOT BE LARGER THAN 1/3 THE OVERALL THICKNESS OF SLAB, WALL, OR BEAM IN WHICH THEY ARE EMBEDED.
   - B. CONDUIT SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER.
   - C. CONCRETE COVER SHALL BE 1 1/2".
   - D. INSTALLATION OF CONDUIT SHALL NOT REQUIRE REINFORCEMENT TO BE CUT, BENT OR DISPLACED FROM PROPER LOCATION.
10. TOWER CRANE BLOCKOUT SIZE TO BE PROVIDED BY CONTRACTOR. DESIGN IS BASED ON 10' SQUARE OPENINGS.

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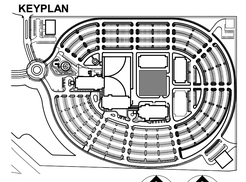
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



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Level P4 & P4.5 Framing Plan

S-104

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SLAB MISC REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
E1	ADD 4-#5 (2 TOP + 2 BOTTOM) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E2	ADD 2-#5 (2 TOP) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E3	ADD 3-#5 (3 TOP) PLACE AT ALL EDGES OF SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E4	ADD 6-#5 (3 TOP + 3 BOTTOM) PLACE AT ALL EDGES OF 23" SLAB WHERE SHOWN. LAP 24" MINIMUM. EXTEND 4'-0" PAST ALL INSIDE CORNERS. USE #5 CORNER BARS AT ALL OUTSIDE CORNERS. HOOK AT END OF SLAB.
E5	4-#4 BUNDLED BARS x 20'-0" LAP 2'-0" WITH HORIZONTAL SHEAR WALL REINF.
H1	#4 BENT-BAR @14" (3'-0" BENT LEG TOP, 1'-6" LEG BOTTOM, 8" LEG VERTICAL)
H2	#4 BENT-BAR @14" (3'-0" BENT LEG TOP, 8" HOOK VERTICAL)
H3	#5 U-BAR @12" (10'-0" LEG TOP, 10'-0" LEG BOTTOM, 5" LEG VERTICAL) PLACE AROUND VERTICAL SHEAR WALL REINF.
H4	#4 BENT-BAR @14" (3'-6" BENT LEG TOP, 8" HOOK VERTICAL)

SLAB TOP REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
T1	#4 x 16'-0" @ 12" TOP
T2	#4 x 6'-0" @ 12" TOP (HOOK)
T3	#4 x 13'-6" @ 12" TOP
T4	#4 x 7'-6" @ 12" TOP (HOOK)
T5	#5 x 13'-6" @ 7" TOP
T6	#7 x 11'-0" @ 14" TOP (HOOK)
T7	#6 x 23'-0" @ 12" TOP (BEND AT RAMP START/END)
T8	(6) #4 x 12'-0" @ 6" O.C. TOP
T9	#4 x 6'-0" @ 6" TOP (HOOK)
T10	#4 x 22'-0" @ 6" TOP (BEND AT RAMP START/END)
T11	#4 x 13'-6" @ 8" TOP O.C.
T12	#4 x 11'-0" @ 6" O.C. TOP (HOOK)

SLAB BOTTOM REBAR SCHEDULE

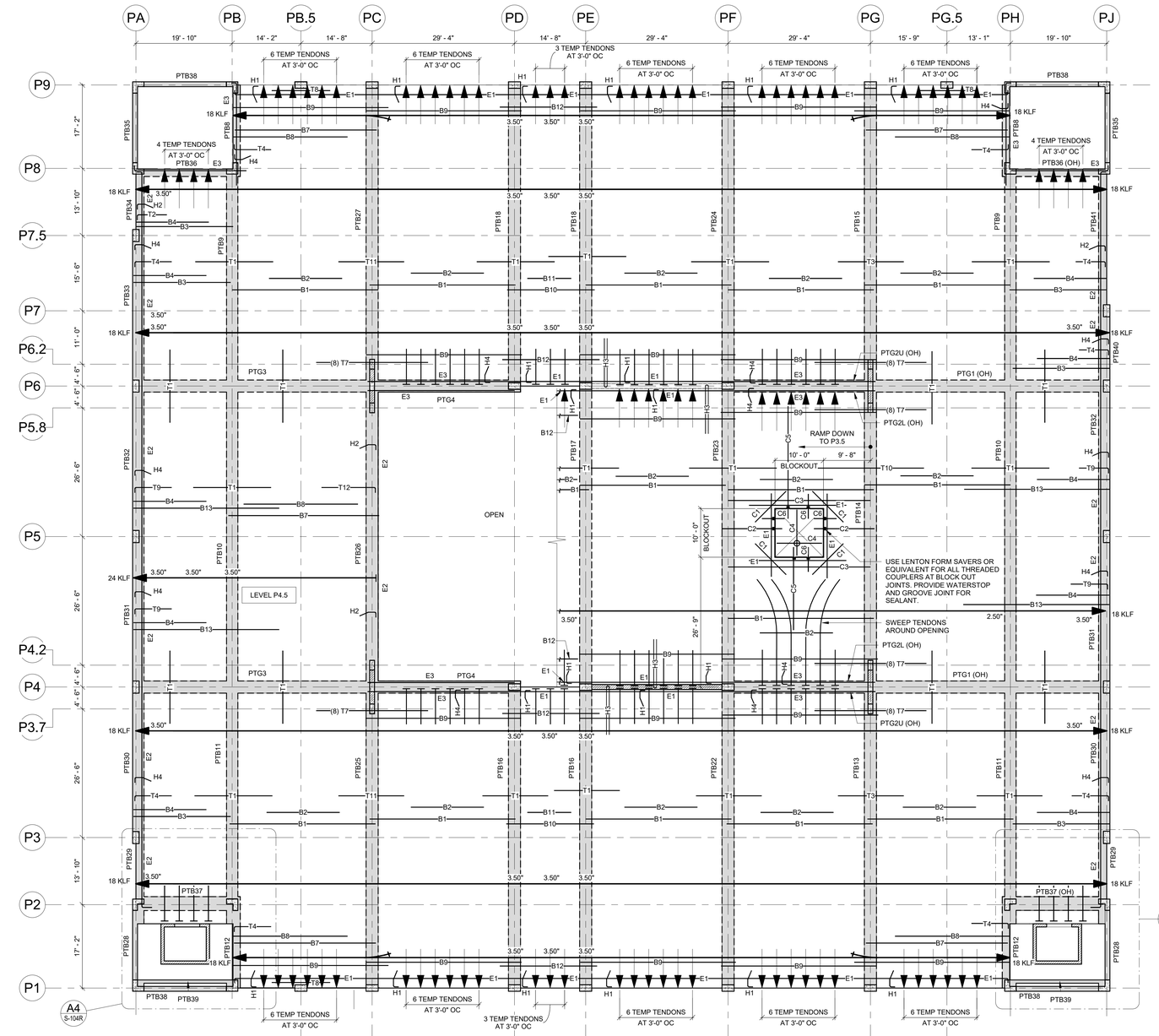
MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
B1	#4 x 30'-0" @ 36" BOTTOM (LONG)
B2	2-#4 x 15'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B3	#4 x 20'-0" @ 36" BOTTOM (LONG)
B4	2-#4 x 15'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B5	#4 x CONT. @ 14" BOTTOM
B6	#7 x CONT. @ 14" BOTTOM (EACH WAY WITHIN 23" THICK SLAB, HOOK AT SHEARWALLS)
B7	#4 x 31'-0" @ 36" BOTTOM (LONG)
B8	2-#4 x 23'-6" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 12")
B9	10-#4 x 32'-0" @ 12" BOTTOM (PLACE ALONG RAMP EDGES IN LIEU OF 'B1' & 'B2')
B10	#4 x 18'-0" @ 36" BOTTOM LONG
B11	2-#4 x 9'-0" @ 36" BOTTOM (SHORT: SPACE WITH LONG @ 14")
B12	10-#4 x 20'-0" @ 12" BOTTOM (PLACE ALONG RAMP EDGES IN LIEU OF 'B10' & 'B11')
B13	#4 x 30'-0" @ 12" BOTTOM (LONG)

CRANE REBAR SCHEDULE

MARK	QUANTITY, SIZE, LENGTH, SPACING AND COMMENTS
C1	4-#4 x 8'-0" @ 7" MID-DEPTH (PLACE DIAGONALLY AT CORNERS SHOWN)
C2	#4 x 13'-0" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)
C3	5-#4 x 32'-0" @ 7" BOTTOM (PLACE ALONG OPENING EDGE IN LIEU OF 'B1' & 'B2')
C4	#4 x 6'-3" @ 7" BOTTOM (PLACE EACH WAY IN CRANE OPENING INFILL)
C5	#4 x 19'-3" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)
C6	#4 x 6'-3" @ 14" BOTTOM (USE THREADED COUPLERS AT 2'-6" FROM END)

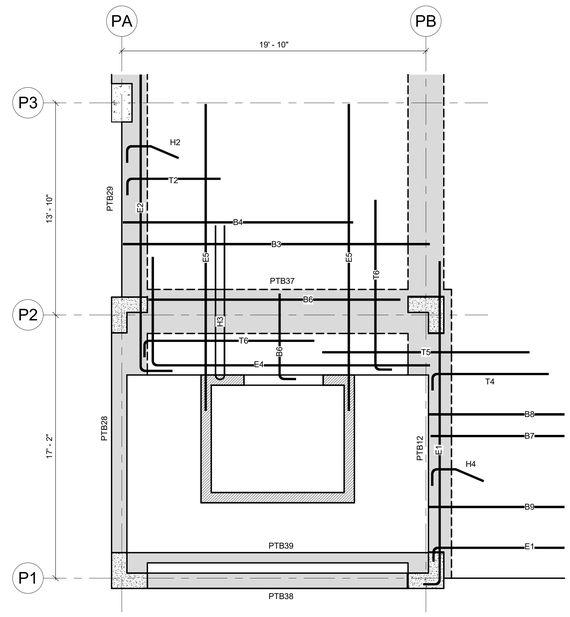
REINFORCING NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. TYPICAL SLAB SHALL BE 7" THICK POST-TENSIONED CONCRETE SLAB EXCEPT 23" THICK CORE AREAS. CONCRETE STRENGTH SHALL BE 3000 PSI AT TIME OF STRESSING AND 5000 PSI MINIMUM AT 28 DAYS. TYPICAL 3" CONCRETE WASH AT SLAB EDGES PER ARCHITECT ARE ADDED MONOLITHICALLY TO SLAB THICKNESSES GIVEN.
3. ELEVATED POST-TENSIONED SLABS SHALL BE AN ENCAPSULATED SYSTEM WITH POST-TENSIONED TENDONS THAT CONSIST OF 1/2" DIA. 270 KSI UNBONDED WIRE TENDONS. SLAB POST-TENSIONING FORCES GIVEN ARE EFFECTIVE PRESTRESS FORCES AFTER ALL SHORT AND LONG-TERM LOSSES AS SUBSTANTIATED BY CALCULATIONS PROVIDED BY THE PT SUPPLIER'S ENGINEER. IN NO CASE SHALL GREATER THAN 27 KIPS PER TENDON BE USED TO DETERMINE TENDON QUANTITIES PROVIDED. ADDED TENDONS EXTEND 6'-0" PAST SUPPORT CENTERLINES. ALL PT TENDONS MUST BE STRESSED WITHIN 96 HOURS OF CASTING CONCRETE.
4. UNLESS NOTED OTHERWISE ON PLAN, FOR UNIFORM SLAB TENDONS, PROVIDE SIMPLE-PARABOLIC TENDON PROFILES WITH HIGHPOINTS OF 5.75" AT EACH FACE OF SUPPORTING BEAM OR GIRDER, LOWPOINTS OF 1.00" AT MIDSPAN, AND ENDPOINTS OF 3.50" AT E.O.S. AND FACE OF EXTERIOR BEAM (IF ANY). FOR TEMPERATURE SLAB TENDONS, PROVIDE A STRAIGHT PROFILE WITHIN THE MIDDLE-THIRD OF THE SLAB DEPTH, AND ENDPOINTS OF 3.50" AT E.O.S. AND FACE OF EXTERIOR BEAMS (IF ANY).
5. REBAR SHALL BE GRADE 60 KSI WITH STANDARD CLASS B LAPS AND STANDARD HOOKS PLACED 2" CLEAR OF EDGE OF SLAB U.N.O. USE STIRRUP BENDS FOR BEAM STIRRUPS AND SMALL SECTION BENDS.
6. FOR INTERIOR SPANS, CENTER SLAB TOP BARS ON BEAM CENTER AND CENTER SLAB BOTTOM BARS ON MIDSPAN UNLESS SHOWN OTHERWISE. FOR EXTERIOR SPANS, START SLAB TOP REBAR HOOKS AND SLAB BOTTOM BARS AT 2" OFF E.O.S.
7. ALL #4 SLAB TOP BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS AT MIDSPAN. ALL #4 SLAB BOTTOM BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS AT BEAM CENTERS. ALL #4 TEMPERATURE BARS NOTED AS CONTINUOUS SHALL USE 18" LAPS.
8. ALL SLAB REBAR COVER SHALL BE 1" CLEAR TOP & 3/4" CLEAR BOTTOM.
9. SEE BEAM & GIRDER REBAR SCHEDULE SHEETS FOR ALL BEAM & GIRDER DETAILS, SCHEDULE AND NOTES THAT CORRESPOND TO BEAM & GIRDER MARKS THIS SHEET.



3/32" = 1'-0"

A1 P4 & P4.5 Level Reinforcing Plan



1/4" = 1'-0"

A4 P4 Level Reinforcing Plan - Callout 1

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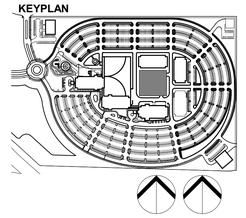
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



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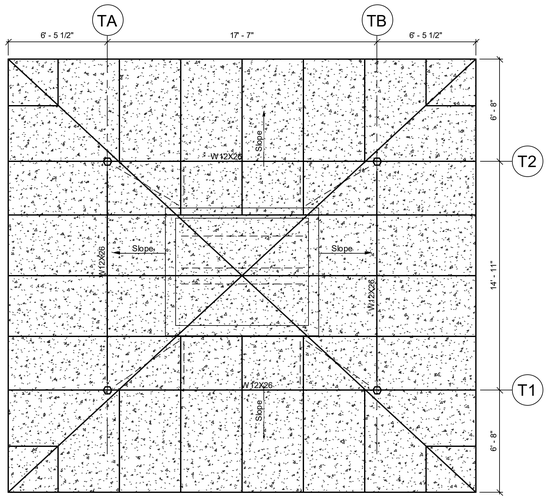
Level P4 & P4.5 Reinforcing Plan

S-104R

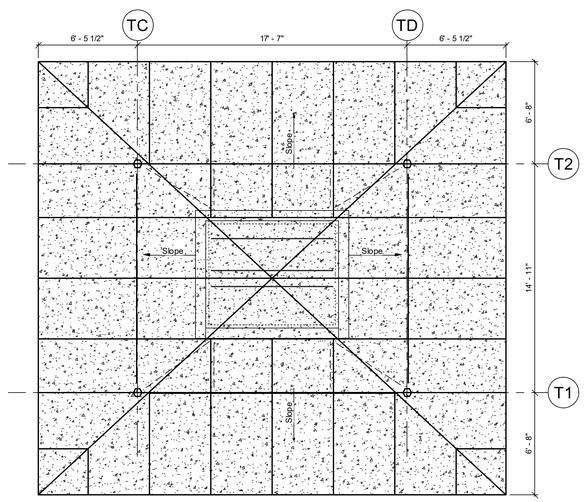


TOWER ROOF FRAMING PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. T/STL. BEAM EL. +(48'-0) UNLESS NOTED OTHERWISE.
3. INDICATES LG TRUSSES @ 4'-0" MAX W/ 1 1/2" TYPE B, 20 GA. STEEL DECK G60. SEE S-001 FOR TRUSS LOADING CRITERIA. SPAN DECK PERPENDICULAR TO TRUSSES W/ MAX SPAN OF 4'-0".
4. ATTACH ROOF DECKING (3 SPAN) TO TRUSSES W/ #14 TEK SCREWS IN 36/7 PATTERN AND BETWEEN TRUSSES AT SIDELAPS W/ #12 TEK SCREWS @ 12" OC MAX.
5. ATTACH TRUSSES TO W12 PERIMETER FRAMING AND 8" CONC. WALLS.

1/4" = 1'-0"

C2 Tower Roof Framing Plan

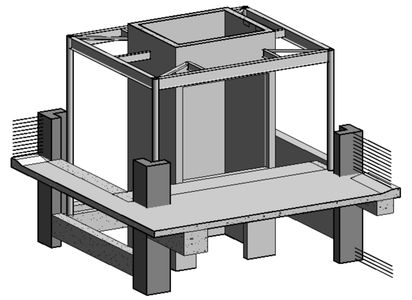


TOWER ROOF FRAMING PLAN NOTES:

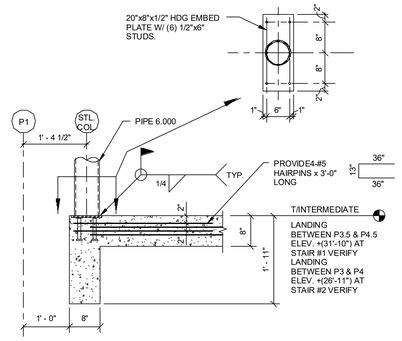
1. SEE S-001 FOR GENERAL NOTES.
2. T/STL. BEAM EL. +(48'-0) UNLESS NOTED OTHERWISE.
3. INDICATES LG TRUSSES @ 4'-0" MAX W/ 1 1/2" TYPE B, 20 GA. STEEL DECK G60. SEE S-001 FOR TRUSS LOADING CRITERIA. SPAN DECK PERPENDICULAR TO TRUSSES W/ MAX SPAN OF 4'-0".
4. ATTACH ROOF DECKING (3 SPAN) TO TRUSSES W/ #14 TEK SCREWS IN 36/7 PATTERN AND BETWEEN TRUSSES AT SIDELAPS W/ #12 TEK SCREWS @ 12" OC MAX.
5. ATTACH TRUSSES TO W12 PERIMETER FRAMING AND 8" CONC. WALLS.

