



THE USER SHOWS AVERAGE OF 100% FOR THE FOLLOWING ITEMS: 1. GENERAL NOTES, 2. SPECIAL INSPECTIONS, 3. SHOP DRAWING REVIEW, 4. REINFORCING STEEL, 5. CONCRETE, 6. PLUMBING SLEEVES, 7. FORMWORK AND SHORING, 8. CONCRETE TESTING, 9. BUILDING MOVEMENTS, 10. DESIGN LOADS.

1000 GENERAL NOTES:

- 1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR OPENINGS, DEPRESSIONS, EQUIPMENT WEIGHTS AND LOCATIONS, EMBEDDED ITEMS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
2. DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
3. NO STRUCTURAL MEMBER OR COMPONENT SHALL BE CUT, NOTCHED, OR OTHERWISE ALTERED UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL COSTS INCURRED BY THE ENGINEER OF RECORD FOR REVIEW OF ANY SUCH DEVIATIONS.
4. DO NOT SCALE DRAWINGS.
5. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.
6. DETAILS ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE DRAWINGS CAN BE DETERMINED BY THE TITLE OF DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE REFERENCED AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DETERMINED BY THE ENGINEER OF RECORD.
7. THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCIES BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
8. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SEQUENCE AND SAFETY. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
9. THE STRUCTURAL ENGINEER'S OBLIGATIONS TO REVIEW SHOP DRAWINGS AND OTHER SUBMITTALS AND TO RETURN THEM IN A TIMELY MANNER ARE CONDITIONED UPON THE PRIOR REVIEW AND APPROVAL OF THE SHOP DRAWINGS OR SUBMITTALS BY A WRITERS AS REQUIRED IN THE CONSTRUCTION CONTRACT AND THE CONTRACTOR'S SUBMITTAL OF THE SHOP DRAWINGS AND OTHER SUBMITTALS IN ACCORDANCE WITH A WRITTEN SCHEDULE DISTRIBUTED IN ADVANCE TO THE ENGINEER IDENTIFYING THE DATES FOR THE SUBMITTAL OF THE VARIOUS SHOP DRAWINGS AND SUBMITTALS.
10. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF TLC ENGINEERING SOLUTIONS IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHALL NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK.
11. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXCEED LIFE SPAN AND TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE OWNER. THIS PROGRAM SHALL INCLUDE ITEMS SUCH AS, BUT NOT LIMITED TO, PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO SALT ENVIRONMENT OR OTHER HARSH CHEMICALS.
12. IN THE PROFESSIONAL OPINION OF TLC ENGINEERING SOLUTIONS, INC. THE STRUCTURAL CONTRACT DOCUMENTS FOR THIS PROJECT HAVE BEEN PREPARED IN ACCORDANCE WITH THE DESIGN CRITERIA AS SET FORTH IN THE 2021 INTERNATIONAL BUILDING CODE (IBC), WITH APPENDICES A AND C.
13. NO PROVISIONS HAVE BEEN MADE FOR VERTICAL OR HORIZONTAL EXPANSION EXCEPT AS SHOWN ON CONTRACT DOCUMENTS.
14. THE USE OF REPRODUCTIONS OF THESE CONTRACT DOCUMENTS AND USE OF CAD/REVIT FILES BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS IS PROHIBITED UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED FROM ENGINEER OF RECORD.

1010 BUILDING MOVEMENTS

THE BUILDING MOVEMENT SPECIFIED HEREIN IS ANTICIPATED TO OCCUR AND SHOULD BE CONSIDERED BY THE CONTRACTOR IN THE PERFORMANCE OF THE WORK.

- 1. THE FOLLOWING PROVISION FOR SUPERIMPOSED LOAD DEFLECTIONS SHALL BE MADE IN THE DESIGN, FABRICATION, AND INSTALLATION OF ALL PARTITIONS, AND OTHER ELEMENTS SUPPORTED BY AND ATTACHED TO THE STRUCTURE.
A. TYPICAL ROOF MEMBERS - SPAN/240 BUT NOT LESS THAN 3/8"
2. STORY DRIFT: LATERAL FRAME DEFLECTION OF H/300 IN THE PLANE OF THE WALL OF ONE FLOOR RELATIVE TO AN ADJACENT FLOOR SHALL BE TAKEN INTO ACCOUNT IN THE DESIGN, FABRICATION AND INSTALLATION OF THE BUILDING CLADDING.

