



FOUNDATION PLAN 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

- 1. SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL
- STRUCTURAL NOTES & SCHEDULES.

 TOP OF SLAB REFERENCE ELEVATION = 0'-0" UNLESS

 OTHERWISE NOTED. SEE ARCHITECTURAL AND CIVIL
- DRAWINGS FOR ACTUAL SITE ELEVATIONS.

 3. CONCRETE FLOOR SLAB IS 6" THICK WITH 6 x 6 W 2.9x W 2.9 WELDED WIRE FABRIC, PROVIDE 10 MIL VAPOR BARRIER AND 6" COMPACTED GRANULAR BASE UNDER SLAB. SEE DETAIL 5/S2.01
- CONCRETE FLOOR SLAB IS 4" THICK WITH 6 x 6 W 2.1x W 2.1 WELDED WIRE FABRIC, PROVIDE 10 MIL VAPOR BARRIER AND 4" COMPACTED GRANULAR BASE UNDER SLAB. SEE DETAIL 1/S2.01
- SEAB. SEE DETAIL 1/S2.01
 SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS.
- 6. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MASONRY OPENINGS NOT SHOWN.
- FOR FOOTING, BASE PLATE AND ANCHOR BOLT DETAIL, SEE SCHEDULE ON SHEET S1.01. AND DETAIL ON S2.01, S2.02
 SEE 4/S2.01 FOR SLAB CORNER REINFORCEMENT. U.N.O.

FOUNDATION PLAN LEGEND		
F4.0	DENOTES COLUMN CONCRETE SPREAD FOOTING WITH FOOTING MARK — SEE FOOTING SCHEDULE ON S1.01 FOR SIZE AND REINFORCING	
C.J.	DENOTES SLAB ON GRADE CONSTRUCTION OR SAWCUT CONTROL JOINT — SEE DETAILS 3/S2.01 AND 2/S2.01 FOR ADDITIONAL INFORMATION	
U.O.N.	DENOTES 'UNLESS OTHERWISE NOTED'	
РЕМВ	DENOTES 'PRE-ENGINEERED METAL BUILDING'	

	FOOTING SCHEDULE					
MARK	FTG. SIZE	REINFORCEMENT	REMARKS			
F3.0	3'-0" x 3'-0" x 2'-0"	4 — #4 EACH WAY, TOP. 4 — #6 EACH WAY, BOTT.	-			
F4.0	4'-0" x 4'-0" x 2'-0"	5 — #4 EACH WAY, TOP. 5 — #6 EACH WAY, BOTT.	_			
F5.0	5'-0" x 5'-0" x 2'-0"	6 — #4 EACH WAY, TOP. 6 — #6 EACH WAY, BOTT.	_			
F8.6	8'-0" x 6'-0" x 2'-0"	5 — #4 EACH L.W TOP. 9 — #4 EACH S.W, TOP. 5 — #6 EACH L.W, BOTT. 9 — #6 EACH S.W, BOTT.	-			

1. S.W.=> SHORT WAY. 2. L.W.=> LONG WAY. 3. E.W.=> EACH WAY

COLUMN SCHEDULE					
MARK COL. SIZE BASE & SIZE & TYPE BOLT DIA. REMAI				REMARKS	
C1	HSS 4 x 4 x 1/4	¾" x 10" x 0'-10" - B1	3⁄4"	4 BOLTS	
C2	HSS 4 x 4 x 1/4	¾" x 5" x 0'−10" − B2	3⁄4"	2 BOLTS	
C3	HSS 4 x 4 x 1/4	¾" x 7½" x 7½" − B3	3/4"	2 BOLTS	

1. SEE DETAIL 7/S2.02 FOR ANCHOR BOLTS DETAILS & BASE PLATE TYPE.

SERVICE AND AS SUCH SHALL REMAIN THE PROPERTY OF THE ARCHITECT. THEY HAVE BEEN PREPARED FOR A SPECIFIC PROJECT AND SHALL NOT BE USED IN CONJUNCTION WITH ANY OTHER PROJECTS WITHOUT PRIOR WRITTEN PERMISSION OF THE ARCHITECT.