1/4" = 1'-0"

C3 Tower Roof Framing Plan

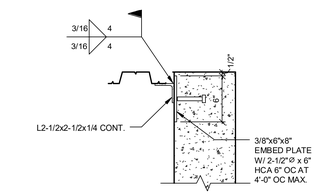


D5 Isometric @ Elevator



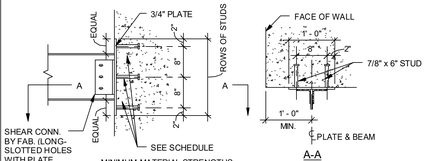
3/4" = 1'-0"

C5 Section At Pipe On Stair Landing



1 1/2" = 1'-0"

B5 Section At Cont. Deck Support



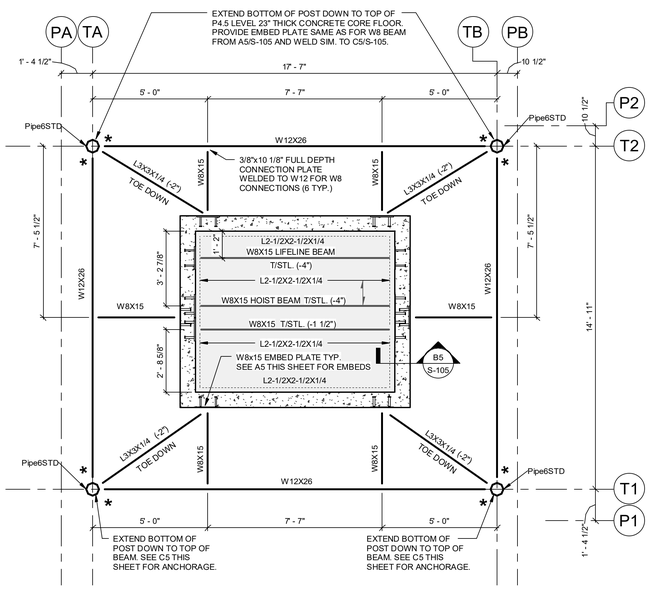
A-A

MINIMUM MATERIAL STRENGTHS:
STUDS (fut) = 65 ksi
CONCRETE (f'c) = 4 ksi

EMBED PLATE SCHEDULE			
BEAM	LENGTH	ROWS OF STUDS	Ø Vn KIPS
W8	12"	2	60
W10	20"	3	79
W12	20"	3	79
W14	20"	3	79
W16	28"	4	98
W18	28"	4	98
W21	36"	5	116
W24	36"	5	116
W27	44"	6	135

3/4" = 1'-0"

A5 Beam End Plate @ Concrete Wall

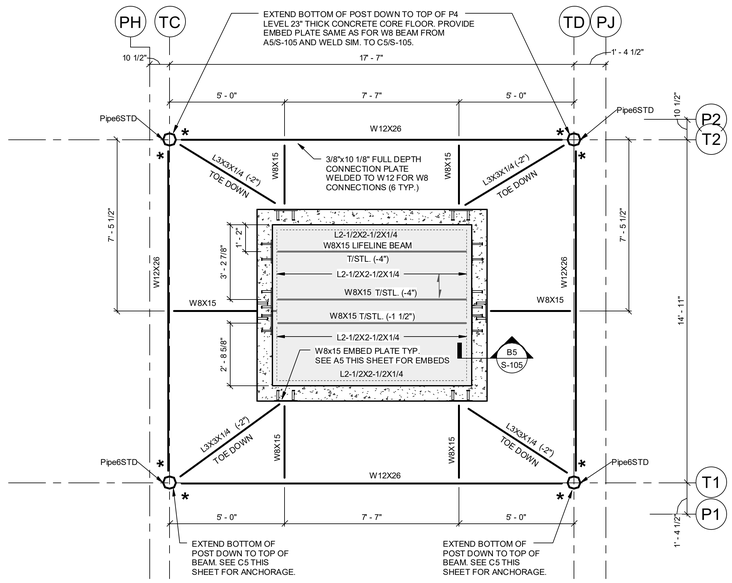


TOWER FRAMING PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. INDICATES STEEL ROOF DECK AND FRAMING. PROVIDE 1 1/2" TYPE "B" 20 GAGE STEEL ROOF DECK G60. T/STEEL BEAMS = +(51'-0") UNLESS NOTED OTHERWISE. ATTACH DECK TO W8 & L2 1/2x2 1/2 W/ #12 TEK SCREWS AT 12" OC MAX.
   INDICATES STEEL BEAMS = +(48'-0")
3. INDICATES SPAN DIRECTION OF DECK.
4. TYPICAL FINISH IS HDG & PREP FOR PAINT ON STEEL EXPOSED TO VIEW.
5. * INDICATES 3/8"x8 1/2" SINGLE PLATE CONNECTION W/ 3-3/4"Ø A325N BOLTS. RESTRAIN BOTTOM FLANGE W/ 1/4"x4" FLANGE PLATE

3/8" = 1'-0"

A1 Tower Framing Plan



TOWER FRAMING PLAN NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. INDICATES STEEL ROOF DECK AND FRAMING. PROVIDE 1 1/2" TYPE "B" 20 GAGE STEEL ROOF DECK G60. T/STEEL BEAMS = +(51'-0") UNLESS NOTED OTHERWISE. ATTACH DECK TO W8 & L2 1/2x2 1/2 W/ #12 TEK SCREWS AT 12" OC MAX.
   INDICATES STEEL BEAMS = +(48'-0")
3. INDICATES SPAN DIRECTION OF DECK.
4. TYPICAL FINISH IS HDG & PREP FOR PAINT ON STEEL EXPOSED TO VIEW.
5. * INDICATES 3/8"x8 1/2" SINGLE PLATE CONNECTION W/ 3-3/4"Ø A325N BOLTS. RESTRAIN BOTTOM FLANGE W/ 1/4"x4" FLANGE PLATE

3/8" = 1'-0"

A3 Tower Framing Plan

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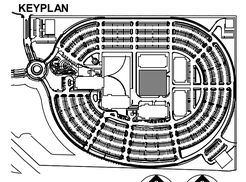
PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



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DATE: 00.00.2025

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Tower Framing Plans

S-105

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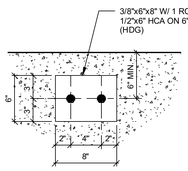
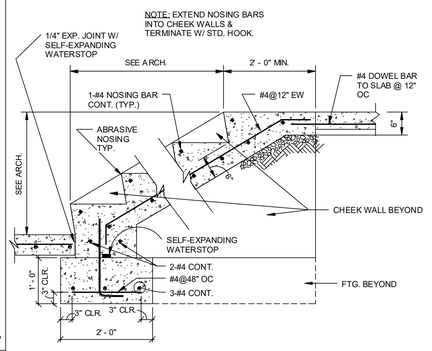


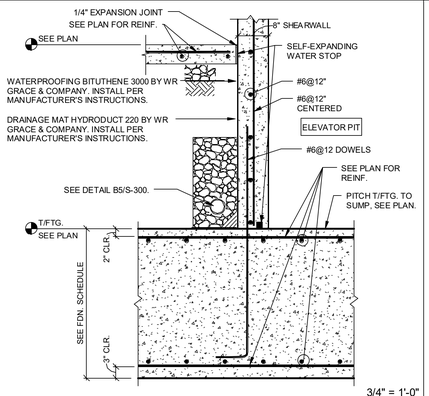
1 1/2" = 1'-0"

D1 Handrail/Guardrail Embed Plate Elevation



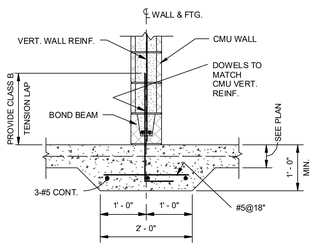
3/4" = 1'-0"

D2 Typical Stair-on-Grade Detail



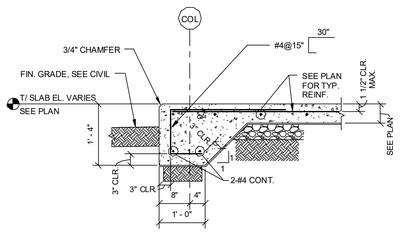
3/4" = 1'-0"

C1 Section At Elevator Pit



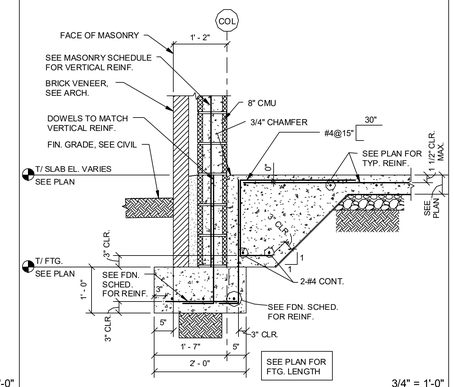
3/4" = 1'-0"

C2 Section At Interior CMU Wall Support



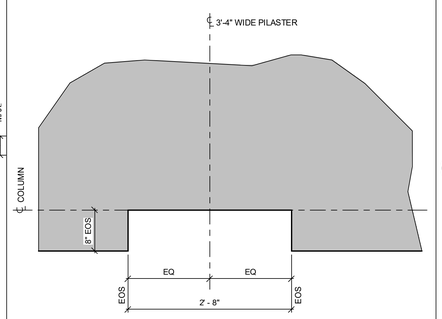
3/4" = 1'-0"

C3 Section @ Slab Turn Down



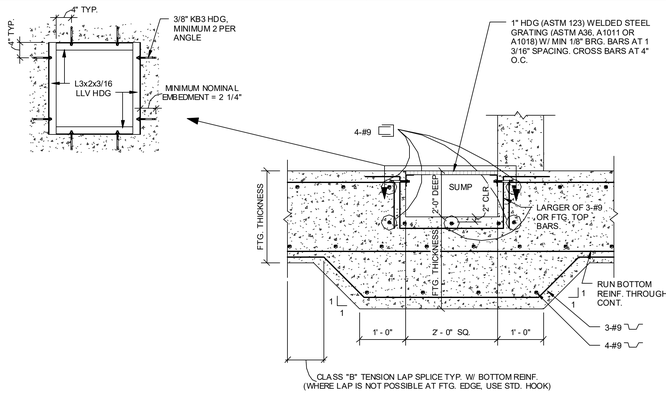
3/4" = 1'-0"

C4 Section @ Pilaster



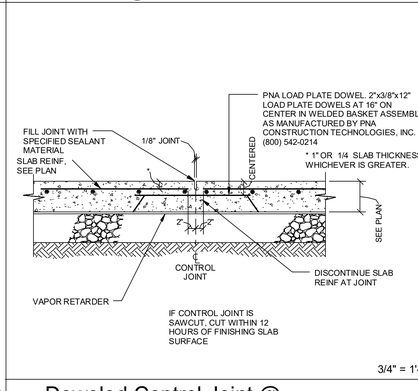
1" = 1'-0"

C5 EOS Plan Detail @ Pilaster



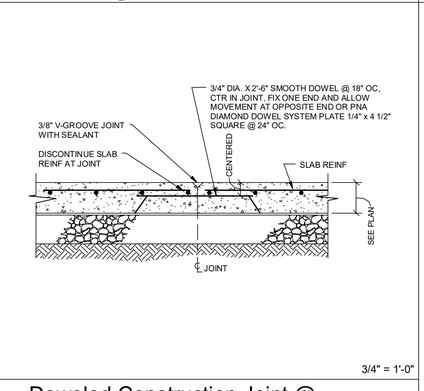
3/4" = 1'-0"

B1 Typical Sump Detail



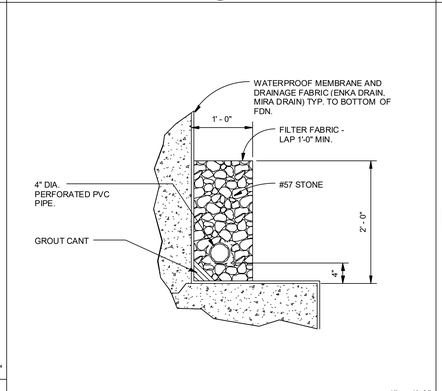
3/4" = 1'-0"

B3 Doweled Control Joint @ Slab-On-Grade



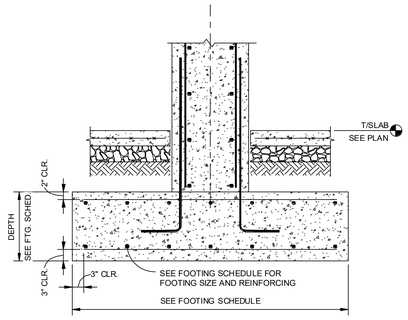
3/4" = 1'-0"

B4 Doweled Construction Joint @ Slab-On-Grade



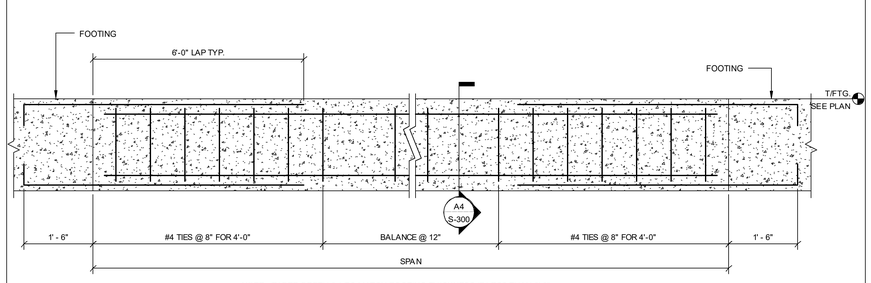
1" = 1'-0"

B5 Typical French Drain Detail



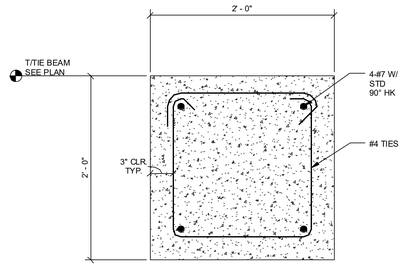
3/4" = 1'-0"

A1 Typical Footing Detail



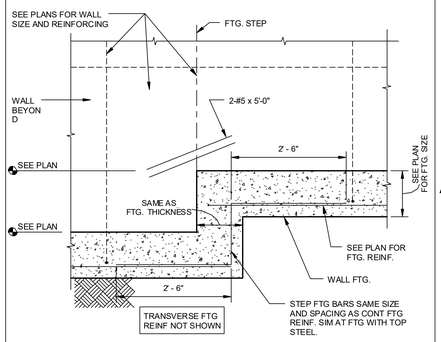
3/4" = 1'-0"

A2 Typical Tie Beam Diagram



1 1/2" = 1'-0"

A4 Tie Beam Section



3/4" = 1'-0"

A5 Typical Step Footing Detail

1 2 3 4 5

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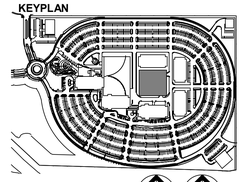
KEYPLAN



TRUE NORTH PLAN NORTH

0 8" 1'-4" 2'-8"
SCALE: 3/4" = 1'-0"
UNLESS NOTED OTHERWISE

PROJECT NO: 0470.62

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Foundation Sections & Details

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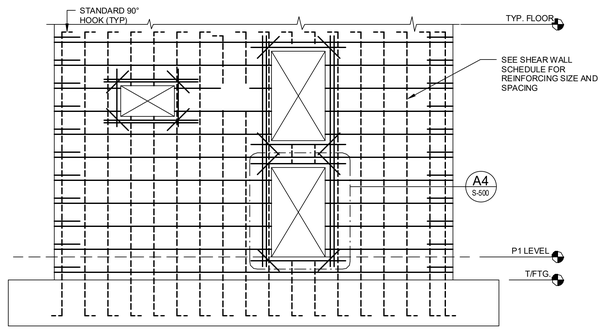


1/4" = 1'-0"

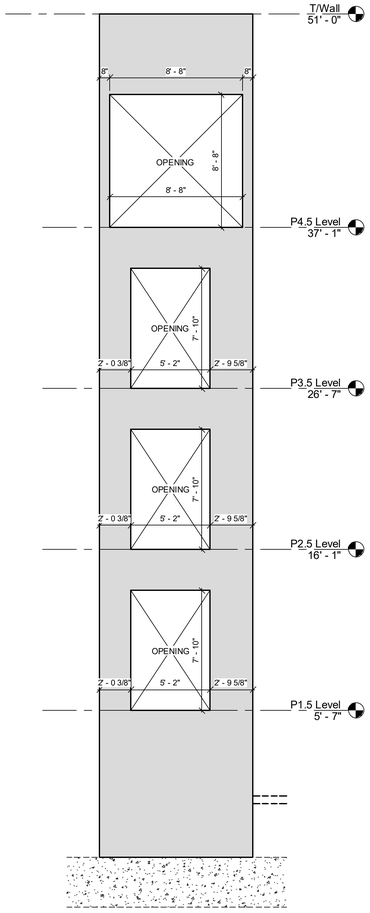
D1 Typical Shear Wall Reinforcing Detail

ORDINARY SHEAR WALL REINFORCING SCHEDULE

LEVEL	CONC. STRENGTH	VERT. REINF. 1	VERT. REINF. 2	HORIZ. REINF.	HORIZ. REINF.
WALL TYPE / THICKNESS	5 KSI UNO	12"	8"	1	2
ELEVATOR ROOF					
P4 & P4.5 LEVEL					
P3 & P3.5 LEVEL		#5@12" #5@12"	#6@12"	#5@12"	#6@12"
P2 & P2.5 LEVEL		#5@12" #5@12"	#6@12"	#5@12"	#6@12"
P1 & P1.5 LEVEL		#6@12" #6@12"	#6@12"	#6@12"	#6@12"
FOUNDATION		#6@12" #6@12"	#6@12"	#6@12"	#6@12"
REMARKS					

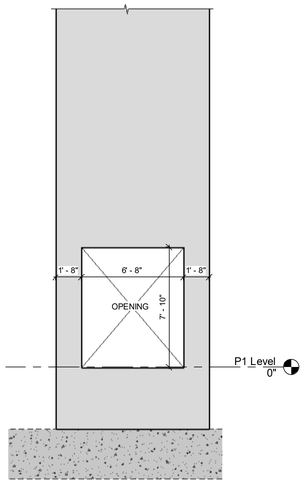
SHEAR WALL SCHEDULE NOTES:

1. SEE SCHEDULE FOR TYPICAL VERTICAL AND HORIZONTAL REINFORCING. PROVIDE ADDITIONAL REINFORCING AT OPENINGS, EDGES, AND CORNERS AS INDICATED IN A1 THIS DRAWING.
2. SHEAR WALL SPLICE DETAILS SHALL CONFORM TO DETAILS OF ACI 315 AND REQUIREMENTS OF ACI 318 UNLESS SPECIFICALLY NOTED OTHERWISE.
3. CONTRACTOR MAY SPLICE BARS AT EACH LEVEL OR AT ALTERNATE LEVELS. USE CLASS "B" TENSION LAP TYP.
4. HOOK ALL VERTICAL BARS INTO SLAB OR BEAM WHERE SHEAR WALL ENDS.
5. SEE TYPICAL WALL REINFORCING DETAILS/SECTIONS FOR ADDITIONAL INFORMATION.
6. SEE S-001 FOR GENERAL NOTES.
7. THE SIZE OF ALL DOORWAYS, PENETRATIONS AND MECH OPENINGS THROUGH SHEAR WALLS SHALL BE VERIFIED FROM THE MECH AND/OR ARCH PLANS.
8. ALL SPECIAL SHEAR WALL REINFORCING ASTM A615, GR. 60.