1060 DESIGN LOADS:

- 1. THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2021 INTERNATIONAL BUILDING CODE WITH FULLY INTEGRATED CODE BASED ON THE 2021 INTERNATIONAL BUILDING CODE.
2. THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:
A. DEAD LOADS:
ROOF STRUCTURE (INC. MEP) 22 PSF
ALLOWANCE FOR FUTURE PV 5 PSF
CANOPY 12 PSF
B. LIVE LOADS:
ROOF CANOPY 20 PSF
20 PSF
C. WIND LOADS: PER ASCE 7-16.
SEE SHEET S-004 FOR COMPONENTS AND CLADDING PRESSURES
ULTIMATE DESIGN WIND SPEED, Vult 145 MPH (3 SEC. GUST)
NOMINAL DESIGN WIND SPEED, Vnom 113 MPH (3 SEC. GUST)
RISK CATEGORY II
EXPOSURE C
D. SEISMIC LOADS: PER ASCE 7-16.
RISK CATEGORY II
SEISMIC IMPORTANCE FACTOR, Ie 1.0
SPECTRAL RESPONSE ACCELERATION, SHORT DURATION, Ss 0.373
SPECTRAL RESPONSE ACCELERATION, 1.0 SECOND DURATION, S1 0.092
DESIGN SPECTRAL RESPONSE ACCELERATION, SHORT DURATION, Sds 0.373
DESIGN SPECTRAL RESPONSE ACCELERATION, 1.0 SECOND DURATION, Sd1 0.141
SITE CLASS D
SEISMIC DESIGN CATEGORY, SDC B
SEISMIC FORCE-RESISTING SYSTEM(S) INTERMEDIATE PRECAST CONCRETE SHEAR WALLS (E/W) STEEL ORDINARY MOMENT FRAMES (O) 1.25W (KIPS (E/W) 0.081W (KIPS (NS) 0.123 (E/W) 0.081 (NS) 4 (E/W) 3.5 (NS) EQUIVALENT LATERAL FORCE PROCEDURE
E. SNOW LOADS: PER ASCE 7-16.
GROUND SNOW LOAD, Pg 10 PSF
SNOW IMPORTANCE FACTOR 1.0
EXPOSURE FACTOR (Ce) 1.0
THERMAL FACTOR (Ct) 1.0
WAREHOUSE BUILDING
ROOF SNOW LOAD 6.8 PSF
CANOPY ROOF SNOW LOAD 6.5 PSF

1070 SPECIAL INSPECTIONS

- 1. SPECIAL INSPECTIONS SHALL BE PERFORMED BY ONE OR MORE AGENCIES APPROVED BY THE EOR AND BUILDING OFFICIAL.
2. IN ACCORDANCE WITH CHAPTER 17 OF THE 2021 INTERNATIONAL BUILDING CODE, SPECIAL INSPECTIONS ARE REQUIRED FOR THE FOLLOWING MATERIALS, SYSTEMS, COMPONENTS, AND/OR WORK:
3. FREQUENCY AND SCOPE OF SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS AND CHAPTER 17 OF THE 2021 INTERNATIONAL BUILDING CODE.
4. SPECIAL INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, EOR, AND OWNER FOR RECORD.
A. STRUCTURAL STEEL
B. WELDING/BOLTING
C. COLD-FORMED STEEL
D. CAST-IN-PLACE CONCRETE
E. REINFORCING STEEL
F. POST INSTALLED ANCHORS
G. FOUNDATIONS - SOIL
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING AND SCHEDULING ALL SPECIAL INSPECTIONS.
6. THE CONTRACTOR SHALL BEAR THE COST OF CORRECTING OR REPLACING ALL MATERIALS, SYSTEMS, COMPONENTS, AND/OR WORK THAT DO NOT MEET THE REQUIREMENTS OF THE SPECIAL INSPECTOR.

1330 SHOP DRAWING REVIEW:

- 1. SHOP DRAWINGS SHALL ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN ON THE CONTRACT DOCUMENTS. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC. REVIEW OF SUBMITTALS AND SHOP DRAWINGS DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS.
2. SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR AND MARKED "APPROVED" PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. NON-CONFORMING DRAWING SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
3. SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED ELECTRONICALLY.
4. THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER OF RECORD.
5. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT/ENGINEER OF RECORD REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL. CONTRACTOR IS RESPONSIBLE FOR COSTS CAUSED BY MULTIPLE RE-SUBMITTALS (MORE THAN ONE) AT ARCHITECT/ENGINEERS' CURRENT HOURLY RATES.

1331 SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS:

- 1. THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER:
A. COLD FORMED STEEL EXTERIOR OR LOAD BEARING WALL SYSTEMS
B. OPEN WEB STEEL JOISTS
C. STRUCTURAL STEEL CONNECTIONS REQUIRING ENGINEERING
D. TILT-WALL ERECTION DRAWINGS
E. TEMPORARY WALL BRACING
F. PREFABRICATED STEEL STAIRS & RAILINGS
G. PREFABRICATED LADDER AND LADDER FRAMING SUPPORT
2. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES. LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND DRAWINGS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. SHOP DRAWINGS AND CALCULATIONS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.
3. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER.
4. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA (IN WHICH THE PROJECT RESIDES). COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA (IN WHICH THE PROJECT RESIDES) AS AN INDICATION THAT HE/SHE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED SET FOR THEIR RECORDS.
5. DRAWINGS PREPARED SOLELY TO SERVE AS A GUIDE FOR FABRICATION AND INSTALLATION (SUCH AS REINFORCING STEEL SHOP DRAWINGS OR STRUCTURAL STEEL ERECTION DRAWINGS) AND REQUIRING NO ENGINEERING, DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.
6. CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.
7. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:
A. THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
B. THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.
C. THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE.
D. THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE.
8. SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED AND WILL BE RETURNED.