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DATE

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CN=Mark S. Roy
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DN: C=US,

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DRAWN BY: RPA/GBP RPA/MSR
PROJECT #: 24008
ISSUE DATE: 04.30.2025

PHASE:
CONSTRUCTION DOCUMENTS

SHEET NAME & NUMBER

FOUNDATION PLAN

\$1.01

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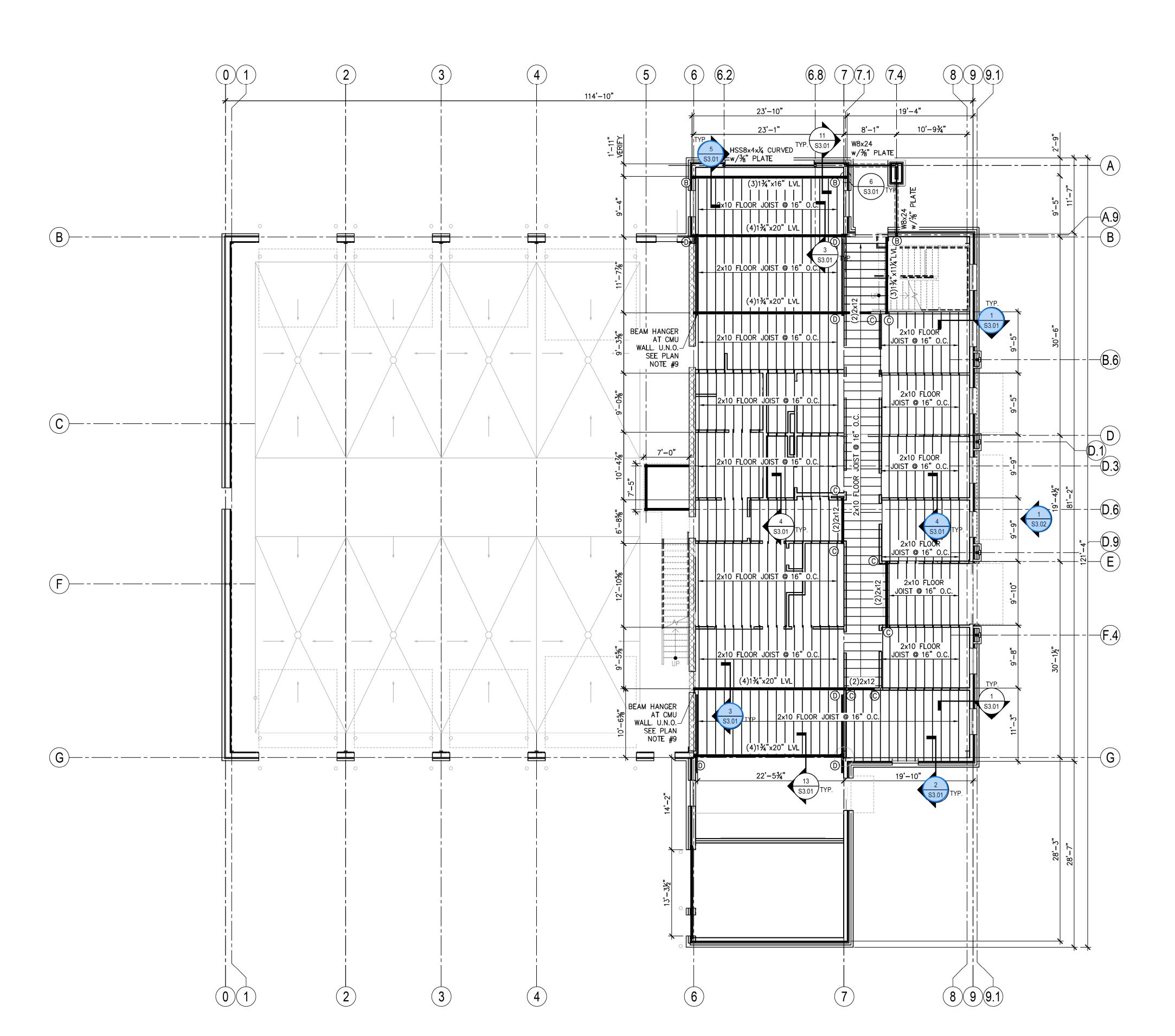
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Washington, NC 27889

RPA Project No.: 2024223





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STATION

MEZZANINE FLOOR FRAMING PLAN

1/8" = 1'-0"

MEZZANINE FLOOR FRAMING PLAN NOTES:

- 1. SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL STRUCTURAL NOTES AND SCHEDULES.
- 2. ALL BUILDING DIMENSIONS ARE FROM FACE TO FACE OF STUD WALLS, U.N.O. 3. SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS, WALL
- OPENINGS, ETC. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 FINISHED FLOOR ELEVATION IS +11'-4". ABOVE GROUND
- LEVEL SLAB. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR ACTUAL FINISHED FLOOR ELEVATION. 6. PROVIDE DOUBLE JOIST UNDER ALL WALLS PARALLEL TO
- FLOOR TRUSS/JOIST. U.N.O. PROVIDE (2)HEADER AND (2)TRIMMER FOR ALL OPENING
- BIGGER THEN 2'-0"x2'-0'. U.N.O. 8. SEE DETAIL 1 & 2/S2.01 FOR MASONRY CONSTRUCTION

HEAVY DUTY FACE MOUNT JOIST HANGER. FILL BLOCK

9. ATTACH LVL BEAM TO CMU WALL WITH 'HGUM' SIMPSON

CORE SOLID AT HANGER LOCATION. U.N.O.