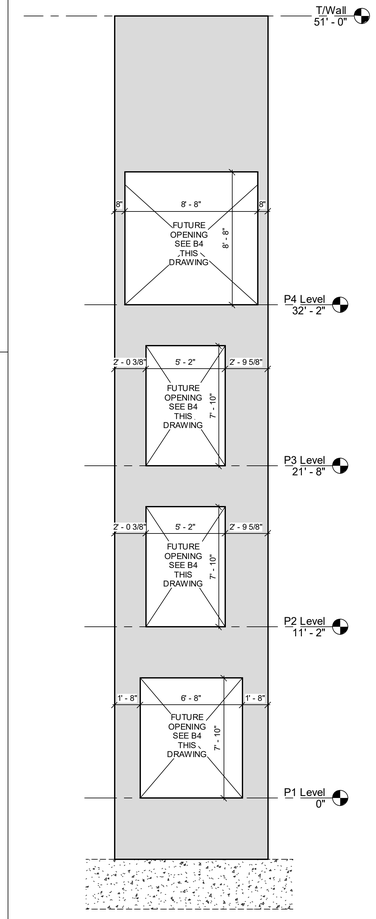
1/4" = 1'-0"

A1 8" Thick Conc. Shear Wall Openings @ Elevator #1 (North Elevation)



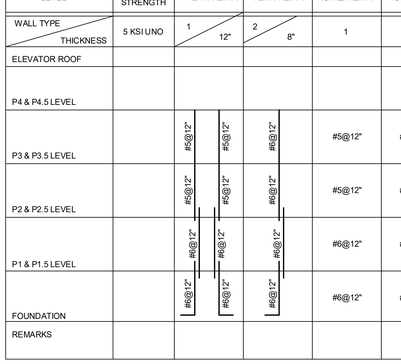
1/4" = 1'-0"

A2 8" Thick Conc. Shear Wall Openings @ Elevator #1 (South Elevation)



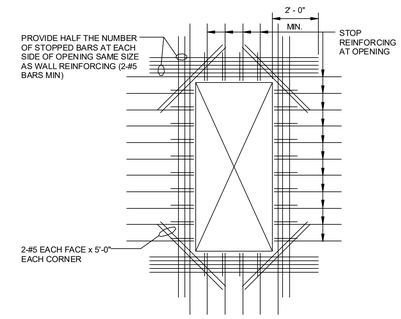
1/4" = 1'-0"

A3 8" Thick Conc. Shear Wall Openings @ Elevator #2 (North Elevation)



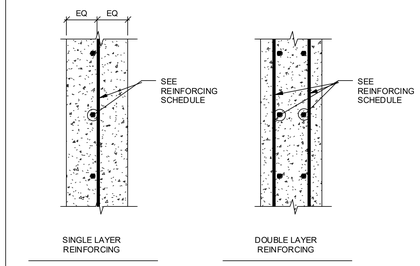
3/8" = 1'-0"

B4 Future Concrete Wall Opening Reinforcing Detail



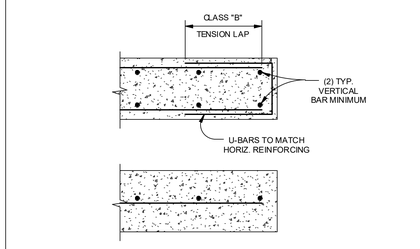
3/8" = 1'-0"

A4 Typical Concrete Wall Opening Detail



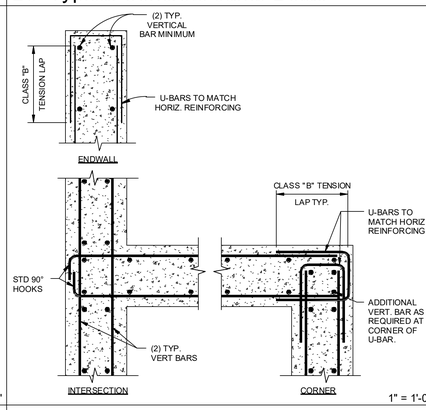
1" = 1'-0"

C5 Typical Shear Wall Plan Detail



1" = 1'-0"

B5 Typical Concrete Jamb Detail



1" = 1'-0"

A5 Reinforcing Plan @ Shear Wall Corner

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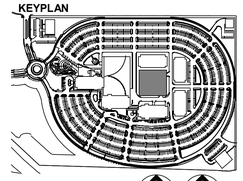

TRUE NORTH PLAN NORTH

0 1'-4" 2'-8" 5'-4"
SCALE: 3/8" = 1'-0"
UNLESS NOTED OTHERWISE

PROJECT NO: 0470.62

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Shear Wall Schedule & Details

S-500

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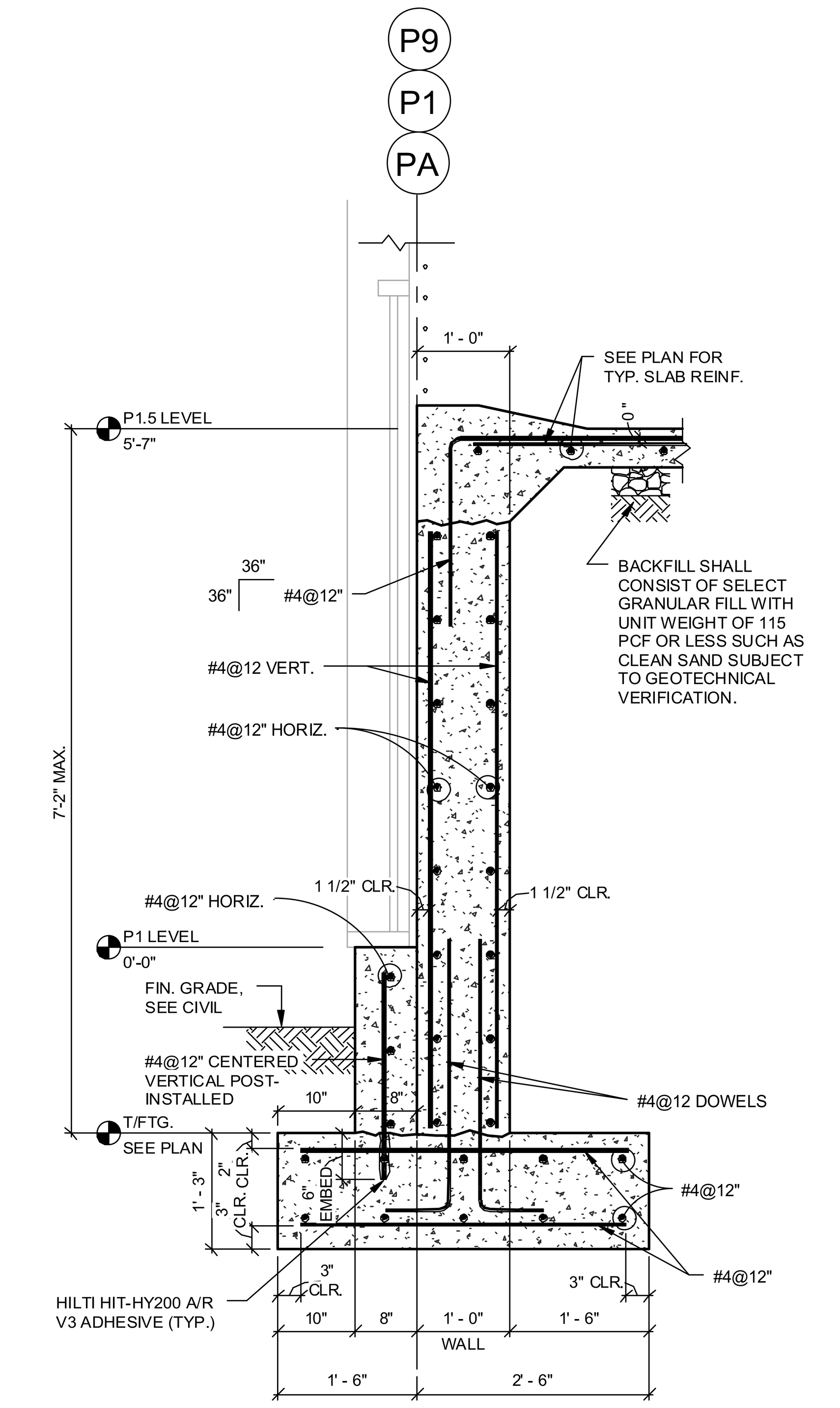
Model File: P:\Design\Strategies PROJECTS\0470.62 Carolina Forest Parking Deck\500 Drawings\Revit\Structural\047062_CF Parking Deck - Struct 2025.rvt

NOTES:
1. ALL CONCRETE COLD JOINTS TO BE CLEAR & FREE OF LAITANCE & ROUGHENED TO FULL AMPLITUDE OF 1/4".
2. PRIOR TO POURING SLAB ON GRADE DURING CONSTRUCTION, CONTRACTOR TO ENSURE NO WATER IS BEHIND WALLS. PERMANENT WALL DRAINAGE IS NOT REQUIRED EXCEPT WHERE T/FTG IS -(4'-0").

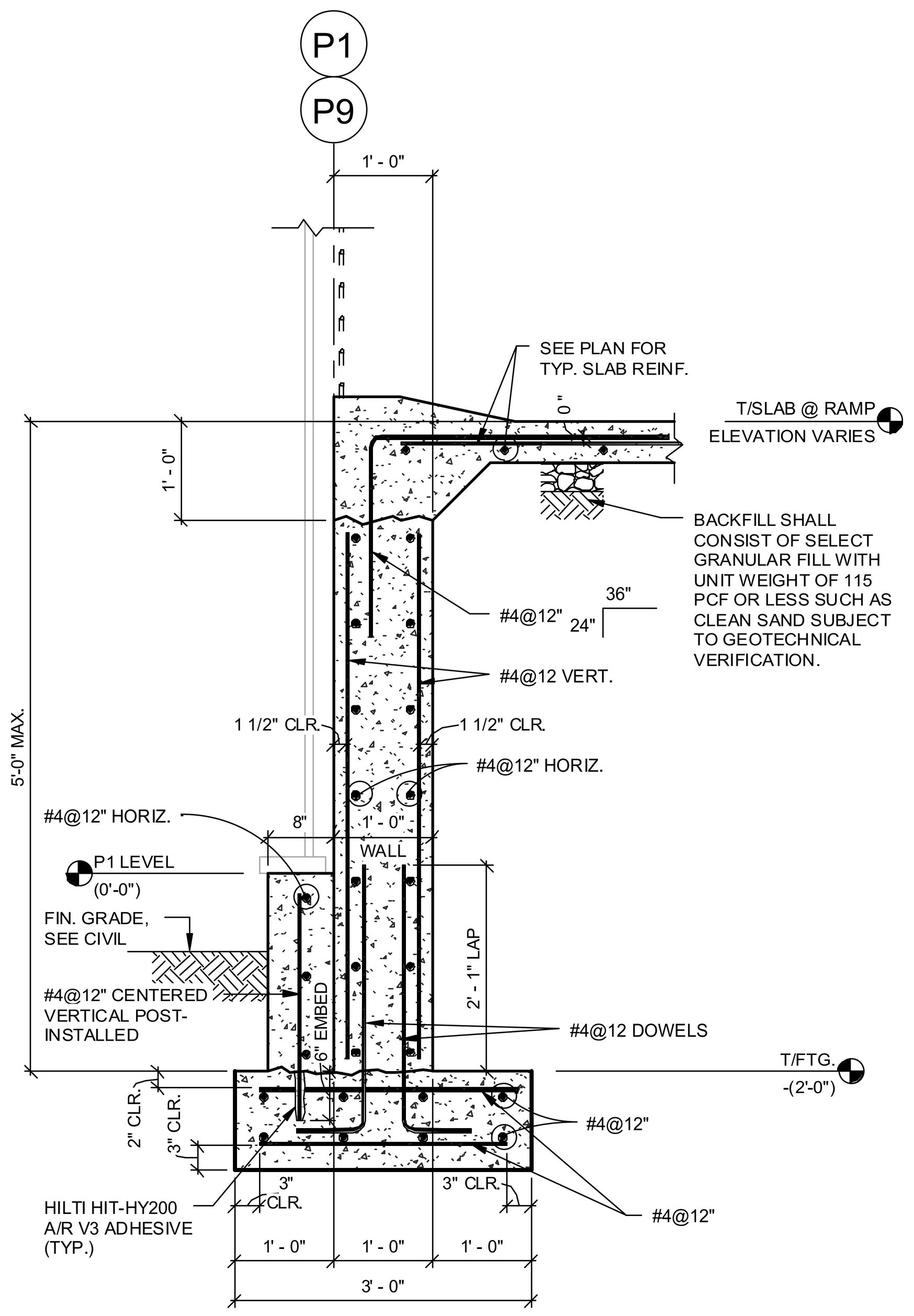


NOTES:
1. FOUNDATION DESIGN FOR NET ALLOWABLE BEARING PRESSURE OF 2500 PSF WITHOUT GROUND IMPROVEMENTS.
2. SEE PLAN FOR FOUNDATION BUMPOUT AT MASONRY PILASTERS.

3/4" = 1'-0"

C1 Section @ Retaining Wall "B"

NOTES:
1. ALL CONCRETE COLD JOINTS TO BE CLEAR & FREE OF LAITANCE & ROUGHENED TO FULL AMPLITUDE OF 1/4".
2. PRIOR TO POURING SLAB ON GRADE DURING CONSTRUCTION, CONTRACTOR TO ENSURE NO WATER IS BEHIND WALLS. PERMANENT WALL DRAINAGE IS NOT REQUIRED EXCEPT WHERE T/FTG IS -(4'-0").

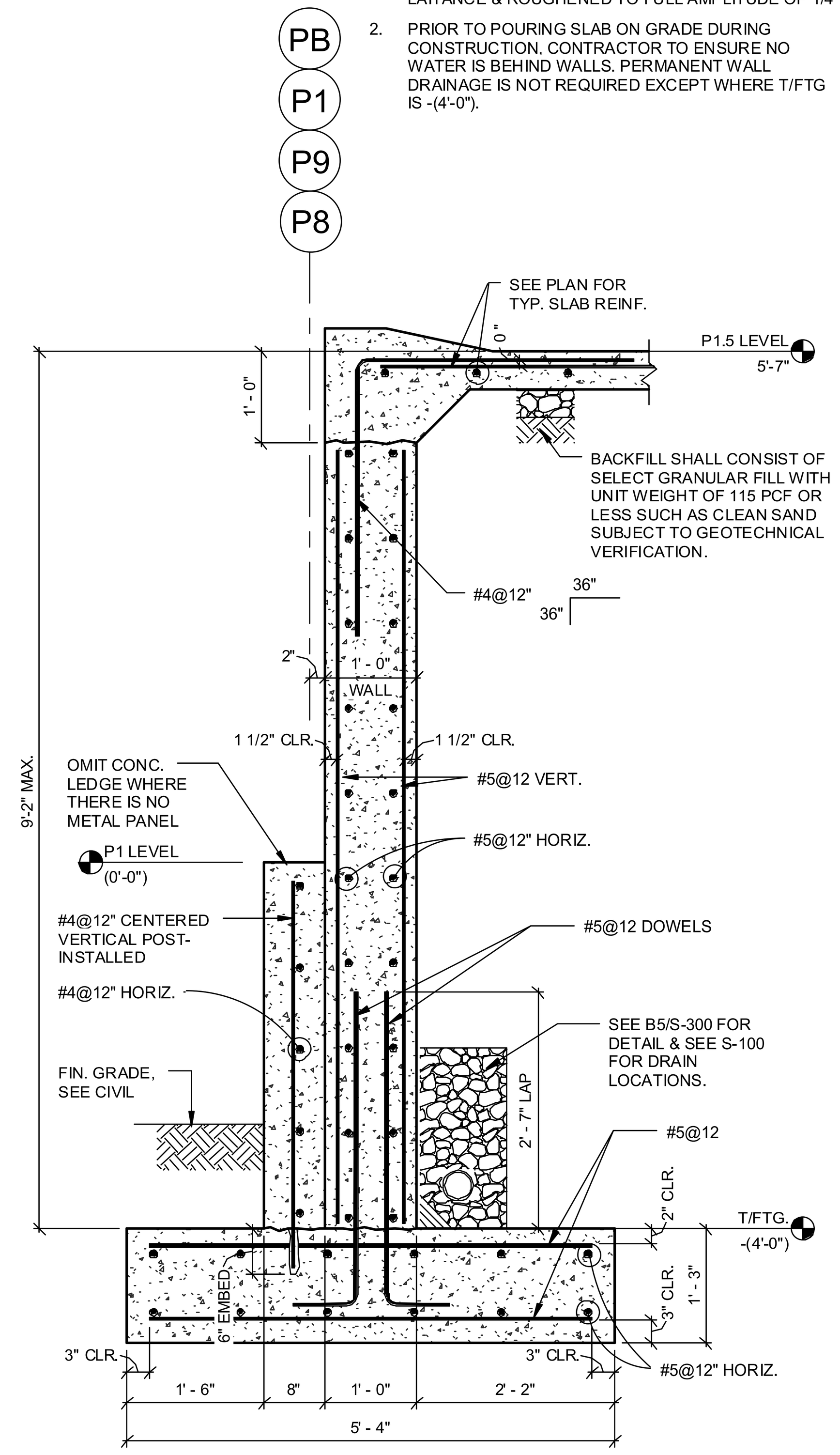


NOTES:
1. FOUNDATION DESIGN FOR NET ALLOWABLE BEARING PRESSURE OF 2500 PSF WITHOUT GROUND IMPROVEMENTS.
2. SEE PLAN FOR FOUNDATION BUMPOUT AT MASONRY PILASTERS.

3/4" = 1'-0"

C2 Section @ Retaining Wall "A"

NOTES:
1. ALL CONCRETE COLD JOINTS TO BE CLEAR & FREE OF LAITANCE & ROUGHENED TO FULL AMPLITUDE OF 1/4".
2. PRIOR TO POURING SLAB ON GRADE DURING CONSTRUCTION, CONTRACTOR TO ENSURE NO WATER IS BEHIND WALLS. PERMANENT WALL DRAINAGE IS NOT REQUIRED EXCEPT WHERE T/FTG IS -(4'-0").

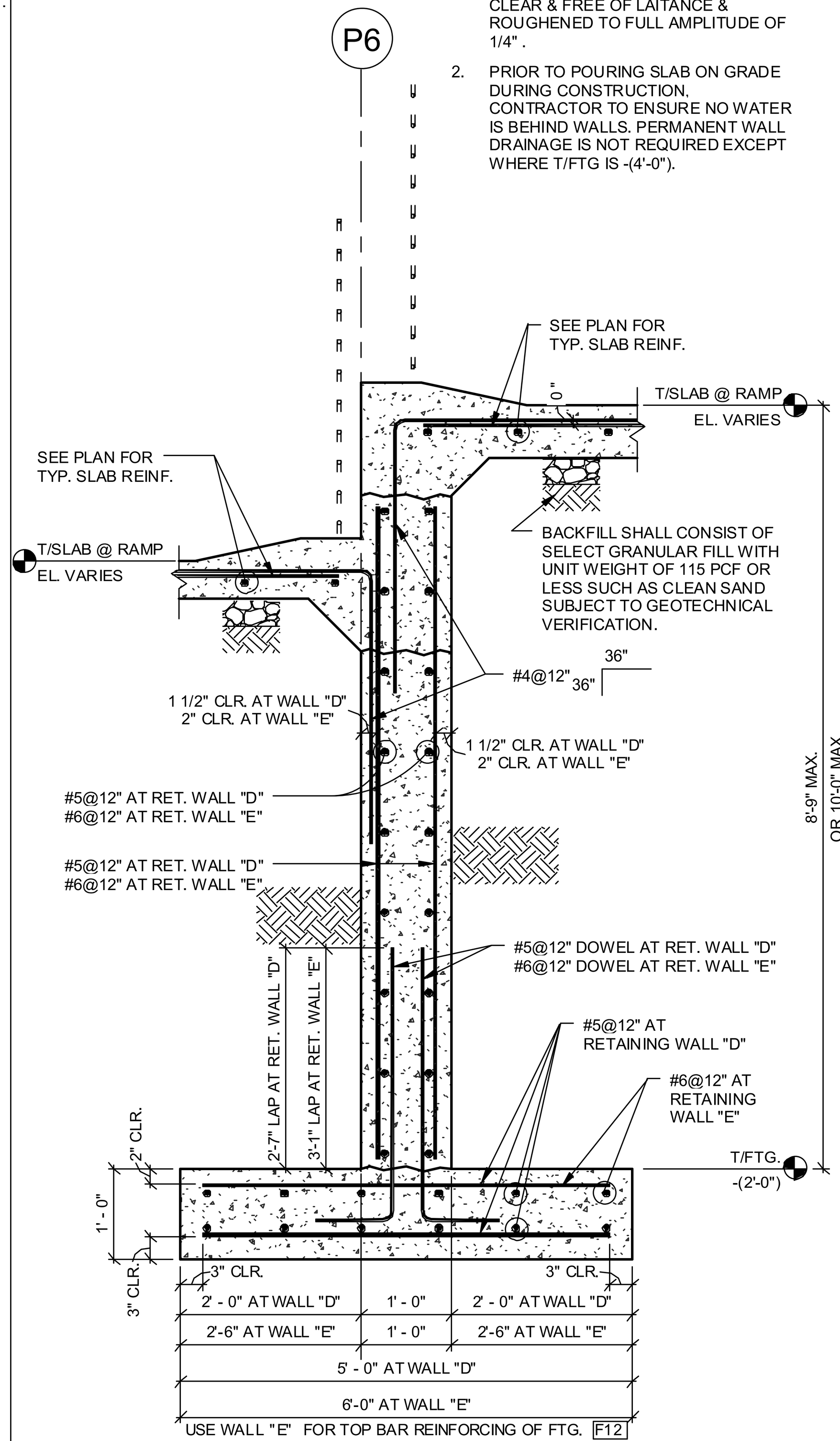


NOTES:
1. FOUNDATION DESIGN FOR NET ALLOWABLE BEARING PRESSURE OF 2500 PSF WITHOUT GROUND IMPROVEMENTS.