1333 SUBMITTALS:

- 1. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED APPROVED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.
2. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:
ITEMS MARKED (D) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA
ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEER'S RECORD ONLY.
A. STRUCTURAL STEEL
B. REINFORCING STEEL
C. COLD-FORMED STEEL FRAMING (D)
D. CONCRETE MIX DESIGNS
E. TILT-UP PANELS
F. TEMPORARY WALL BRACING (D)
G. OPEN WEB STEEL JOISTS (D)
H. POST-INSTALLED ADHESIVE ANCHORS (#)
I. POST-INSTALLED MECHANICAL ANCHORS (#)
J. PREFABRICATED STEEL STAIRS & RAILINGS (D)
K. PREFABRICATED LADDER AND LADDER FRAMING SUPPORT (D)
L. FABRIC AWNINGS (D)
3. MANUFACTURER'S LITERATURE. SUBMIT TWO COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

1334 REQUEST FOR INTERPRETATION:

- 1. RFI SHALL ORIGINATE WITH CONTRACTOR AND SHALL BE SUBMITTED IN THE FORM SPECIFIED WITHIN CONTRACT DOCUMENTS. RFI SHALL BE SUBMITTED IN A PROMPT MANNER AS TO AVOID DELAYS IN CONTRACTORS WORK.
2. RFI SHALL BE SUBMITTED AS SPECIFIED WITHIN THE CONTRACT DOCUMENTS AND SHALL BE FORWARDED TO THE ENGINEER VIA THE ARCHITECT OR DIRECTLY TO THE ENGINEER BY THE CONTRACTOR WHEN APPROVED BY THE ARCHITECT.
3. ENGINEER SHALL TAKE UP TO 5 BUSINESS DAYS TO REVIEW AND RETURN RFIS. HOWEVER, THE ENGINEER WILL ATTEMPT TO EXPEDITE THE REVIEW OF ALL RFIS WITHIN A REASONABLE TIME FRAME.
4. RFI RESPONSES ARE NOT INTENDED TO AUTHORIZE ANY INCREASE IN CONSTRUCTION COST, SCHEDULE OR TIME EXTENSIONS, OR CONSTRUCTION IN CONFLICT WITH ANY APPLICABLE CODES OR SPECIFIED DESIGN STANDARDS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE DESIGN TEAM IMMEDIATELY OF ANY PERCEIVED SCOPE, SCHEDULE, OR COST IMPACTS OR ADJUSTMENTS. IF CONTRACTOR REQUESTS ANY ADDITIONAL COST, INCREASE IN SCHEDULE OR ADJUSTMENT IN SCOPE, THE CONTRACTOR SHALL NOT PROCEED WITH ADDITIONAL WORK UNTIL APPROVED IN WRITING BY THE CONSTRUCTION ADMINISTRATOR.

2310 FOUNDATIONS:

- 1. SEE THE FOLLOWING GEOTECHNICAL REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH:
REPORT No. 22-36084
PREPARED BY: ECS SOUTHEAST, LLC
GEOTECHNICAL ENGINEERING REPORT
DATED: JUNE 9, 2025
2. FOLLOW THE RECOMMENDATIONS LISTED IN THE GEOTECHNICAL REPORT FOR SITE PREPARATION WORK. AT A MINIMUM, SITE PREPARATION WORK SHALL INCLUDE:
A. STRIPPING AND GRUBBING OF THE BUILDING FOOTPRINT PLUS A MARGIN OF 5 FEET AROUND THE BUILDING, REMOVING ALL ORGANIC MATERIALS WITHIN 5 FEET OF THE SURFACE.
B. PROOF ROLLING THE BUILDING SITE TO LOCATE ANY UNFORESEEN SOFT AREAS. ANY SOFT AREAS SHALL BE EXCAVATED AND REPLACED WITH CLEAN FILL. A DENSITY OF AT LEAST 95% FOR A DEPTH OF 2 FEET IS REQUIRED UNDER THE BUILDING FOOTPRINT.
C. ALL FILL SHALL BE CLEAN SAND AND FREE OF ORGANIC MATERIALS. COMPACT FILL IN 12 INCH (UNCOMPACTED) THICKNESS LIFTS TO A MINIMUM OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY VALUE.
D. EXCAVATIONS FOR FOUNDATIONS SHALL BE COMPACTED TO 95% FOR A DEPTH OF AT LEAST 2 FEET BELOW THE BOTTOM OF THE FOUNDATION AND SLABS.
E. DEWATERING MAY BE REQUIRED TO ACHIEVE THE REQUIRED COMPACTION VALUES, AND IF USED, SHOULD DRAIN DOWN THE WATER LEVEL TO AT LEAST 2 FEET BELOW THE BOTTOM OF THE EXCAVATION.
3. SLABS ON GRADE SHALL BE PLACED OVER A 15 MIL CLASS "A" VAPOR RETARDER. VAPOR RETARDER SHALL BE LAPPED A MINIMUM OF 6" OR AS RECOMMENDED BY THE MANUFACTURER. JOISTS ARE GREATER AND TAPED AT ALL JOINTS. ALL PUNCTURES IN THE VAPOR RETARDER SHALL BE REPAIRED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. ALL PENETRATIONS THROUGH THE VAPOR RETARDER (COLUMNS, PLUMBING, CONDUITS, ETC) SHALL BE SEALED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. VAPOR RETARDER SHALL BE CONTINUOUS UNDER WALL FOUNDATIONS OR SEALED TO EXTERIOR WALLS PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
4. FOUNDATIONS FOR THIS BUILDING SHALL BE FOUNDED ON GROUND IMPROVEMENT TECHNIQUE USING EARTHQUAKE DRAINS AND UTILIZE ON AN ALLOWABLE BEARING PRESSURE OF 3,000 PSF. THE EARTHQUAKE DRAINS SHALL BE INSTALLED BY A SPECIALTY CONTRACTOR EXPERIENCED IN THIS TYPE WORK. SUBMIT SIGNED AND SEALED DELEGATED ENGINEERING CALCULATIONS AND PLANS FOR REVIEW. SEE GEOTECHNICAL REPORT FOR ADDITIONAL REQUIREMENTS AND SOIL PROPERTIES.
5. CANTILEVERED RETAINING WALL AND RESTRAINED FOUNDATION WALL DESIGN IS BASED ON THE FOLLOWING SOIL PROPERTIES FOR CONTROLLED FILL:
ACTIVE SOIL PRESSURE COEFFICIENT (Ka) 0.31
AT-REST SOIL PRESSURE COEFFICIENT (K0) 0.47
PASSIVE SOIL PRESSURE COEFFICIENT (Kp) 3.25
COEFFICIENT OF FRICTION 0.30