MEZZANINE FLOOR FRAMING PLAN LEGEND				
A	DENOTES (4) 6"-16GA STUD PACK			
B	DENOTES (3) 6"-16GA STUD PACK			
©	DENOTES (3) 3%"-16GA STUD PACK			
0	DENOTES (5) 6"-16GA STUD PACK			
U.N.O.	DENOTES 'UNLESS NOTED OTHERWISE'			

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MEZZANINE FLOOR

FRAMING PLAN

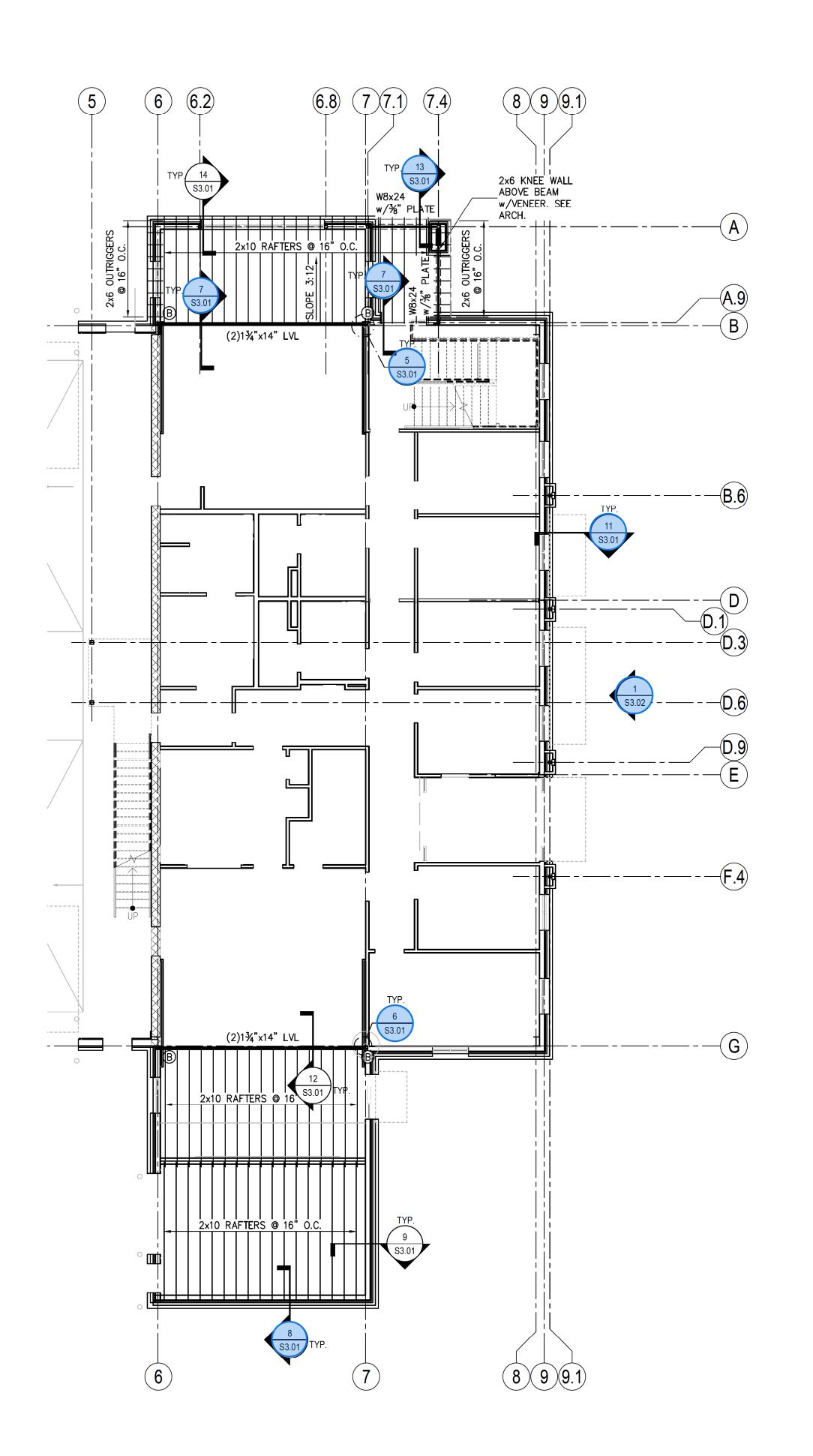
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LOW ROOF &

1/8" = 1'-0"

ROOF FRAMING PLAN

STUD WALLS, U.N.O.