3/4" = 1'-0"

C3 Section @ Retaining Wall "C"

NOTES:
1. ALL CONCRETE COLD JOINTS TO BE CLEAR & FREE OF LAITANCE & ROUGHENED TO FULL AMPLITUDE OF 1/4".
2. PRIOR TO POURING SLAB ON GRADE DURING CONSTRUCTION. CONTRACTOR TO ENSURE NO WATER IS BEHIND WALLS. PERMANENT WALL DRAINAGE IS NOT REQUIRED EXCEPT WHERE T/FTG IS -(4'-0").

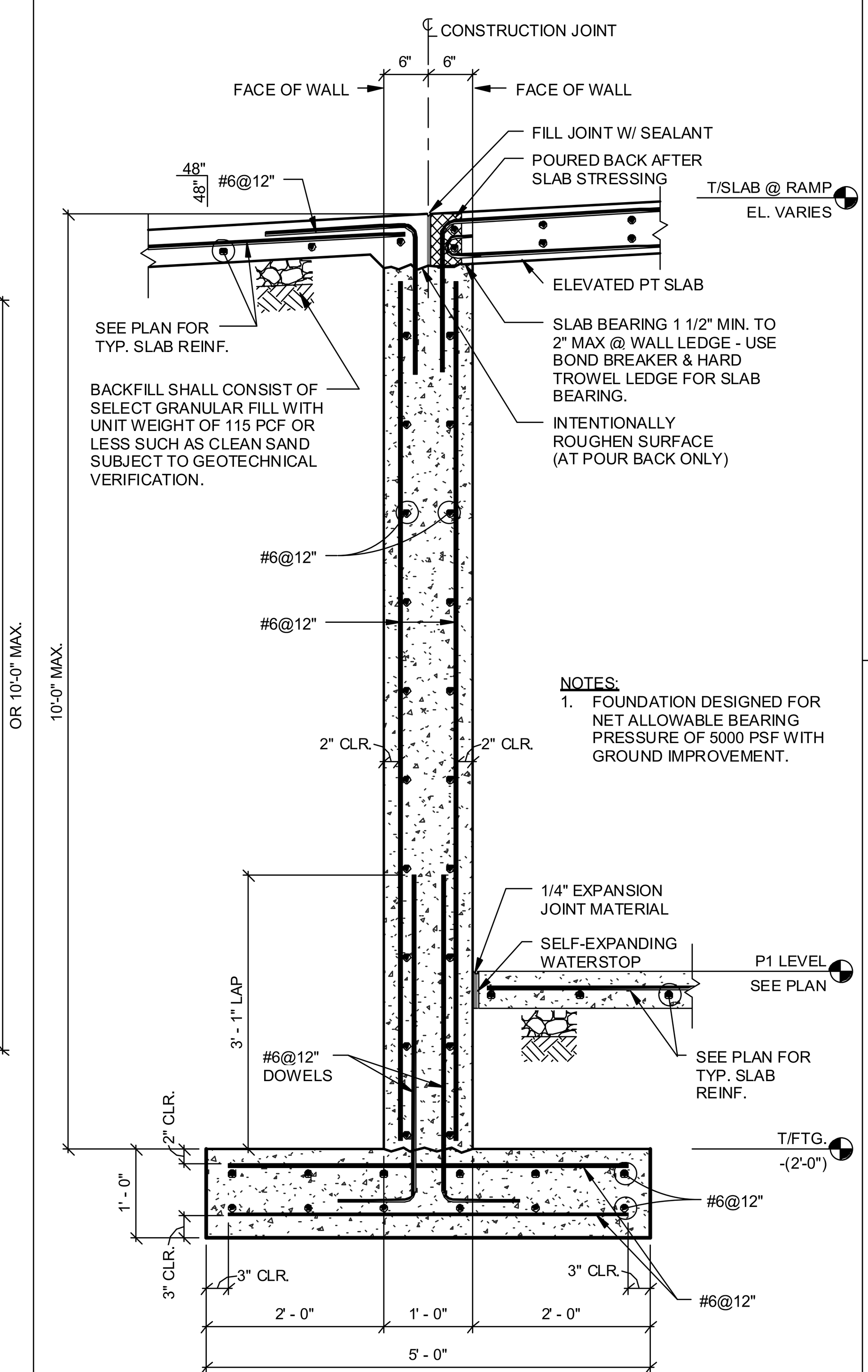


NOTES:
1. FOUNDATION DESIGN FOR NET ALLOWABLE BEARING PRESSURE OF 2500 PSF WITHOUT GROUND IMPROVEMENTS.

3/4" = 1'-0"

C4 Section @ Retaining Wall "D" & "E"

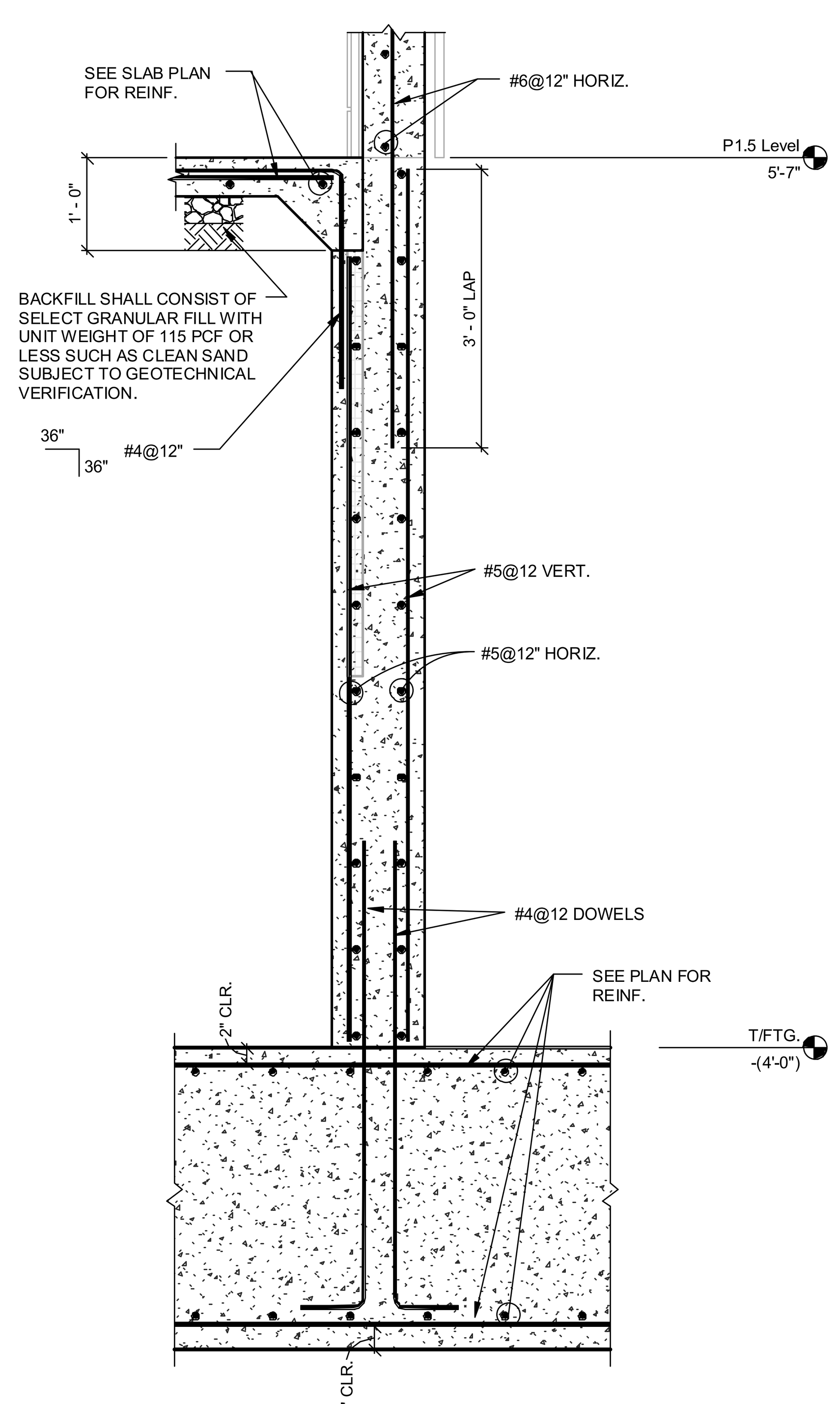
NOTES:
1. ALL CONCRETE COLD JOINTS TO BE CLEAR & FREE OF LAITANCE & ROUGHENED TO FULL AMPLITUDE OF 1/4".
2. PRIOR TO POURING SLAB ON GRADE DURING CONSTRUCTION, CONTRACTOR TO ENSURE NO WATER IS BEHIND WALLS. PERMANENT WALL DRAINAGE IS NOT REQUIRED EXCEPT WHERE T/FTG IS -(4'-0").



NOTES:
1. FOUNDATION DESIGNED FOR NET ALLOWABLE BEARING PRESSURE OF 5000 PSF WITH GROUND IMPROVEMENT.

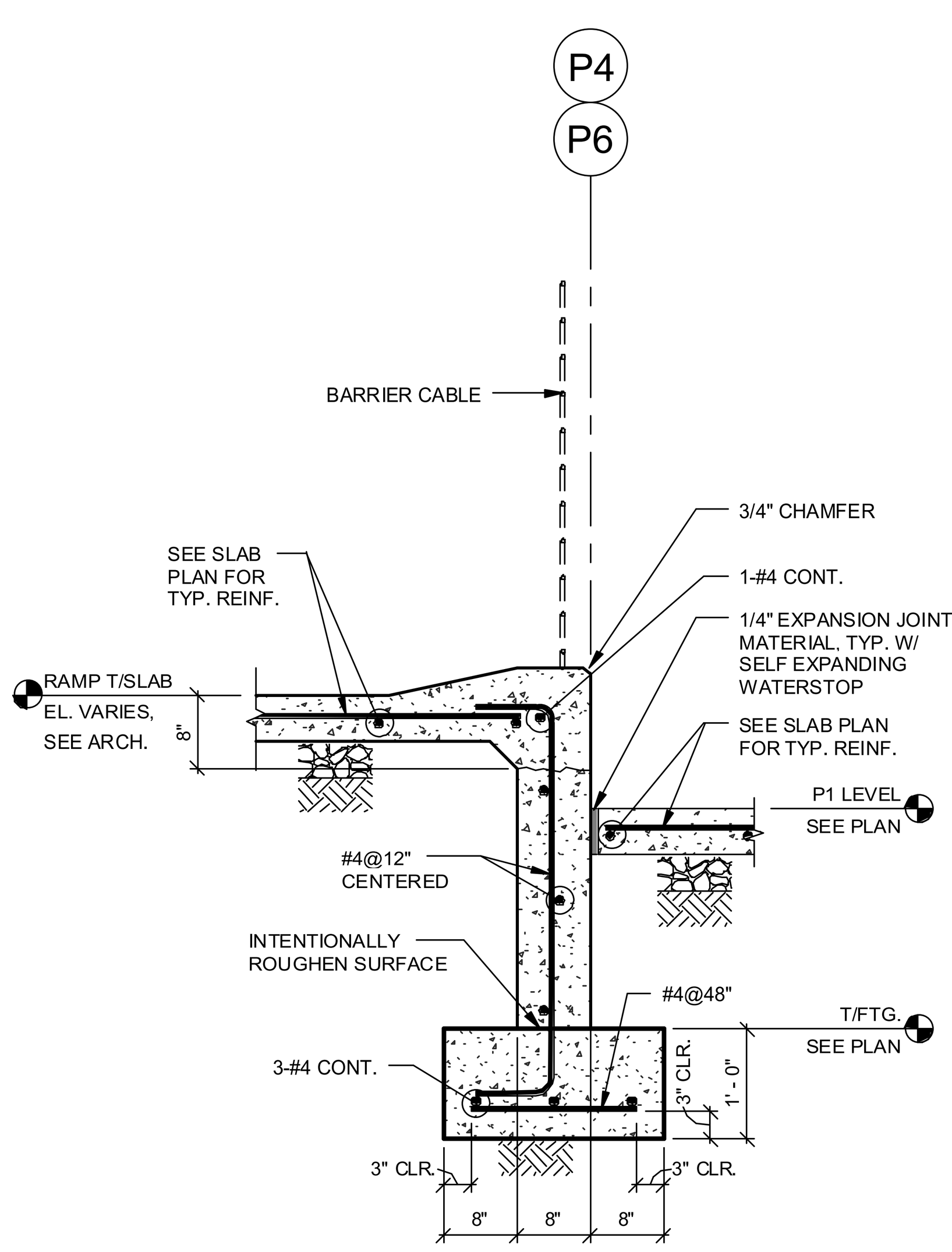
3/4" = 1'-0"

C5 Section @ Retaining Wall "F"



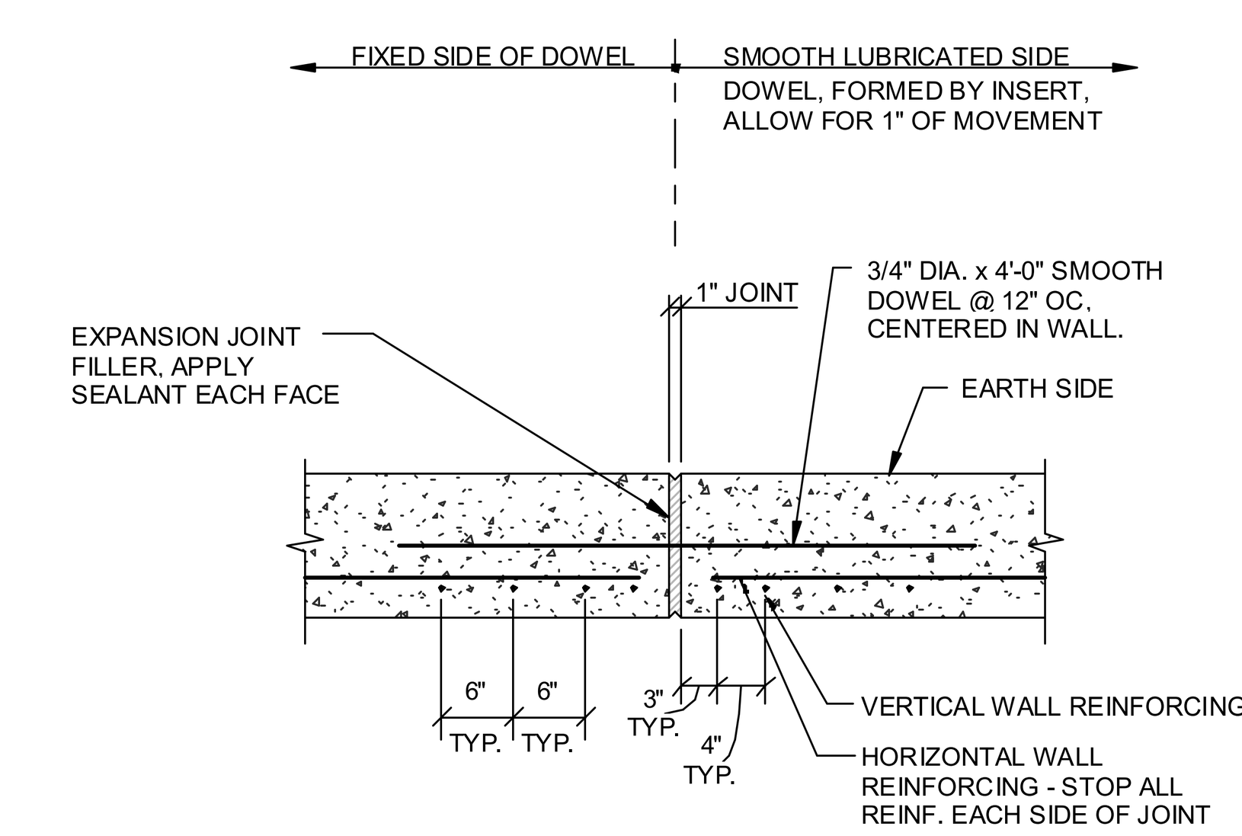
3/4" = 1'-0"

A1 Section



3/4" = 1'-0"

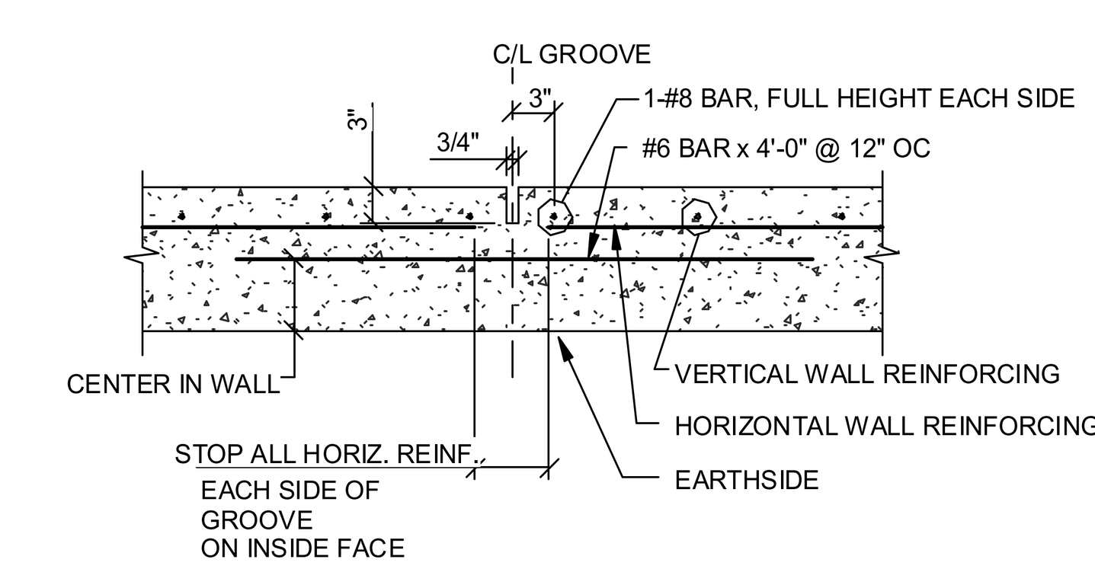
A2 Section At Knee Wall



NOTE:
1. SEE FOUNDATION PLAN FOR PROPOSED LOCATIONS. CONTRACTOR TO SUBMIT JOINT LOCATIONS FOR REVIEW IF DIFFERENT THAN INDICATED ON DRAWINGS.

3/4" = 1'-0"

B5 Plan Of Wall Expansion Joint



NOTE:
1. SEE FOUNDATION PLAN FOR PROPOSED LOCATIONS. CONTRACTOR TO SUBMIT JOINT LOCATIONS FOR REVIEW IF DIFFERENT THAN INDICATED ON DRAWINGS.