3302 CONCRETE:

- 1. SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX.
CONCRETE STRUCTURE TYPE COMPRESSIVE STRENGTH SLUMP COARSE AGGREGATE MAXIMUM W/C RATIO BASELINE GWP KPa CO2e/CU YD
FOUNDATIONS 4000 PSI 4'-6" ASTM #57 0.50 205
TILT WALL PANELS\*\* 4000 PSI 4'-6" ASTM #57 0.48 205
SLAB-ON-GRADE 4000 PSI 4'-6" ASTM #57 0.50 205
\*\* SEE TILT WALL PANEL ELEVATIONS FOR OTHER CONCRETE STRENGTHS REQUIRED.
2. ALL PLUMPED CONCRETE SHALL CONTAIN A HIGH RANGE WATER REDUCING AGENT TO INCREASE SLUMP BEYOND THAT NOTED ABOVE.
3. CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ACI STANDARDS AND SPECIFICATIONS.
4. SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE.
5. CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM STANDARD C84 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.
6. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE.
7. SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-CLASS D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. SCOFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY.
8. CALCIUM CHLORIDES SHALL NOT BE UTILIZED. OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.
9. CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.
10. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318.6.3.
11. CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 5 OF ACI 318.
12. WHEN ADHESIVES OR WATER VAPOR IMPERMEABLE FLOOR COVERINGS ARE BEING USED ON CONCRETE SURFACES, THE CONTRACTOR SHALL VERIFY THROUGH APPROPRIATE TESTING THAT THE WATER CONTENT OF THE CONCRETE, AND VAPOR TRANSMISSION RATE THROUGH THE CONCRETE, IS WITHIN THE ALLOWABLE RANGE BEFORE INSTALLATION. THE MANUFACTURER OF THE ADHESIVE OR FLOOR COVERING SHALL REVIEW AND APPROVE THE RESULTS OF THE TESTING PRIOR TO INSTALLATION OF PRODUCTS.

3310 REINFORCING STEEL:

- 1. SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SOURCE REINFORCING STEEL FROM THE LOWEST CARBON LOCAL SUPPLIER WITH ACHIEVABLE GWP GOAL OF 0.614 Kg/0.26Kg. PREFERENCE FOR EAF PRODUCTION AND RENEWABLE ENERGY SOURCING.
2. PROVIDE CONCRETE COVER OVER PRIMARY REINFORCEMENT, TIES, AND STIRRUPS, AS FOLLOWS, UNLESS OTHERWISE NOTED:
LOCATION AND CONDITION MINIMUM COVER
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. ALL BARS 3"
B. CONCRETE EXPOSED TO EARTH OR WEATHER #6 OR GREATER 2" #5 OR SMALLER 1.5"
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND #11 OR SMALLER 3/4" ALL BARS 1.5"
3. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.
4. PROVIDE STANDARD HOOKS AT DISCONTINUOUS ENDS OF ALL TOP BARS.
5. WHERE REINFORCING IS SHOWN CONTINUOUS, SPLICE BOTTOM BARS OVER SUPPORTS AND TOP BARS AT CENTER OF SPAN. ALL OTHER LAP SPLICES SHALL BE IN ACCORDANCE WITH SPLICE TABLES AND DETAILS SHOWN ON DRAWINGS.
6. PROVIDE DOWELS INTO FOOTINGS, PILE CAPS, SUPPORT BEAMS, ETC. TO MATCH VERTICAL BARS WITH CLASS B TENSION LAP SPLICES, U.N.O.
7. AT CHANGES IN DIRECTION OF CONCRETE WALLS AND TIE BEAMS, PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL STEEL.
8. WHERE HOOKS ARE SHOWN ON THE PLANS OR DETAILS, HOOKS SHALL BE DETAILED TO EXTEND DEEP ENOUGH INTO SUPPORTING STRUCTURE TO DEVELOP THE FULL STRENGTH OF THE HOOKED BAR. PROVIDE ADDITIONAL TIES OR STIRRUPS IN SUPPORTING STRUCTURE AS REQUIRED TO SATISFY ACI 318 HOOK DEVELOPMENT, CONFINEMENT, AND ANCHORAGE CRITERIA.
9. AT CANTILEVER SLABS AND BEAMS, REINFORCING BARS IN DIRECTION OF CANTILEVER SHALL BE DETAILED TO FULLY DEVELOP THE BAR STRENGTH INTO THE SUPPORTING STRUCTURE, EITHER BY PROVIDING FULL CLASS B LAP SPLICE OR STANDARD ACI HOOKS EMBEDDED DEEP ENOUGH BEYOND SUPPORT TO DEVELOP STRENGTH OF BAR.

3321 FORMWORK AND SHORING:

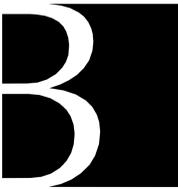
- 1. NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28-DAY DESIGN STRENGTH.
2. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

3323 PLUMBING SLEEVES:

- 1. MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER.
2. PRIOR TO CONSTRUCTION ALL SLEEVE LOCATIONS AND SIZES NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE ENGINEER.
3. PLACE TWO #3 STIRRUPS @ 3" O.C. EACH SIDE OF SLEEVE PENETRATIONS IN BEAMS.
3400 CONCRETE TESTING:
1. AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:
A. A. ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE."
B. B. ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY, REQUIRED LAB CURED CYLINDER QUANTITIES AND TEST AGE AS FOLLOWS:
(2) AT 7 DAYS
(3) AT 28 DAYS
ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28-DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

3470 TILT-UP WALL PANELS:

- 1. TILT-UP WALL PANELS SHALL BE OF THE CONFIGURATION SHOWN ON DRAWINGS AND SHALL HAVE AT A MINIMUM THE REINFORCEMENT SHOWN ON DRAWINGS.
2. CONTRACTOR SHALL COORDINATE THE LOCATIONS OF JOIST SEATS, BEAM EMBEDS, BRIDGING CONNECTIONS, DECK ANGLE CONNECTIONS, ETC. WITH THE APPROVED STEEL FRAMING SHOP DRAWINGS AND WITH THE STRUCTURAL DRAWINGS.
3. THE CENTERLINE OF THE VERTICAL BARS SHALL COINCIDE WITH THE CENTERLINE OF THE STRUCTURAL THICKNESS OF THE PANEL FOR SINGLE-LAYER MATS. PANELS WITH TWO LAYERS OF MAT STEEL SHALL HAVE A MAT 1-1/2" CLEAR ON EACH FACE, UNLESS NOTED OTHERWISE.
4. EXPOSED EDGES OF THE PANELS SHALL BE CHAMFERED, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR PANEL FINISHES, REVEALS, CHAMFERS, ETC.
5. TILT-UP WALL PANELS HAVE BEEN DESIGNED TO WITHSTAND GRAVITY LOADS AND WIND LOADS AS INDICATED ON THE DRAWINGS IN THE FINAL COMPLETED STRUCTURE CONFIGURATION. HOWEVER, NO ALLOWANCE HAS BEEN MADE FOR HANDLING AND LIFTING OR TEMPORARILY BRACED STRESSES OR CONDITIONS.
6. SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW SHOWING PANEL LAYOUT WITH REINFORCEMENT, EMBEDS, OPENINGS, DIMENSIONS, JOINTS, ETC. AS REQUIRED TO PROPERLY CONSTRUCT THE WALL PANELS.
7. SHOP DRAWINGS AND CALCULATIONS FOR LIFTING AND BRACING SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA.
8. REFER TO THE ARCHITECTURAL DRAWINGS FOR FINISH REQUIREMENTS, CHAMFERS, REVEALS, ETC.
9. THE CONTRACTOR SHALL PROVIDE DESIGN FOR THE LIFT INSERTS AND ANY ADDITIONAL REINFORCING STEEL REQUIRED FOR THE LIFTING OPERATIONS OR TEMPORARY BRACING CONDITIONS. HOWEVER, NO ADDITIONAL REINFORCING SHALL BE ADDED WITHOUT THE EXPRESSED APPROVAL OF THE ENGINEER OF RECORD. THE DESIGNERS OF THE LIFTING INSERTS AND TEMPORARY BRACING MUST CONSIDER THE REINFORCING ALREADY PRESENT IN THE PANELS AS INDICATED IN THIS SET OF CONSTRUCTION DRAWINGS.
10. THE CONTRACTOR SHALL CHECK ALL PANEL DIMENSIONS, PLATE LOCATIONS AND DETERMINE THE LOCATIONS OF ALL OPENINGS REQUIRED. NO PANEL WORK SHALL BE PERFORMED WITHOUT CONTRACTORS APPROVAL OF ALL OF THE ABOVE. THE CONTRACTOR IS INDICATING THAT ALL THE ABOVE HAS BEEN REVIEWED AND THE PANEL DRAWINGS ARE APPROVED FOR ACCURACY BY THE COMMENCEMENT OF PANEL CONSTRUCTION EVEN IF FORMAL STAMPED APPROVAL HAS NOT BEEN INDICATED ON THOSE DRAWINGS. SUBMIT SHOP DRAWINGS SHOWING PANEL DIMENSIONS AND OPENING LOCATIONS.
11. MISCELLANEOUS OPENINGS MAY BE REQUIRED FOR FIRE LINES, PLUMBING, SANITARY LINES, ELECTRICAL CONDUITS, ETC. CORE DRILLING AFTER ERECTION OF PANELS MUST HAVE THE APPROVAL OF THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO PERFORMANCE OF THE WORK.