3/4" = 1'-0"

A5 Plan Of Wall Contraction Joint

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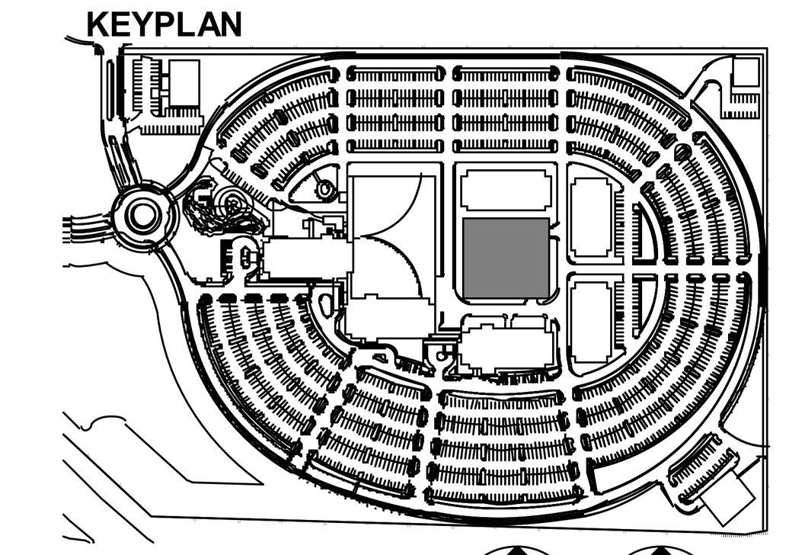
MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



TRUE NORTH PLAN NORTH

SCALE: 3/8" = 1'-0" UNLESS NOTED OTHERWISE

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Retaining Wall Schedules & Details

S-501

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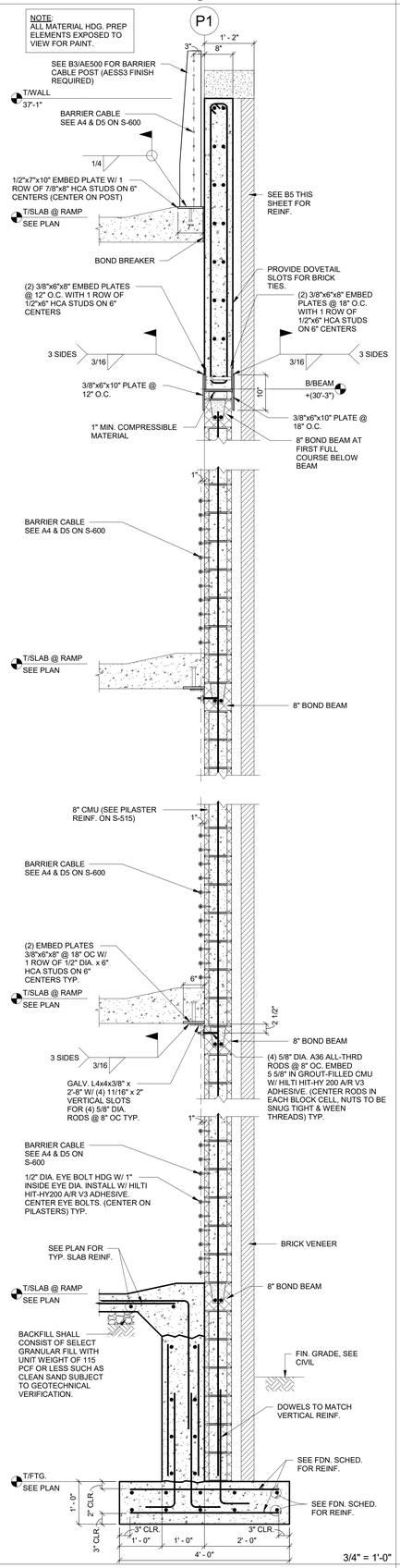
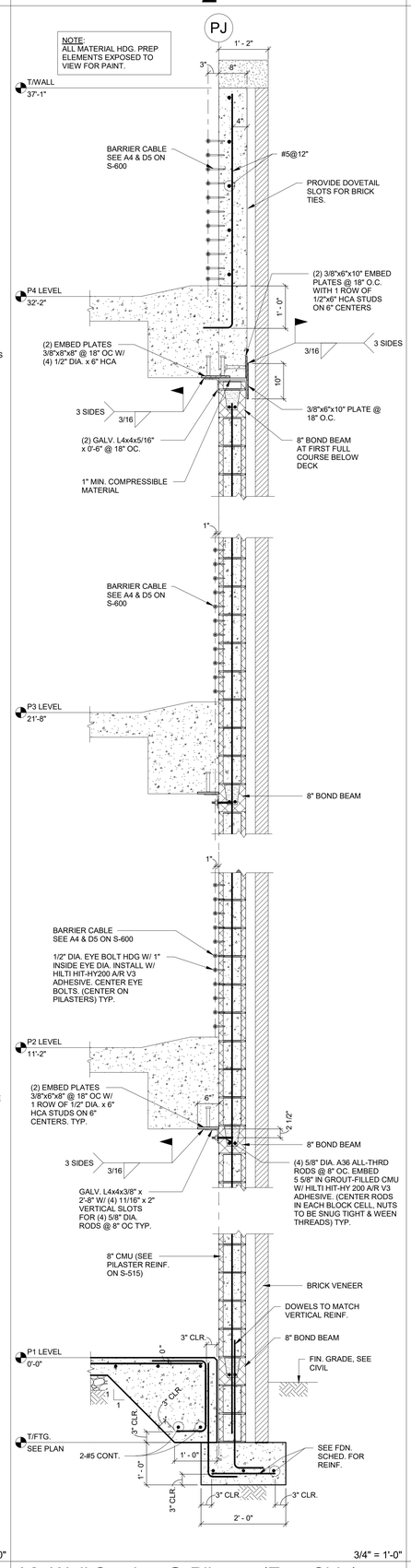
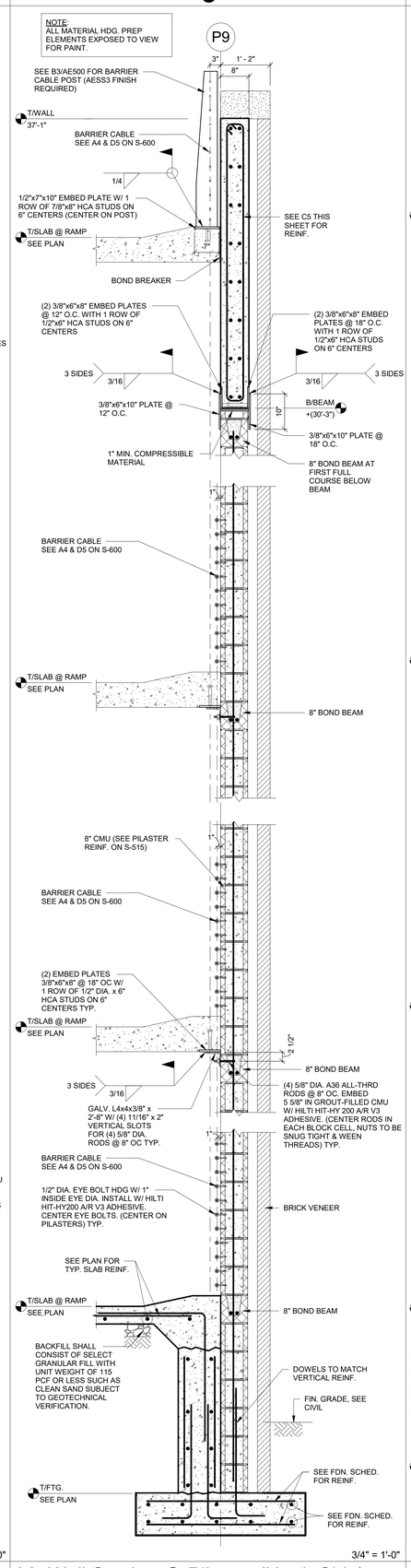


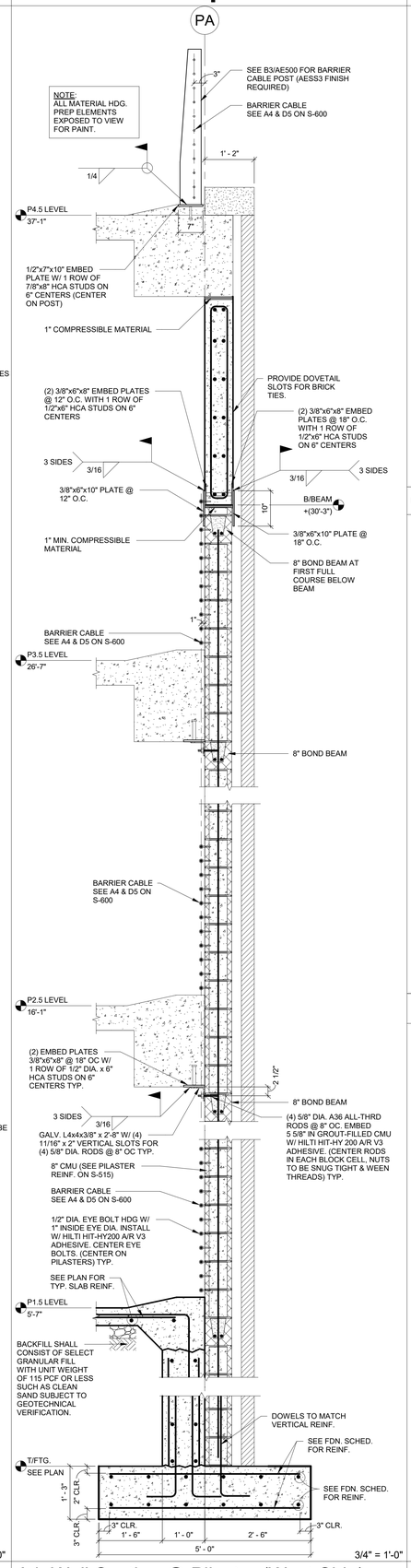
A1 Wall Section @ Pilaster (South Side)



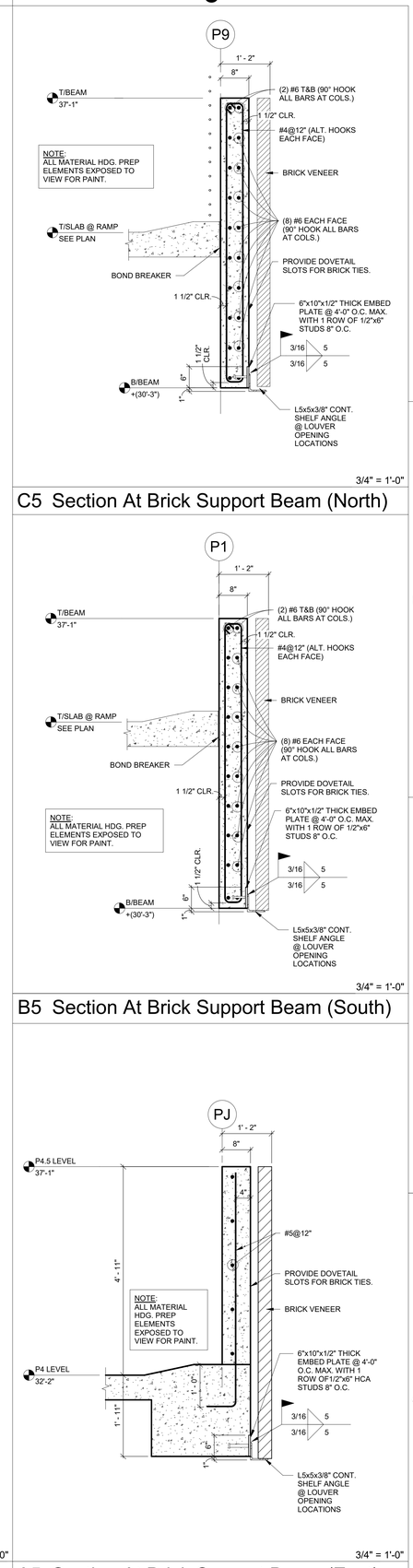
A2 Wall Section @ Pilaster (East Side)



A3 Wall Section @ Pilaster (North Side)



A4 Wall Section @ Pilaster (West Side)



C5 Section At Brick Support Beam (North)

B5 Section At Brick Support Beam (South)

A5 Section At Brick Support Beam (East)

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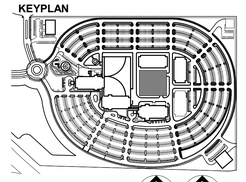
KEYPLAN



TRUE NORTH PLAN NORTH

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Wall Sections

S-502

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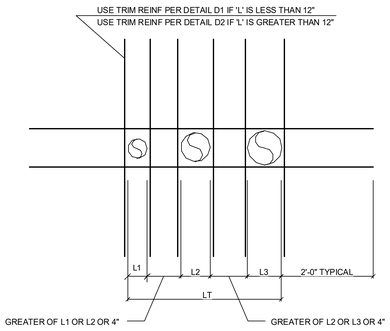
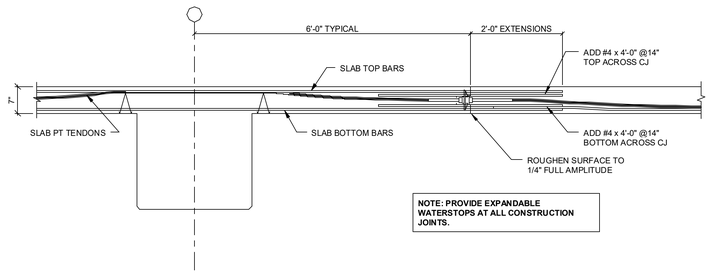


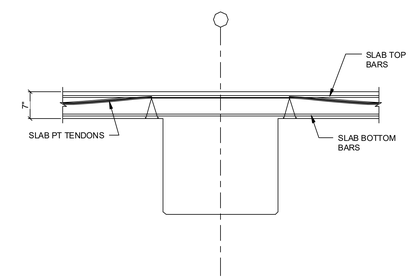
3/4" = 1'-0"

C1 Typical Slab Multiple Openings



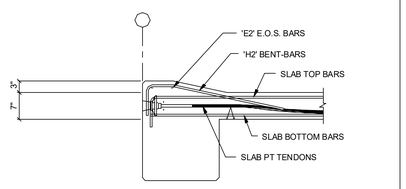
3/4" = 1'-0"

C2 Typical Slab Construction Joint



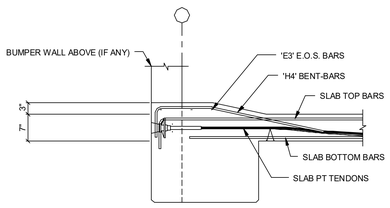
3/4" = 1'-0"

B1 Slab At Beam Midspan



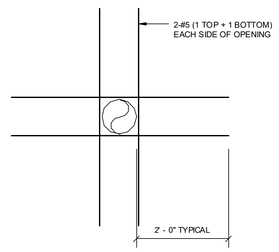
3/4" = 1'-0"

B2 Slab At 20"x23" Beam Midspan



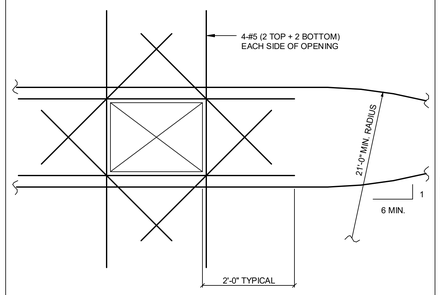
3/4" = 1'-0"

B3 Slab At "Bumpout" On P4 & P4.5



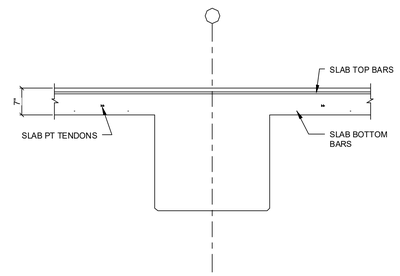
3/4" = 1'-0"

B4 Typical Slab Opening Less Than 12"



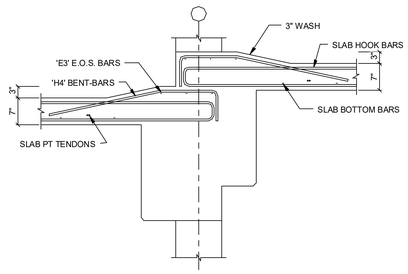
3/4" = 1'-0"

B5 Typical Slab Opening More Than 12"



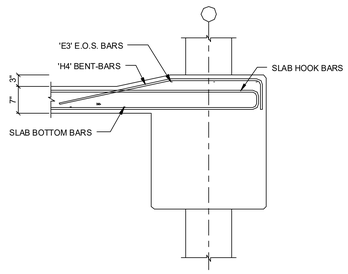
3/4" = 1'-0"

A1 Slab At Girder Midspan



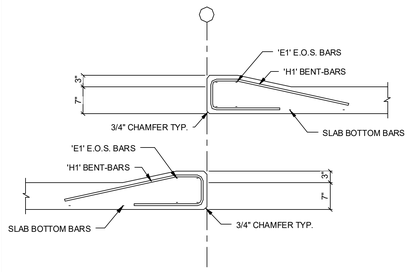
3/4" = 1'-0"

A2 Slab At Girder Backspan (Ramp)



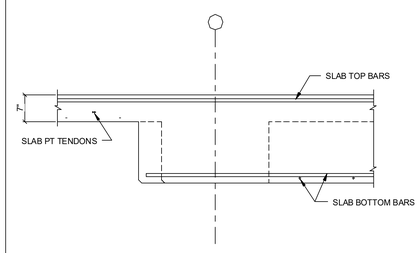
3/4" = 1'-0"

A3 Slab At Girder Backspan (Open)



3/4" = 1'-0"

A4 Slab At Ramp Edges



3/4" = 1'-0"

A5 23" Slab At Core Area

1 2 3 4 5

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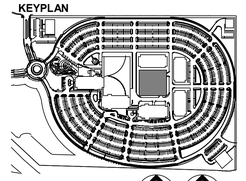
KEYPLAN



TRUE NORTH PLAN NORTH

0 8" 1'-4" 2'-8"
SCALE: 3/4" = 1'-0"
UNLESS NOTED OTHERWISE

PROJECT NO: 0470.62
DATE: 00.00.2025

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Slab Sections

S-505

Printed on: 9/2/2025 1:57:01 PM
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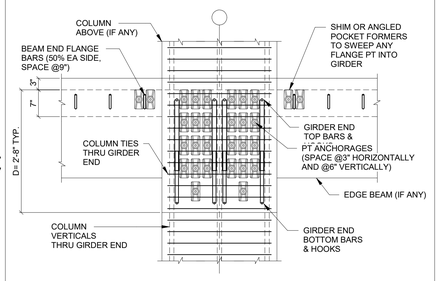
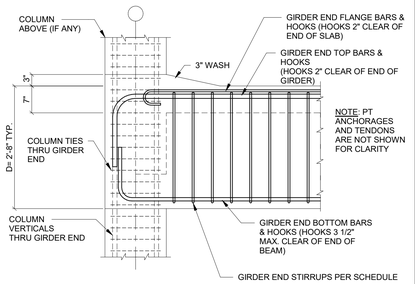


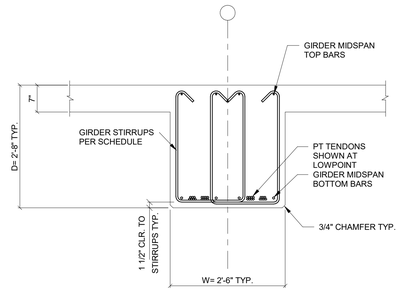
3/4" = 1'-0"

C1 30"x32" Girder End View



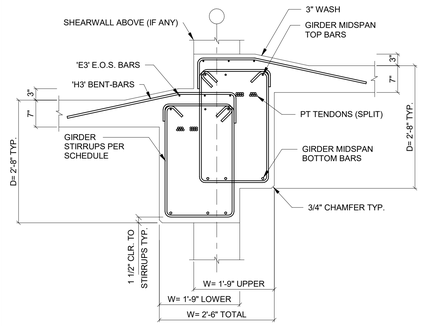
3/4" = 1'-0"

C2 30"x32" Girder Side View



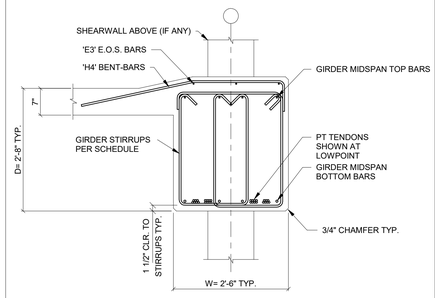
3/4" = 1'-0"

C3 30"x32" Girder Midspan



3/4" = 1'-0"

C4 21"x32" Girder Backspan (Ramp)



3/4" = 1'-0"

C5 30"x32" Girder Backspan (Open)

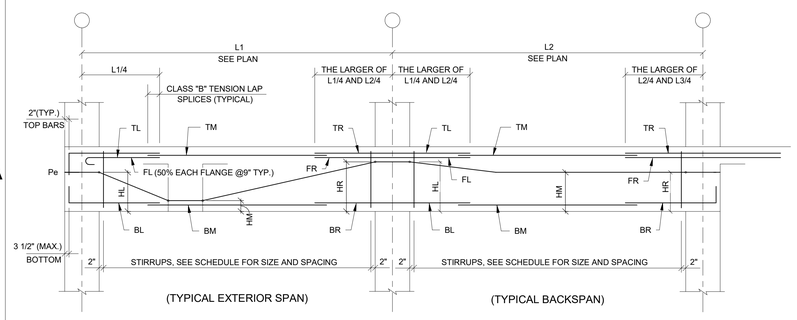
GIRDER LAP SPLICE SCHEDULE

BAR	CLASS 'A' TENSION		CLASS 'B' TENSION		COMPRESSION	
	TOP BAR	OTHER BAR	TOP BAR	OTHER BAR	DEVELOP	COMP LAP
	Ldt	Ldo	Lbt	Lbo	Ldc	
#5	21"	16"	27"	21"	12"	19"
#6	30"	23"	39"	30"	14"	23"
#7	51"	39"	66"	51"	16"	27"
#8	67"	51"	86"	67"	18"	30"
#9	85"	65"	110"	85"	21"	34"
#10	107"	83"	139"	107"	23"	39"

TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW BAR.
WHEN LAPPING A SMALLER BAR WITH A LARGER BAR, USE THE LAP LENGTH OF THE SMALLER BAR.