12. THE REINFORCING STEEL SUPPLIER SHALL PROVIDE SHOP DRAWINGS INDICATING ALL NECESSARY INFORMATION REQUIRED TO ACCURATELY POSITION THE REBAR AS INDICATED. ENSURE CHAIRS, BOLSTERS OR OTHER MEANS OF SUPPORTING REBARS ARE PROVIDED AND ACCURATELY DETAILED. SEE PLANS FOR MINIMUM SPLICE LENGTH FOR REINFORCING BARS.
13. THE SLAB SHALL BE PRETREATED WITH A RELEASING AGENT PRIOR TO PLACEMENT OF CONCRETE OR REINFORCEMENT FOR THE TILT UP. MANUFACTURER'S REQUIREMENTS SHALL BE UTILIZED IN PLACING OF THE RELEASING AGENT AND COMPATIBILITY WITH ANY FUTURE COATINGS SHALL BE VERIFIED.
14. TEMPORARY BRACING OF TILT UP CONCRETE PANELS DURING CONSTRUCTION:
14.1. TILT WALL PANEL DESIGN AS DEPICTED ON DESIGN DRAWINGS IS BASED ON THE FINAL CONSTRUCTION CONFIGURATION INCLUDING INSTALLATION OF PERMANENT BRACING INCLUDING, BUT NOT LIMITED TO FLOOR, ROOF FRAMING, METAL DECK AND SLAB SYSTEMS. THE DESIGN DOES NOT ADDRESS MEANS AND METHODS OF CONSTRUCTION OR TEMPORARY CONSTRUCTION CONDITIONS.
14.2. IT IS THE FULL RESPONSIBILITY OF THE CONTRACTOR TO ERECT TILT WALL PANELS IN A SAFE MANNER, AND TO ADEQUATELY BRACE, OR OTHERWISE PROTECT THE PANELS AGAINST WIND OR OTHER FORCES/EFFECTS DURING CONSTRUCTION AND UNTIL THE FINAL PERMANENT STRUCTURAL SYSTEM IS COMPLETED.
14.3. CONTRACTOR TO SUBMIT COMPLETE PROPOSED BRACING PLANS AND CALCULATIONS, PREPARED SPECIFICALLY FOR THIS PROJECT BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA OUTLINING PROPOSED TEMPORARY BRACING SYSTEM, SEQUENCE OF CONSTRUCTION OF BRACING WITH CONSIDERATION OF CONFLICT WITH OTHER BUILDING SYSTEMS AND COMPONENTS AND EVALUATION OF PANEL OR FLOOR SLAB TO ACCOMMODATE TEMPORARY LOADS AND UNBRACED CONDITIONS AS WELL AS BRACING FORCE RESOLUTION.
14.4. ANY DESIGN OR MODIFIED CONSTRUCTION REQUIRED TO ACCOMMODATE PROPOSED BRACING SCHEME IS THE FULL RESPONSIBILITY OF THE CONTRACTOR, INCLUDING ANY ADDITIONAL REINFORCING OF TILT WALL PANELS OR INSTALLATION OF DEADEN OR ADDITIONAL SLAB REINFORCEMENT. GENERAL BRACING SCHEMES DEVELOPED BY BRACING MANUFACTURERS ARE NOT SUFFICIENT OR ACCEPTABLE.
14.5. TEMPORARY BRACING LOADING TO BE IN COMPLIANCE WITH DESIGN DOCUMENTS AND THE LATEST EDITION OF ASCE 7 AND TILT-UP CONCRETE ASSOCIATION GUIDELINES. ACTUAL WIND PRESSURE IS TO BE CALCULATED, BUT IN NO CASE SHOULD BE BASED ON LESS THAN 70 MPH (3 SECOND GUST) WIND SPEED. CONSTRUCTION FIELD OFFICE TO MONITOR NATIONAL WEATHER SERVICE FORECASTS AND BE PREPARED TO SHUT DOWN AND CLEAR PROJECT SITE IF DESIGN WIND SPEED IS ANTICIPATED TO BE EXCEEDED.
14.6. CONTRACTOR'S SPECIALTY BRACING ENGINEER OR DESIGNATED REPRESENTATIVE (MINIMUM 5 YEARS EXPERIENCE WITH TILT WALL BRACING SYSTEMS) SHALL VISIT SITE DURING PANEL INSTALLATION TO CONFIRM THAT BRACING IS IN COMPLIANCE WITH DESIGN INTENTIONS, AND SUBMIT REPORT OF FINDINGS TO CONTRACTOR AND STRUCTURAL ENGINEER OF RECORD.
15. ALL MECHANICAL OPENINGS REQUIRED IN TILT PANELS SHALL BE LOCATED SUCH THAT NO OPENINGS ARE CLOSER THAN 2'-0" TO PANEL EDGE OR ADJACENT OPENINGS. ALL MECHANICAL OPENINGS SHALL BE INCLUDED IN TILT PANEL SHOP DRAWINGS AND SUBMITTED FOR APPROVAL.
16. SHIM PLATES FOR TILT PANELS SHALL BE SPACED AT PANEL QUARTER POINTS AND AT EACH SIDE OF DOOR OR WINDOW OPENINGS GREATER THAN 8'-0" IN WIDTH, WITH NO LESS THAN TWO SHIM PLATES PER PANEL LEG.
17. DO NOT ATTACH ANY STRUCTURAL MEMBERS TO TILT PANELS UNTIL PANELS HAVE BEEN FULLY GROUTED.
18. SLAB ON GRADE HAS BEEN DESIGNED TO MEET THE MINIMUM REQUIRED LOADS AS STATED HEREIN; HOWEVER, THE SLAB HAS NOT BEEN DESIGNED TO SUPPORT THE WEIGHT OF THE LIFTING CRANE OR OTHER HEAVY EQUIPMENT.



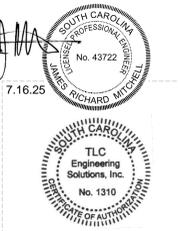
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Table with 3 columns: DATE, SUBMISSION, PERMIT SET. Row 1: 07/16/2025, PERMIT SET.

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STRUCTURAL GENERAL NOTES
SHEET NUMBER: S002























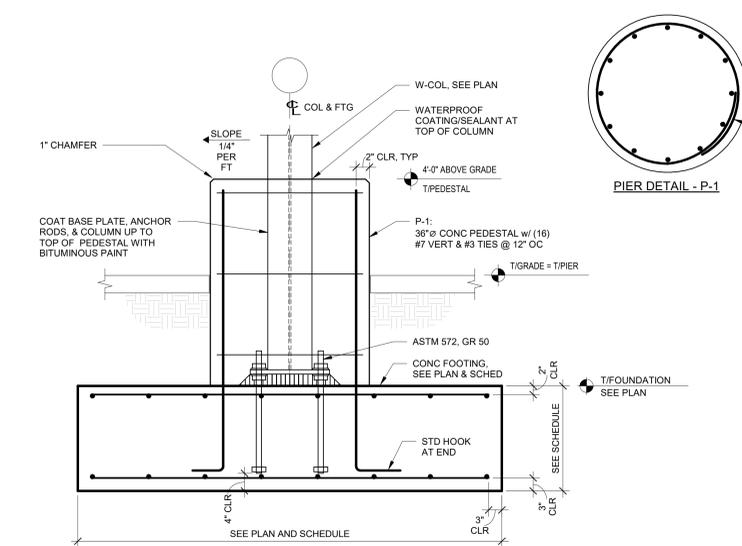






THE LINE SHOWN ABOVE IS SUBJECT TO CHANGE WITHOUT NOTICE.

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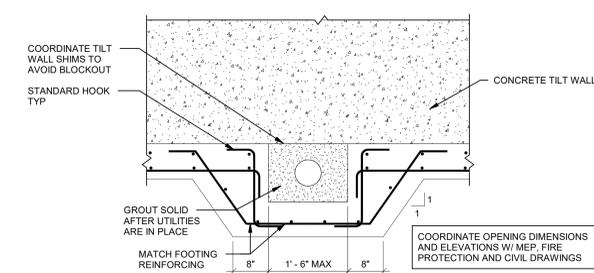


**E3 LIGHT POLE FOUNDATION DETAIL**  
3/4" = 1'-0"

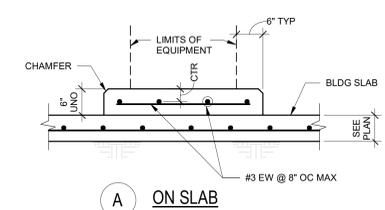
COLUMN	BASE PLATE (TLXW)	ANCHORS	HOLE DIAMETER	MIN WASHER DIAMETER	ANCHOR ROD EDGE DISTANCE
HSS 10x10	PL-1'x20'x20'	(4) 1"Ø	1 13/16"	3/8" x 3"	2"

COLUMN	BASE PLATE	ANCHORS	HOLE DIAMETER	MIN WASHER DIAMETER	ANCHOR ROD EDGE DISTANCE
W24x250	PL 2'x36"x32"	(12) 1 3/4"Ø	2 3/4"	5/8"x4"	3"

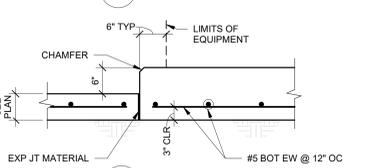
**C1 CANOPY COLUMN BASE**  
3/4" = 1'-0"



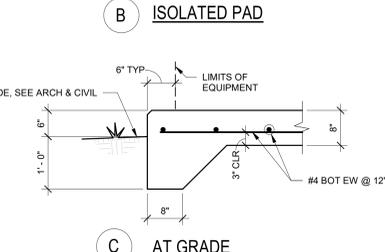
**B1 TYP SLAB CONTROL JT DETAIL**  
3/4" = 1'-0"



**A ON SLAB**



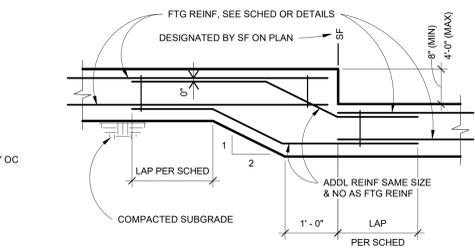
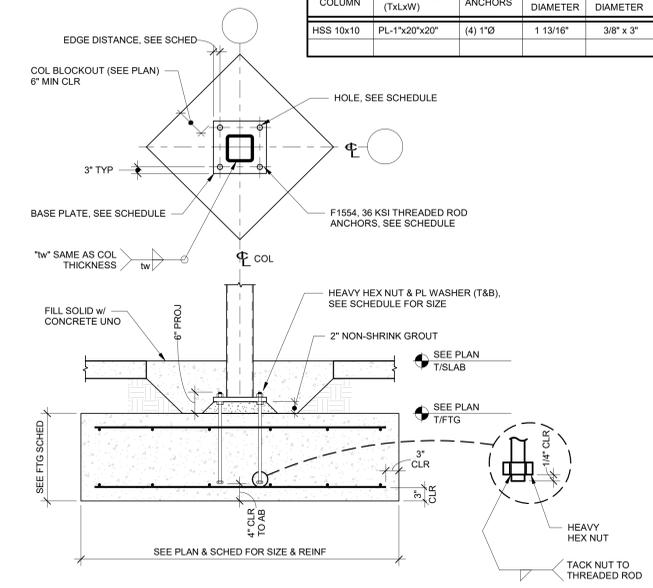
**B ISOLATED PAD**