GIRDER REINFORCING SCHEDULE

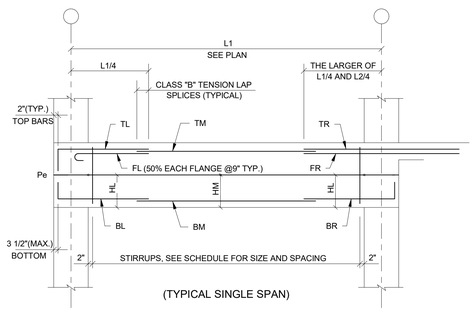
MARK	SHAPE	DIMENSIONS WIDTH	DIMENSIONS DEPTH	POST-TENSIONING				TOP BEAM BARS			TOP FLANGE BARS		BOTTOM BEAM BARS			BEAM STIRRUPS			BAR DIAGRAM SCHEMATIC SUPPORT LEFT	BAR DIAGRAM SCHEMATIC SUPPORT RIGHT	REMARKS
		W	D	Pe	HL	HM	HR	TL	TM	TR	FL	FR	BL	BM	BR	SIZE	TYPE	SPACING			
PTG1		30"	32"	810 K	20.7"	2.5"	29.5"	4-#9 H	4-#9	4-#10 C	6-#4 H	6-#4 C	4-#10 H	11-#9	4-#8 C	#4		27@5" EE, BAL @ 6"			PT PROFILE IS HARPED WITH LOWPOINT AT SUPPORTED BEAMS
PTG2U		21"	32"	405 K	29.5"	26.0"	20.7"	P	4-#6	3-#8 H	P	3-#4 H	P	3-#7	3-#6 H	#4		19@5" EE, BAL @ 7"			
PTG2L		21"	32"	405 K	29.5"	26.0"	20.7"	P	4-#6	3-#8 H	P	3-#4 H	P	3-#7	3-#6 H	#4		19@5" EE, BAL @ 7"			
PTG3		30"	32"	810 K	20.7"	2.5"	29.5"	4-#9 H	4-#9	4-#10 C	6-#4 H	6-#4 C	4-#10 H	11-#9	4-#8 C	#4		27@5" EE, 19@6" EE, BAL @ 8"			PT PROFILE IS HARPED WITH LOWPOINT AT SUPPORTED BEAMS
PTG4		30"	32"	810 K	29.5"	26.0"	20.7"	P	4-#6	4-#8 H	P	3-#4 H	P	4-#8	4-#6 H	#4		19@5" EE, 5@6" EE, BAL @ 7"			



MILD REINFORCING LEGEND:

TL = TOP LEFT
TM = TOP MIDDLE
TR = TOP RIGHT
FL = FLANGE LEFT
FM = FLANGE MIDDLE
FR = FLANGE RIGHT
BL = BOTTOM LEFT
BM = BOTTOM MIDDLE
BR = BOTTOM RIGHT
H = STANDARD HOOK AT END OF BEAM
C = CONTINUE INTO NEXT CONTINUOUS SPAN
E = EXTEND INTO ADJACENT OFFSET SPAN
P = SEE PREVIOUS CONTINUOUS SPAN FOR BARS

CONT = BARS ARE CONTINUOUS AS SHOWN IN SCHEMATIC



3/4" = 1'-0"

A1 Typical Girder Reinforcing Placing Diagram

GIRDER NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. CONCRETE STRENGTH SHALL BE 3000 PSI AT TIME OF STRESSING AND 5000 PSI MINIMUM AT 28 DAYS. ALL POST-TENSIONED TENDONS MUST BE STRESSED WITHIN 96 HOURS OF CASTING CONCRETE.
3. ELEVATED POST-TENSIONED GIRDERS SHALL BE AN ENCAPSULATED SYSTEM WITH POST-TENSIONED TENDONS THAT CONSIST OF 1/2" DIA. 270 KSI UNBONDED WIRE TENDONS. SLAB POST-TENSIONING FORCES GIVEN ARE EFFECTIVE PRESTRESS FORCES AFTER ALL SHORT AND LONG-TERM LOSSES AS SUBSTANTIATED BY CALCULATIONS PROVIDED BY THE PT SUPPLIER'S ENGINEER. IN NO CASE SHALL GREATER THAN 27 KIPS PER TENDON BE USED TO DETERMINE TENDON QUANTITIES PROVIDED.
4. UNLESS NOTED OTHERWISE ON PLAN, FOR BEAMS, PROVIDE "L/10" REVERSE-PARABOLIC TENDON PROFILES WITH HIGHPOINTS SCHEDULED AT EACH CENTERLINE OF SUPPORTING COLUMN OR GIRDER, AND LOWPOINTS SCHEDULED AT MIDSPAN. FOR GIRDERS, PROVIDE STRAIGHT-LINE "HARPED" TENDON PROFILES WITH HIGHPOINTS SCHEDULED AT EACH CENTERLINE OF COLUMN, AND LOWPOINTS SCHEDULED AT TRANSVERSE BEAM.
5. ADDED TENDONS SHALL FOLLOW THE SAME PROFILE GIVEN AND EXTEND INTO THE ADJACENT SPAN THE GREATER OF "L/5" OF THE MAIN SPAN OR "L/5" OF THE ADJACENT SPAN.
6. REBAR SHALL BE GRADE 60 KSI WITH STANDARD CLASS B LAPS AND STANDARD HOOKS PLACED 2" CLEAR OF END OF GIRDER U.N.O. USE STIRRUP BENDS FOR GIRDER STIRRUPS AND SMALL SECTION BENDS.
7. FOR INTERIOR SPANS, CENTER GIRDER TOP BARS ON SUPPORT CENTER AND CENTER GIRDER BOTTOM BARS ON MIDSPAN UNLESS SHOWN OTHERWISE. FOR EXTERIOR SPANS, START GIRDER TOP REBAR HOOKS AND GIRDER BOTTOM HOOKS AS SHOWN THIS SHEET.
8. ALL GIRDER TOP BARS NOTED AS CONTINUOUS SHALL USE CLASS "B" LAPS AT MIDSPAN. ALL GIRDER BOTTOM BARS NOTED AS CONTINUOUS SHALL USE CLASS "B" LAPS AT GIRDER SUPPORTS. SEE LAP SCHEDULE TO DETERMINE ALL LAP LENGTHS. USE CLASS 'B' LAPS FOR ALL LAPS UNLESS NOTED OTHERWISE.
9. ALL GIRDER REBAR COVER SHALL BE 1 1/2" CLEAR TO STIRRUPS ALL SIDES. AT INTERSECTION OF BEAMS WITH GIRDERS, PLACE BEAM TOP BARS IN TOPMOST LAYER AND GIRDER BOTTOM BARS IN BOTTOM MOST LAYER.
10. SEE REINFORCING PLAN SHEETS FOR ALL GIRDER MARKS.
11. GIRDER SIDES MAY TAPER 3/4" EACH SIDE.

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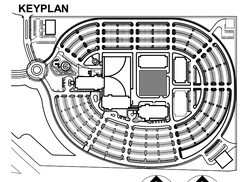

TRUE NORTH PLAN NORTH

PROJECT NO: 0470.62

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Girder Schedule, Placing Diagram & Sections

S-506

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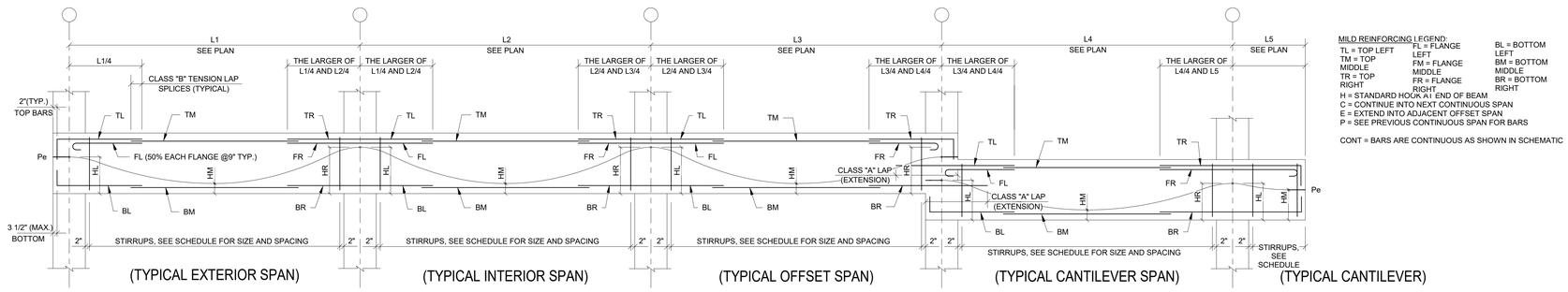
BEAM REINFORCING SCHEDULE

MARK	SHAPE	DIMENSIONS WIDTH	DIMENSIONS DEPTH	POST-TENSIONING Pe	POST-TENSIONING HL	POST-TENSIONING HM	POST-TENSIONING HR	TOP BEAM BARS TL	TOP BEAM BARS TM	TOP BEAM BARS TR	TOP FLANGE BARS FL	TOP FLANGE BARS FR	BOTTOM BEAM BARS BL	BOTTOM BEAM BARS BM	BOTTOM BEAM BARS BR	BEAM STIRRUPS SIZE	BEAM STIRRUPS TYPE	BEAM STIRRUPS SPACING	BAR DIAGRAM SCHEMATIC SUPPORT LEFT / SUPPORT RIGHT	REMARKS
PTB1		20"	23"	216 K	11.5"	11.5"	14.3"	3-#7 H	3-#5	3-#7 C	3-#4 H	3-#4 H	3-#6 H	5-#7	4-#7 C	#4		4@4"EE, BAL@7"		
PTB2		20"	23"	216 K	14.3"	8.0"	20.5"	P	3-#5	3-#7 C	P	3-#4 H	P	5-#7	4-#7 C	#4		4@4"EE, BAL@7"		
PTB3		20"	23"	216 K	20.5"	10.0"	20.5"	P	3-#5	3-#7 C	P	3-#4 H	P	5-#7	4-#7 C	#4		4@4"EE, BAL@7"		
PTB4		20"	23"	216 K	20.5"	10.0"	20.5"	P	3-#5	3-#7 C	P	3-#4 H	P	5-#7	4-#7 C	#4		4@4"EE, BAL@7"		
PTB5		20"	23"	216 K	20.5"	8.0"	14.3"	P	3-#5	3-#7 C	P	3-#4 H	P	5-#7	4-#7 C	#4		4@4"EE, BAL@7"		
PTB6		20"	23"	216 K	14.3"	11.5"	11.5"	P	3-#7	2-#7 H + 1-#7 C	P	3-#4 H	P	5-#7	3-#7 H + 1-#7 C	#4		4@4"EE, BAL@7"		
PTB7		20"/8"	23"	108 K	16.0"	16.0"	16.0"	P	3-#7	-	P	3-#4 H	P	5-#7	-	-	-	ALL @ 6"		23" THICK SLAB AREA CANTILEVERS TO SUPPORT STAIRS.
PTB8		18"	23"	324 K	11.5"	11.5"	20.5"	4-#8 H	CONT	4-#8 C	3-#4 H	3-#4 C	2-#8 H + 1-#8 H	CONT + CONT	2-#8 C + 1-#8 H	#4		ALL @ 6" O.C.		PLACE "+ 1-#8 H" BOTTOM BAR AT OFFSET FACE THAT IS 1'-8" OFF GRIDLINE PER BAR DIAGRAM SCHEMATIC.
PTB9		28"	32"	567 K	20.5"	13.0"	25.0"	P + 2-#8 H	6-#6	6-#8 C	P + 3-#4 H	6-#4 C	P + 1-#8 H	5-#9	5-#7 E	#4		4@4"EE, 12@7"EE, BAL@14"		PLACE "+ 1-#8 H" TOP & BOTTOM BAR AT OFFSET FACE OF CORNER TOWER PER BAR DIAGRAM SCHEMATIC.
PTB10		28"	32"	567 K	25.0"	2.5"	25.0"	P	6-#6	6-#8 C	P	6-#4 C	5-#7 H	5-#9	5-#7 H	#4		12@7"EE, BAL@14"		
PTB11		28"	32"	567 K	25.0"	13.0"	20.5"	P	6-#6	6-#8 CH + 2-#8 H	P	6-#4 CH	5-#7 E	5-#9	5-#7 CH	#4		6@4"EE, 12@7"EE, BAL@14"		486 K CONTINUES TO 14.5" CGS END AT ELEVATOR OPENING. 'TR' & 'BR' BARS CONTINUE TO HOOK AT ELEVATOR OPENING.
PTB12		34"/18"	23"	324 K	20.5"	9.5"	11.5"	4-#8 C	CONT	4-#8 H	3-#4 C	3-#4 H	5-#8 H	CONT	5-#8 H	#4		ALL @ 8"		PLACE 216 K AND ALL TOP, BOTTOM & STIRRUPS WITHIN 34" WIDE STRIP OF 23" THICK CORE AREA.
PTB13		30"	32"	621 K	20.7"	2.5"	28.5"	5-#8 H	5-#6	5-#8 C	6-#4 H	6-#4 C	5-#7 H	5-#9	5-#7 C	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB14		30"	32"	621 K	28.5"	6.0"	28.5"	P	5-#6	5-#8 C	P	6-#4 C	P	5-#9	5-#7 C	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB15		30"	32"	621 K	28.5"	2.5"	20.7"	P	5-#6	5-#8 H	P	6-#4 H	P	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB16		30"	32"	621 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		PROVIDE STIRRUP CLOSURE BAR AT 19'-0" EE
PTB17		30"	32"	621 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		PROVIDE STIRRUP CLOSURE BAR AT 19'-0" EE
PTB18		30"	32"	621 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 E	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		PROVIDE STIRRUP CLOSURE BAR AT 19'-0" EE
PTB19		30"	32"	621 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB20		30"	32"	621 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB21		30"	32"	621 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB22		30"	32"	702 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB23		30"	32"	702 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB24		30"	32"	702 K	20.7"	2.5"	20.7"	5-#8 H	5-#6	5-#8 H	6-#4 H	6-#4 H	5-#7 H	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB25		30"	32"	621 K	20.7"	2.5"	28.5"	5-#8 H	5-#6	5-#8 C	6-#4 H	3-#4 C + 3-#4 H	5-#7 H	5-#9	5-#7 C	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB26		30"	32"	621 K	28.5"	6.0"	28.5"	P	5-#6	5-#8 C	P	3-#4 C	P	5-#9	5-#7 C	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB27		30"	32"	621 K	28.5"	2.5"	20.7"	P	5-#6	5-#8 H	P + 3-#4 H	6-#4 H	P	5-#9	5-#7 H	#4		8@4"EE, 18@7"EE, BAL@10"		
PTB28		12"/28"	23"	81 K	11.5"	11.5"	11.5"	3-#7 H + 1-#7 H	CONT + CONT	3-#7 C + 1-#7 H	-	-	3-#7 H + 1-#7 H	CONT + CONT	3-#7 C + 1-#7 H	#4		4@4"EE, BAL@6"		PLACE "+ 1-#7 H" TOP & BOTTOM BAR AT OUTSIDE FACE PER BAR DIAGRAM SCHEMATIC.
PTB29		20"	23"	216 K	11.5"	11.5"	14.3"	P	3-#5 + 1-#5 H	4-#7 C	3-#4 H	3-#4 H	P	5-#7 + 1-#7 H	4-#6 C	#4		4@4"EE, BAL@7"		START ADDED TENDONS 13" CGS AT CHANGE IN WIDTH. START TM & BM BARS AT CHANGE IN WIDTH. STIRRUP WIDTH VARIES.
PTB30		28"	23"	216 K	14.3"	8.0"	20.5"	P	4-#5	4-#7 C	P	3-#4 H	P	5-#7	4-#6 C	#4		4@4"EE, BAL@7"		
PTB31		28"	23"	216 K	20.5"	10.0"	20.5"	P	4-#5	4-#7 C	P	3-#4 H	P	5-#7	4-#6 C	#4		4@4"EE, BAL@7"		
PTB32		28"	23"	216 K	20.5"	10.0"	20.5"	P	4-#5	4-#7 C	P	3-#4 H	P	5-#7	4-#6 C	#4		4@4"EE, BAL@7"		
PTB33		28"	23"	216 K	20.5"	8.0"	14.3"	P	4-#5	4-#7 C	P	3-#4 H	P	5-#7	4-#6 C	#4		4@4"EE, BAL@7"		
PTB34		20"	23"	216 K	14.3"	11.5"	11.5"	P	3-#5 + 1-#5 H	3-#7 CH	P	3-#4 CH	P	5-#7 + 1-#7 H	3-#6 CH	#4		6@4"EE, BAL@7"		END ADDED TENDONS AT 13" CGS AT CHANGE IN WIDTH. END TM & BM BARS AT CHANGE IN WIDTH. STIRRUP WIDTH VARIES.
PTB35		12"	23"	81 K	11.5"	11.5"	11.5"	P + 2-#7 H	CONT	3-#7 H	P	-	P + 2-#7 H	CONT	2-#7 H	#4		4@4"EE, BAL@6"		END TR & BR BARS FROM PREVIOUS SPAN WITH HOOK AT END OF STAIR LANDING.
PTB36		18"	23"	135 K	11.5"	11.5"	11.5"	4-#8 H	4-#6	4-#8 C	3-#4 H	3-#4 C	3-#8 H	CONT	3-#8 H	#4		4@4"EE, 11@6"EE, BAL@8"		PT PROFILE IS STRAIGHT
PTB37		34"	23"	324 K	11.5"	11.5"	11.5"	5-#8 H	5-#6	5-#8 C	6-#4 H	6-#4 C	5-#6 H	CONT	5-#8 H	#4		10@5"EE, BAL@8"		PT PROFILE IS STRAIGHT. PLACE TENDONS AND ALL TOP, BOTTOM & STIRRUPS WITHIN 34" WIDE STRIP OF 23" THICK CORE AREA.
PTB38		12"	23"	81 K	11.5"	9.0"	11.5"	3-#7 H	CONT	3-#7 H	-	-	3-#7 H	CONT	3-#7 H	#4		4@4"EE, BAL@6"		
PTB39		8"	23"	40 K	11.5"	11.5"	11.5"	2-#7 H	CONT	2-#7 H	-	-	2-#7 H	CONT	2-#7 H	#4		ALL@4"		BEAM AT INTERMEDIATE STAIR LANDING. BEAM WIDENS TO 28" TO FILL-IN L-SHAPED CORNER COLUMN.
PTB40		28"	23"	216 K	20.5"	11.5"	11.5"	P	3-#5 + 1-#5 H	3-#7 CH	P	3-#4 CH	P	3-#7 + 1-#7 H	3-#6 CH	#4		4@4"EE, BAL@7"		
PTB41		20"	23"	216 K	11.5"	6.0"	11.5"	P	4-#5	4-#7 C	P	3-#4 H	P	5-#7	5-#7 C	#4		4@4"EE, BAL@7"		
PTB42		20"	23"	216 K	20.5"	11.5"	11.5"	P	4-#7	4-#7 CH	P	3-#4 CH	P	5-#7	5-#7 CH	#4		4@4"EE, BAL@7"		
B1		8"	23"	-	-	-	-	2-#6 H	CONT	2-#6 H	-	-	2-#6 H	CONT	2-#6 H	#4		ALL@8"		ADD 1-#6 EACH FACE AT MID-DEPTH CONT. W/ STD HOOK AT SUPPORTS
B2		12"	23"	-	-	-	-	2-#7 H	CONT	2-#7 H	-	-	2-#7 H	CONT	2-#7 H	#4		ALL@8"		ADD 1-#7 EACH FACE AT MID-DEPTH CONT. W/ STD HOOK AT SUPPORTS

BEAM LAP SPLICE SCHEDULE

BAR	CLASS 'A' TENSION TOP BAR Ldt	CLASS 'A' TENSION OTHER BAR Ldo	CLASS 'B' TENSION TOP BAR Lbt	CLASS 'B' TENSION OTHER BAR Lbo	COMPRESSION DEVELOP Ldc	COMPRESSION COMP LAP
#5	27"	21"	35"	27"	12"	19"
#6	32"	25"	42"	32"	14"	23"
#7	38"	29"	49"	38"	16"	27"
#8	43"	33"	56"	43"	18"	30"
#9	48"	37"	63"	48"	21"	34"
#10	54"	42"	70"	54"	23"	38"

TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW BAR. WHEN LAPPING A SMALLER BAR WITH A LARGER BAR, USE THE LAP LENGTH OF THE SMALLER BAR.



MILD REINFORCING LEGEND:

TL = TOP LEFT	FL = FLANGE LEFT	BL = BOTTOM LEFT
TM = TOP MIDDLE	FM = FLANGE MIDDLE	BM = BOTTOM MIDDLE
TR = TOP RIGHT	FR = FLANGE RIGHT	BR = BOTTOM RIGHT

H = STANDARD HOOK AT END OF BEAM
C = CONTINUE INTO NEXT CONTINUOUS SPAN
E = EXTEND INTO ADJACENT OFFSET SPAN
P = SEE PREVIOUS CONTINUOUS SPAN FOR BARS

CONT = BARS ARE CONTINUOUS AS SHOWN IN SCHEMATIC

3/4" = 1'-0"

A1 Typical Beam Reinforcing Placing Diagram

BEAM NOTES:

1. SEE S-001 FOR GENERAL NOTES.
2. CONCRETE STRENGTH SHALL BE 3000 PSI AT TIME OF STRESSING AND 5000 PSI MINIMUM AT 28 DAYS. ALL POST-TENSIONED TENDONS MUST BE STRESSED WITHIN 96 HOURS OF CASTING CONCRETE.
3. ELEVATED POST-TENSIONED BEAMS SHALL BE AN ENCAPSULATED SYSTEM WITH POST-TENSIONED TENDONS THAT CONSIST OF 1/2" DIA. 270 KSI UNBONDED WIRE TENDONS. SLAB POST-TENSIONING FORCES GIVEN ARE EFFECTIVE PRESTRESS FORCES AFTER ALL SHORT AND LONG-TERM LOSSES AS SUBSTANTIATED BY CALCULATIONS PROVIDED BY THE PT SUPPLIER'S ENGINEER. IN NO CASE SHALL GREATER THAN 27 KIPS PER TENDON BE USED TO DETERMINE TENDON QUANTITIES PROVIDED.
4. UNLESS NOTED OTHERWISE ON PLAN, FOR BEAMS, PROVIDE "L/10" REVERSE-PARABOLIC TENDON PROFILES WITH HIGHPOINTS SCHEDULED AT EACH CENTERLINE OF SUPPORTING COLUMN OR GIRDER, AND LOWPOINTS SCHEDULED AT MIDSPAN. FOR GIRDERS, PROVIDE STRAIGHT-LINE "HARPED" TENDON PROFILES WITH HIGHPOINTS SCHEDULED AT EACH CENTERLINE OF COLUMN, AND LOWPOINTS SCHEDULED AT TRANSVERSE BEAM.
5. ADDED TENDONS SHALL FOLLOW THE SAME PROFILE GIVEN AND EXTEND INTO THE ADJACENT SPAN THE GREATER OF "L/5" OF THE MAIN SPAN OR "L/5" OF THE ADJACENT SPAN.
6. REBAR SHALL BE GRADE 60 KSI WITH STANDARD CLASS B LAPS AND STANDARD HOOKS PLACED 2" CLEAR OF END OF BEAM U.N.O. USE STIRRUP BENDS FOR BEAM STIRRUPS AND SMALL SECTION BENDS.
7. FOR INTERIOR SPANS, CENTER BEAM TOP BARS ON SUPPORT CENTER AND CENTER BEAM BOTTOM BARS ON MIDSPAN UNLESS SHOWN OTHERWISE. FOR EXTERIOR SPANS, START BEAM TOP REBAR HOOKS AND BEAM BOTTOM HOOKS AS SHOWN THIS SHEET.
8. ALL BEAM TOP BARS NOTED AS CONTINUOUS SHALL USE CLASS "B" LAPS AT MIDSPAN. ALL BEAM BOTTOM BARS NOTED AS CONTINUOUS SHALL USE CLASS "B" LAPS AT BEAM SUPPORTS. SEE LAP SCHEDULE TO DETERMINE ALL LAP LENGTHS. USE CLASS 'B' LAPS FOR ALL LAPS UNLESS NOTED OTHERWISE.
9. ALL BEAM REBAR COVER SHALL BE 1 1/2" CLEAR TO STIRRUPS ALL SIDES. AT INTERSECTION OF BEAMS WITH GIRDERS, PLACE BEAM TOP BARS IN TOPMOST LAYER AND GIRDER BOTTOM BARS IN BOTTOMMOST LAYER.
10. SEE REINFORCING PLAN SHEETS FOR ALL BEAM MARKS.
11. BEAM SIDES MAY TAPER 3/4" EACH SIDE.

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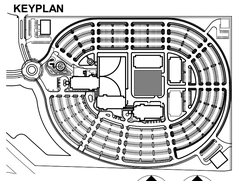
CAROLINA FOREST
PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



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Beam Schedule & Placing Diagram

S-507

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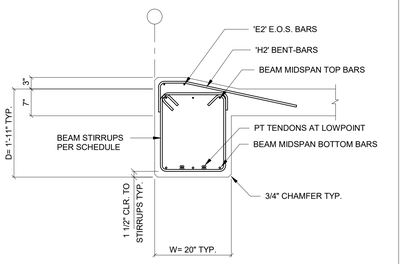
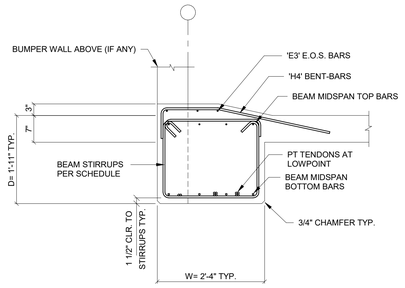


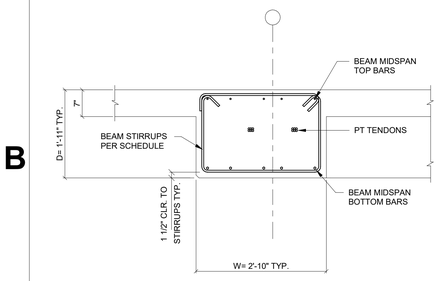
3/4" = 1'-0"

C1 20"x23" Beam Midspan



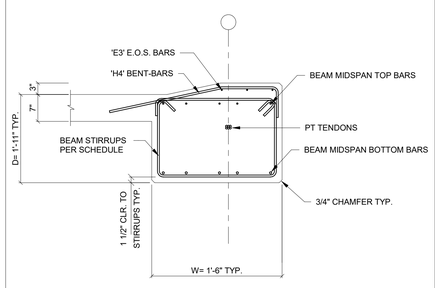
3/4" = 1'-0"

C2 28"x23" Beam At P4 & P4.5



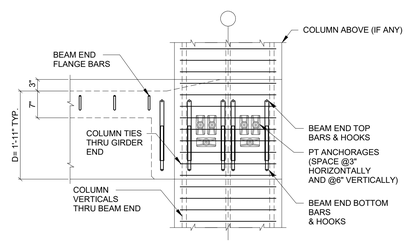
3/4" = 1'-0"

B1 34"x23" Beam Midspan



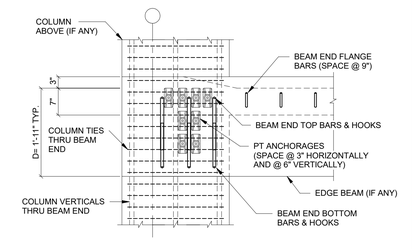
3/4" = 1'-0"

B2 18"x23" Beam Midspan



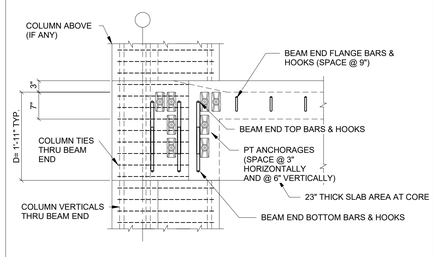
3/4" = 1'-0"

B3 18"x23" Beam End View



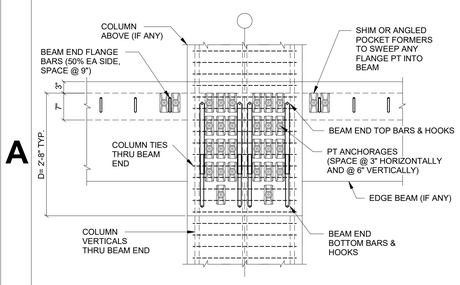
3/4" = 1'-0"

B4 20"x23" Beam End View



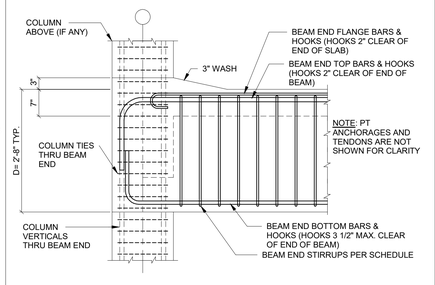
3/4" = 1'-0"

B5 20"/8"x23" Beam End View



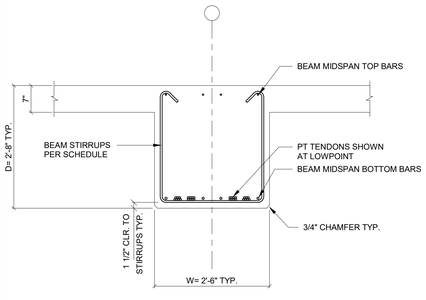
3/4" = 1'-0"

A1 30"x32" Beam End View



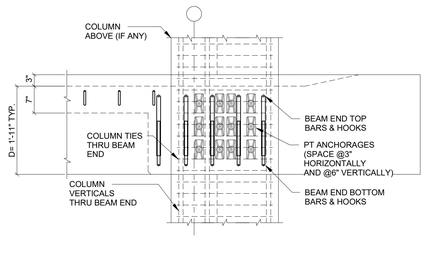
3/4" = 1'-0"

A2 30"x32" Beam Side View



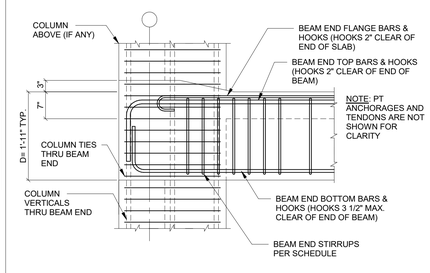
3/4" = 1'-0"

A3 30"x32" Beam Midspan



3/4" = 1'-0"

A4 34"x23" Beam End View



3/4" = 1'-0"

A5 34"x23" Beam Side View

1 2 3 4 5

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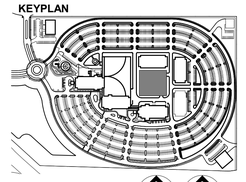
CAROLINA FOREST PARKING DECK

MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



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Beam Sections

S-508

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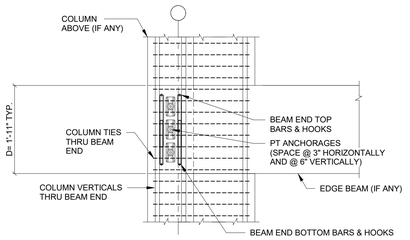
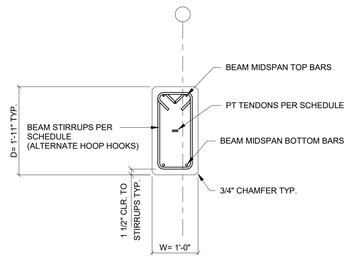


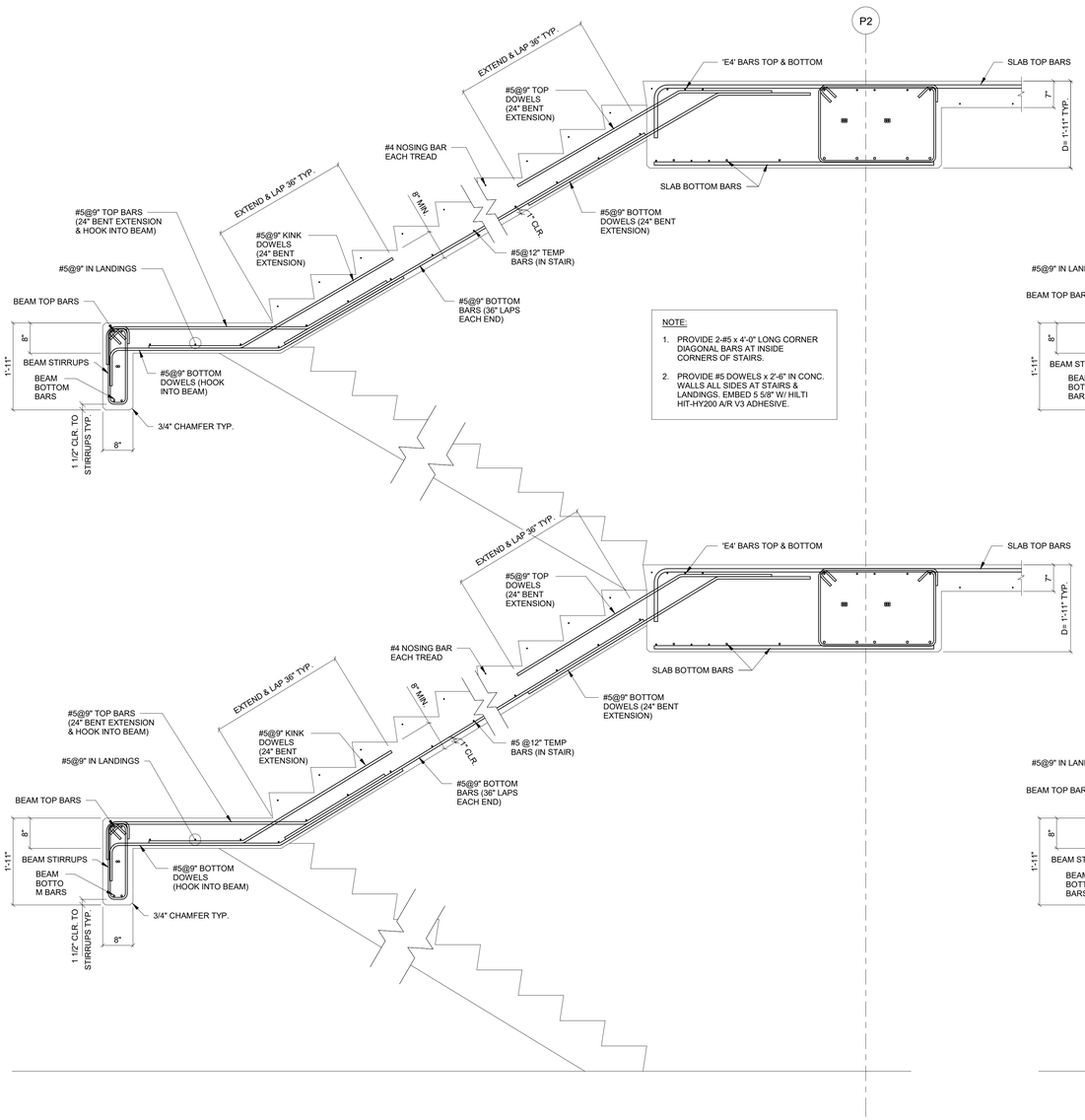
3/4" = 1'-0"

D1 12"x23" Beam North And South Tower On P4 & P4.5



3/4" = 1'-0"

D2 12"x23" Beam North And South Tower On P4 & P4.5



3/4" = 1'-0"

A1 Typical Stair Sections

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CAROLINA FOREST PARKING DECK

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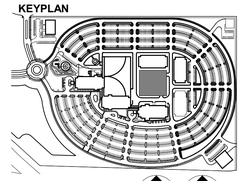

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Stair & Tower Sections

S-510

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MASONRY WALL REINFORCING SCHEDULE

WALL TYPE	WALL THICKNESS	MINIMUM VERTICAL WALL REINFORCEMENT CENTERED, UNO	BOND BEAMS		JOINT REINFORCING		NOTES
			TYPICAL BOND BEAM REINFORCEMENT	MAXIMUM INTERMEDIATE BOND BEAM SPACING	SIZE	SPACING	
ELECT, ELEVATOR, IT ROOM WALLS	8"	#4 @ 48"	2-#4	SEE SECTION C2/S-515 FOR BOND BEAM LOCATIONS. 10'-0" OC MAX	#9 GA	16" OC	STOPS AT STRUCTURE
PERIMETER PILASTER	8"	#5 @ 8"	2-#5	SEE SECTION C2/S-515 FOR BOND BEAM LOCATIONS. 10'-0" OC MAX	#9 GA	16" OC	STOPS AT STRUCTURE

NOTES:

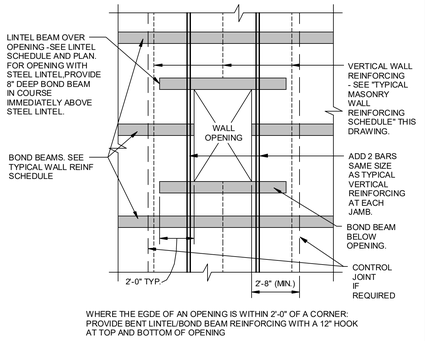
1. SEE "TYPICAL REINFORCING DETAIL AT MASONRY WALL OPENING", THIS DRAWING.
2. PROVIDE VERTICAL BARS IN LENGTHS NOT LESS THAN 10'-0" PLUS REQUIRED LAPS. SPLICES AND TIE VERTICAL BARS SECURELY PRIOR TO MASONRY PLACEMENT. PROVIDE CONDUIT BLOCK AT VERTICAL REINFORCING LOCATIONS.
3. PROVIDE REINFORCING POSITIONERS IN ALL WALLS PER DETAIL B3/S-515
4. SEE TYPICAL UNIT SHAPES, B1/S-515.
5. SEE DETAILS A1, A2, & A4/S-515 FOR TYPICAL REINFORCING DETAILS
6. FILL ALL MASONRY CELLS CONTAINING REINFORCING BARS WITH GROUT. SEE MASONRY NOTES FOR GROUT. ALL 3-HOUR RATED WALLS SHALL HAVE ALL CELLS FILLED W/ GROUT FROM BOTTOM OF WALL TO TOP OF WALL.
7. SEE THIS DWG. AND SECTIONS FOR WALL BRACING DETAILS.
8. LAP SPLICE ALL JOINT REINFORCING 12" MIN.