**C AT GRADE**

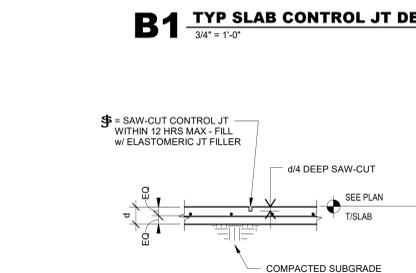
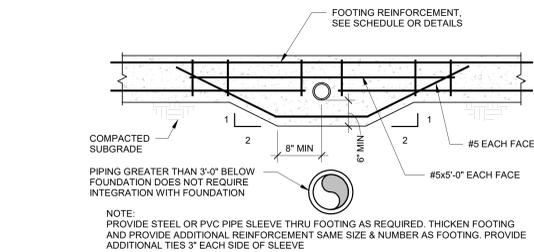
**A2 TYP EQUIPMENT SUPPORT PADS**  
3/4" = 1'-0"

**C3 HSS COLUMN/FTG DETAIL**  
3/4" = 1'-0"

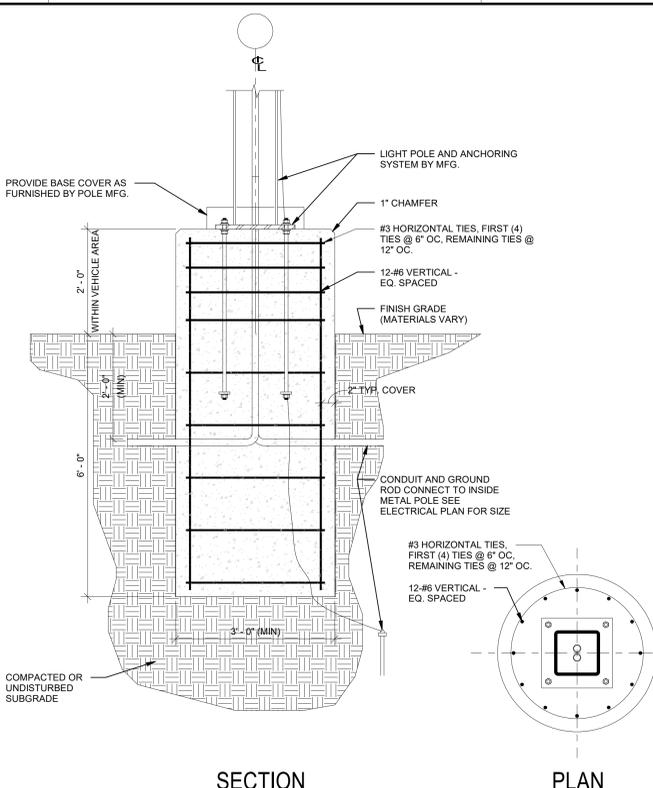


**A3 TYP STEPPED FTG DETAIL**  
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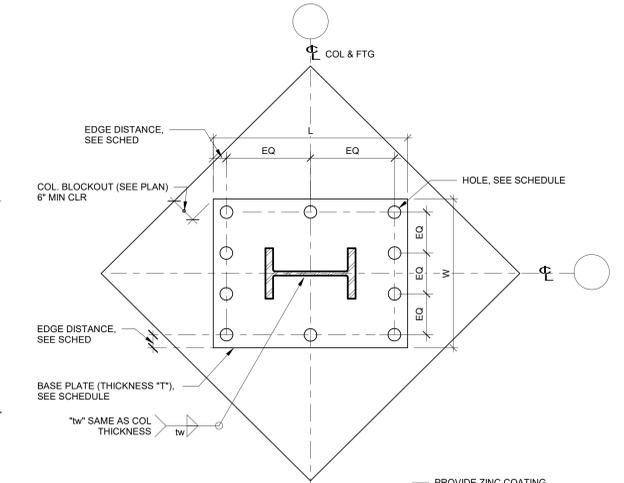
**A4 SLEEVED FTG DETAIL**  
3/4" = 1'-0"



**A1 TYP SLAB CONTROL JT DETAIL**  
3/4" = 1'-0"

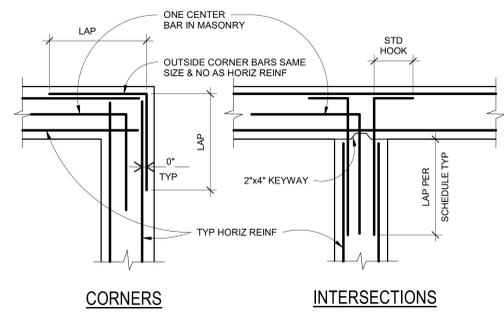


**E6 TYP CONSTRUCTION JOINT DETAIL**  
3/4" = 1'-0"

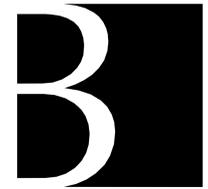


COLUMN	BASE PLATE (TLXW)	ANCHORS	HOLE DIAMETER	MIN WASHER DIAMETER	ANCHOR ROD EDGE DISTANCE	STEEL PROPERTIES
W18	PL-2'x36"x34"	(12) 1 3/4"Ø	2 3/4"	5/8"x4"	3"	ASTM A572, GRADE 50 (FY=50 KSI)

**C5 W-COLUMN/FTG DETAIL**  
3/4" = 1'-0"



**A5 TYP HORIZ REINF IN WALLS & FTGS**  
3/4" = 1'-0"



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**SOUTH CAROLINA REGISTERED PROFESSIONAL ENGINEER**  
No. 43722  
**RICHARD MITCHELL**

**SOUTH CAROLINA REGISTERED PROFESSIONAL ENGINEER**  
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No. 1310

**PROJECT MAGNET**  
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**SECTIONS AND DETAILS**  
SHEET NUMBER:  
**S401**