LINTEL SCHEDULE

LINTEL MARK	WALL TYPE	REINFORCING	DETAIL
5'-0" MAX	8" CMU	8"x8" LINTEL BLOCK W/ 2-#4 BARS	8" / 5"

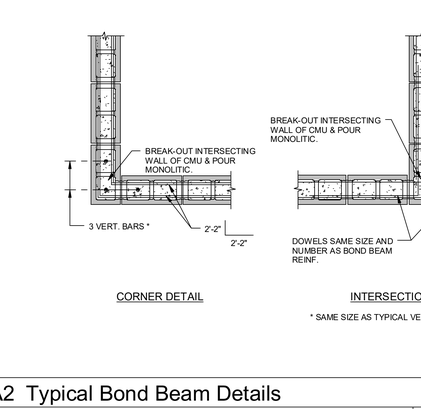
LINTEL SCHEDULE NOTES:

1. FILL ALL BOND BEAM LINTELS WITH 3000 PSI GROUT.
2. FILL ALL CELLS BELOW LINTEL BEARING TO FOUNDATION OR OTHER SUPPORT, WITH GROUT.
3. PROVIDE ADDITIONAL VERTICAL WALL REINFORCING EACH SIDE OF OPENING AS SHOWN ON "TYPICAL REINFORCING DETAIL AT MASONRY WALL OPENING", DETAIL A1/S-515.
4. PROVIDE SPLICE BAR FROM LINTEL TO VERTICAL WALL REINFORCING ABOVE AS FOLLOWS:
   A. AT BOND BEAM LINTEL: HOOK DOWEL INTO BOND BEAM. LAP DOWEL WITH VERTICAL WALL REINFORCING ABOVE LINTEL.
5. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, ETC FOR ADDITIONAL OPENINGS NOT NOTED ON STRUCTURAL DRAWINGS.
6. SEE C1/S-515 FOR LINTEL REINFORCING DETAIL.



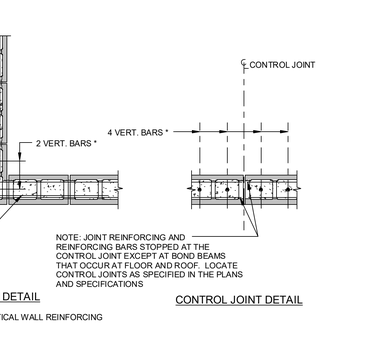
3/4" = 1'-0"

C1 Typical Lintel Reinforcing Detail



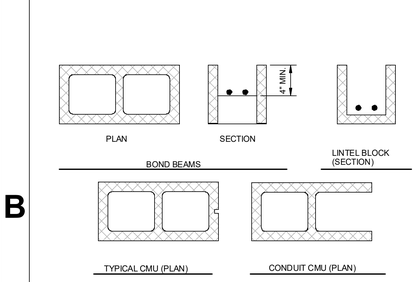
3/4" = 1'-0"

C2 Typical Wall Section



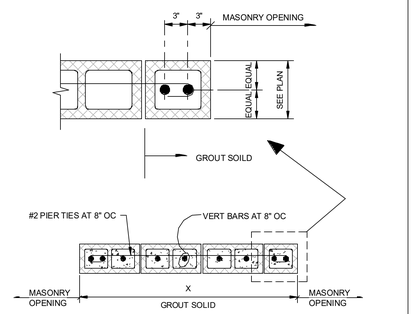
3/4" = 1'-0"

C3 Lateral Bracing Details



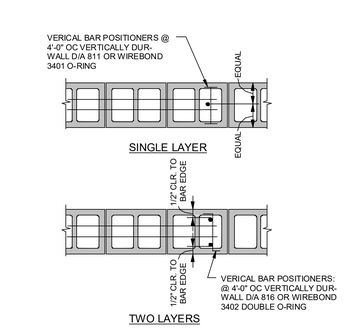
3/4" = 1'-0"

B1 Typical Masonry Units



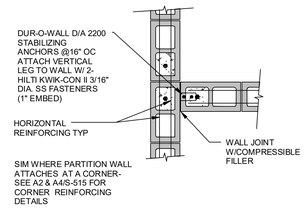
3/4" = 1'-0"

B2 Typical CMU Jamb Detail



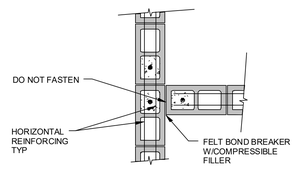
3/4" = 1'-0"

B3 Reinforcing Positioners



3/4" = 1'-0"

B4 Partition Joint



3/4" = 1'-0"

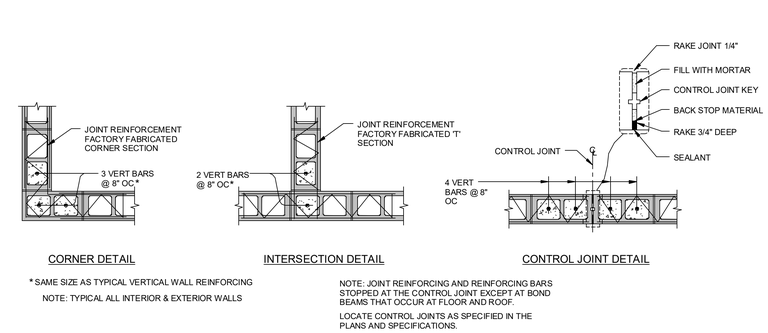
B5 Isolation Joint

3/4" = 1'-0"

A1 Reinforcing @ Masonry Wall Opening

3/4" = 1'-0"

A2 Typical Bond Beam Details



3/4" = 1'-0"

A4 Typical Wall Reinforcing Details

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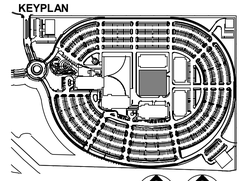
MYRTLE BEACH, SOUTH CAROLINA

KEYPLAN



TRUE NORTH PLAN NORTH

PROJECT NO: 0470.62

DATE: 00.00.2025

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Masonry Schedules, Sections & Details

S-515

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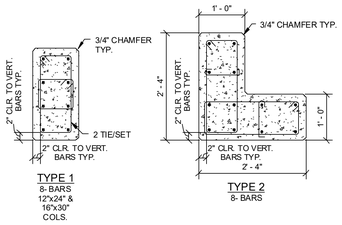
CONCRETE COLUMN SCHEDULE

COLUMN MARK	CC1	CC2	CC3
TOP OF COLUMN LEVEL P4 = +32'-2" LEVEL P4.5 = +37'-1"			
SIZE	16"x30"	12"x24"	12"x28" / 12"x28"
VERTICAL REINF	8-#9	8-#8	8-#8 (12 Total)
TIES	#4@12" UNO	#4@9" UNO	#4@12" UNO
TIE DETAIL	TYPE 1	TYPE 1	TYPE 2
LEVEL P3 = +21'-8" LEVEL P3.5 = +26'-7"			
SIZE	16"x30"	12"x24"	12"x28" / 12"x28"
VERTICAL REINF	8-#9	8-#8	8-#8 (12 Total)
TIES	#4@12" UNO	#4@9" UNO	#4@12" UNO
TIE DETAIL	TYPE 2	TYPE 1	TYPE 2
LEVEL P2 = +11'-2" LEVEL P2.5 = +16'-1"			
SIZE	16"x30"	12"x24"	12"x28" / 12"x28"
VERTICAL REINF	8-#9	8-#8	8-#8 (12 Total)
TIES	#4@12" UNO	#4@9" UNO	#4@12" UNO
TIE DETAIL	TYPE 1	TYPE 1	TYPE 2
LEVEL P1 (SEE PLAN)			
REMARKS	-		

NOTES:
1. SEE TYPICAL REINFORCEMENT DIAGRAM AND TIE DETAILS THIS SHEET.
2. SPACE VERTICAL REINFORCING EQUALLY EACH FACE UNO.
3. ALTERNATE LOCATIONS OF 90° AND 135° TIE HOOKS.
4. SPLICES IN BARS OTHER THAN SHOWN SHALL BE APPROVED BY ENGINEER.
5. LAP SPLICE LENGTHS BASED ON BARS BELOW AT EACH LEVEL.
6. VERTICAL REINFORCING MAY BE CONTINUOUS FOR TWO LEVELS AT CONTRACTOR'S OPTION.
7. ALL VERTICAL REINFORCING SHALL TERMINATE IN FOUNDATION WITH STANDARD HOOK.
8. ALL COLUMN REINFORCING ASTM A615 GR. 60.
9. EMBEDDED PIPES OR CONDUITS IN COLUMNS SHALL NOT DISPLACE MORE THAN 4% OF COLUMN SECTION AREA.

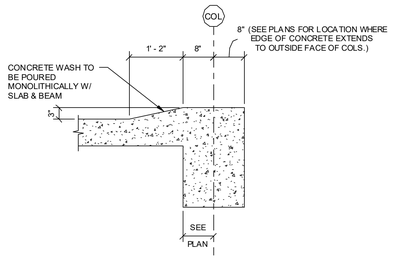


TYPE 1
8- BARS 12"x24" & 16"x30" COLS.

TYPE 2
8- BARS

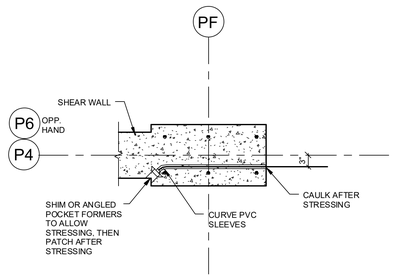
3/4" = 1'-0"

C2 Column Schedule & Tie Details



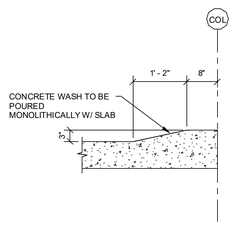
3/4" = 1'-0"

D3 Typical Concrete Wash Detail @ Beam Edge



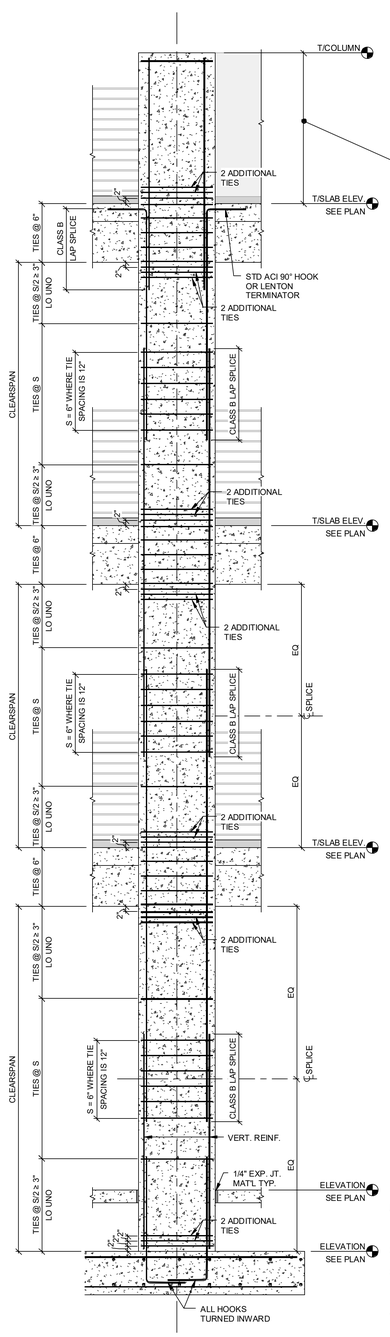
3/4" = 1'-0"

C3 Plan Detail @ Shear Wall/Column Barrier Cable Anchorage



3/4" = 1'-0"

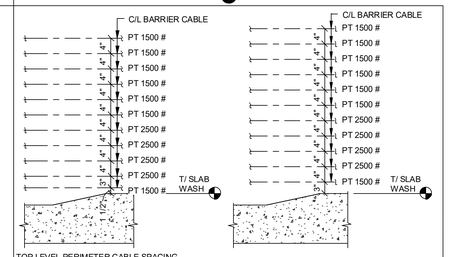
D4 Typical Concrete Wash Detail @ Slab Edge



NOTES:
1. LO = GREATER OF LARGEST COLUMN DIMENSION, 1/6 OF COLUMN CLEARSPAN OR 18".
2. S = TIE SPACING, SEE SCHEDULE.
3. LAP SPLICE (CLASS B) SEPARATE DOWELS WITH THE VERTICAL REINFORCING FOR EACH COLUMN. NON-CONTACT LAP SPLICES SHALL NOT EXCEED 1/5 REQUIRED LAP SPLICE NOR 6".
4. SEE TIE SPACING AT BARRIER CABLE ANCHORS THIS DRAWING.

1/2" = 1'-0"

A4 Typical Column Reinforcing Diagram View



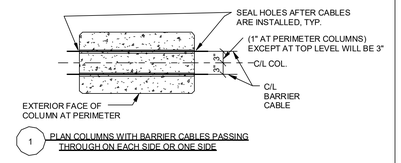
TOP LEVEL PERIMETER CABLE SPACING

BARRIER CABLE NOTES:
1. BARRIER CABLE TO BE 1/2" NOMINAL DIA., GR. 250, 7 - WIRE STRAND GALVANIZED, BARRIER STRAND, WITH HDPE EXTRUDED JACKET (BLACK). STRAND PER ASTM A416, WITH ULTIMATE TENSILE STRENGTH OF 36,000 #s AND NOMINAL AREA OF 0.144 in².
2. BARRIER CABLE TO BE PRESTRESSED TO FORCE OF 2500 #s OR 1500 #s.
3. SEAT BARRIER CABLE WEDGES IN WEDGE-ANCHOR SYSTEM TO A FORCE OF 80% OF THE ULTIMATE STRENGTH OF THE STRAND, FOR END COLUMN CONNECTIONS.
4. BARRIER CABLES TO BE INSTALLED BY QUALIFIED PERSONNEL W/ A MINIMUM OF 3 YRS. EXPERIENCE IN STRESSING CABLES.
5. BARRIER CABLE END COLUMN ANCHORAGE SHALL BE WEDGE-ANCHOR SYSTEM. SEE A4 THIS DRAWING FOR INFO.

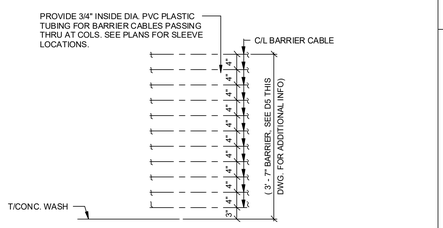
3/4" = 1'-0"

D5 Section @ Barrier / Guardrail

1. 4'-11" COLUMN EXTENSION ABOVE P4 W/ SAME REINF. AS SCHEDULED BELOW AT COLUMN LINES P1-PD, P1-PE, P1-PF, P1-PG, P9-PD, P9-PE, P9-PF, P9-PG, P9-PG.5, P3-PJ, P5-PJ & P7-PJ. NOTE: COLUMNS P1-PF & P9-PF INSIDE (8"x30") TO BE 4'-9 1/4" ABOVE P4 TO ALLOW BARRIER CABLE TO BYPASS.
2. 4'-11" COLUMN EXTENSION ABOVE P4 W/ SAME VERTICAL REINF. & TIE SET AS SCHEDULED BELOW W/ #4 TIES @ 9" AT COLUMN LINES P4-PJ & P6-PJ.
3. 4'-2" COLUMN EXTENSION ABOVE T/SLAB W/ SAME REINF. AS SCHEDULED BELOW AT COLUMN LINES P4-PD, P4-PE, P4-PF, P6-PD, P6-PE & P6-PF.
4. 4'-2" COLUMN EXTENSION ABOVE T/SLAB W/ SAME VERTICAL REINF. & TIE SET AS SCHEDULED BELOW W/ #4 TIES @ 4" AT COLUMN LINES P4-PC, P4-PG, P6-PC & P6-PG.
5. 8'-10 1/2" COLUMN EXTENSION ABOVE P4 W/ SAME VERTICAL REINF. & TIE SET AS SCHEDULED BELOW W/ #4 TIES @ 6" AT COLUMN LINES P1-PA, P1-PB, P1-PH, P1-PJ, P2-PA, P2-PB, P2-PH, P2-PJ, P8-PA, P8-PB, P8-PH, P8-PJ, P9-PA, P9-PB, P9-PH & P9-PJ.

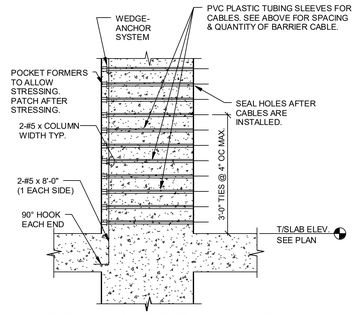


1 PLAN COLUMNS WITH BARRIER CABLES PASSING THROUGH ON EACH SIDE OR ONE SIDE



(TYP. PARKING LEVEL, RAMP)

NOTE:
1. TOP LEVEL PERIMETER BARRIER CABLE SPACING CHANGES, SEE D5 THIS DWG.



SECTION @ END COLS. AT BARRIER CABLE ANCHORAGES

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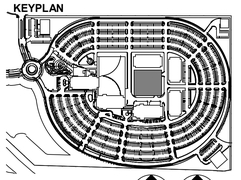

TRUE NORTH PLAN NORTH

0 4" 8" 1'-4"
SCALE: 1 1/2" = 1'-0"
UNLESS NOTED OTHERWISE

PROJECT NO: 0470.62

DATE: 00.00.2025

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Concrete Column Schedule & Details

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