OPENINGS, ETC.

ROOF FRAMING PLAN NOTES:

FLOOR TRUSS/JOIST. U.N.O.

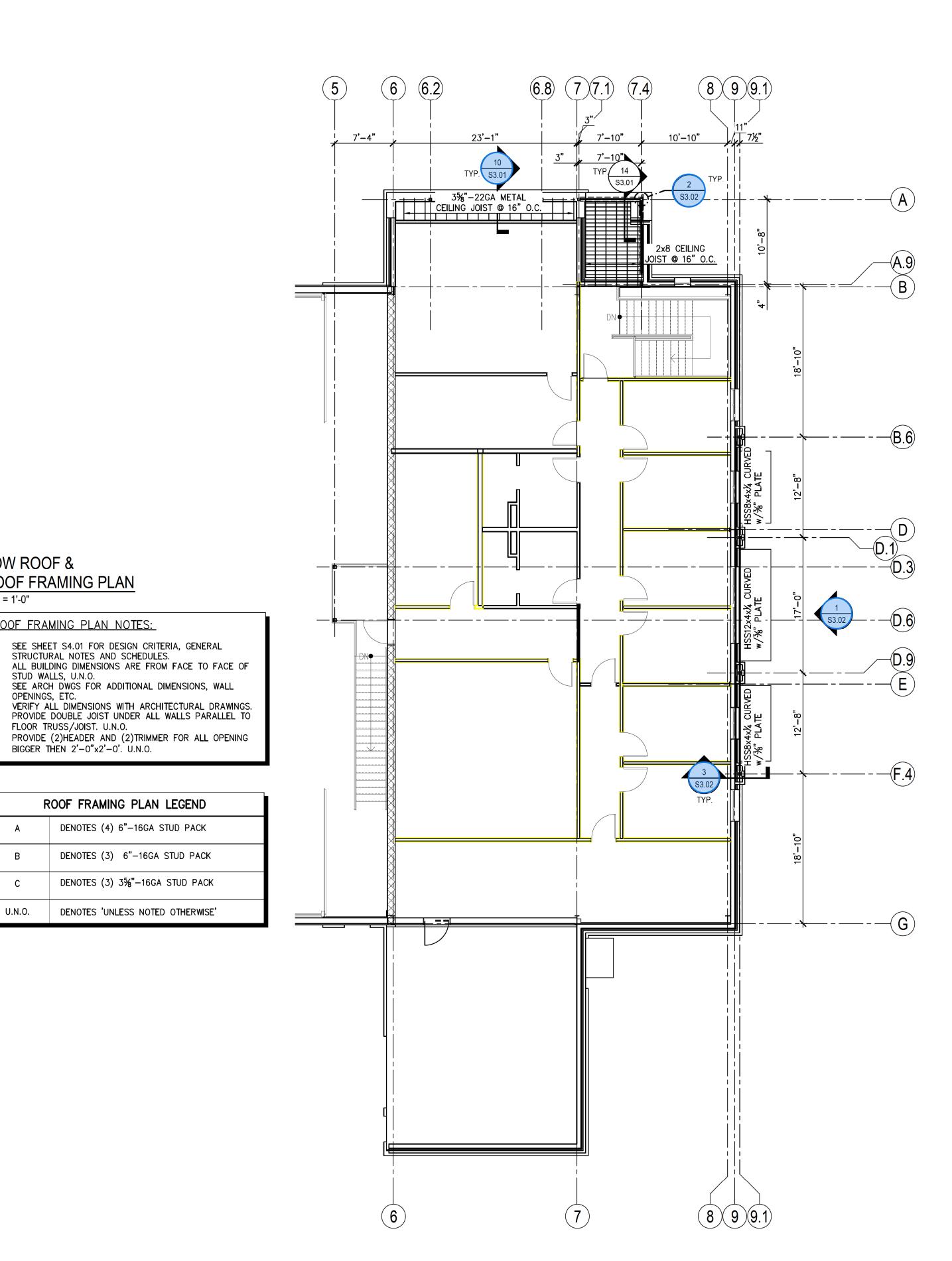
ROOF FRAMING PLAN LEGEND

DENOTES (4) 6"-16GA STUD PACK

DENOTES (3) 6"-16GA STUD PACK

DENOTES (3) 3%"-16GA STUD PACK

DENOTES 'UNLESS NOTED OTHERWISE'





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STATION

SECOND FLOOR **CEILING FRAMING PLAN**

SECOND FLOOR CEILING FRAMING PLAN NOTES:

1. SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL

BIGGER THEN 2'-0"x2'-0'. U.N.O.

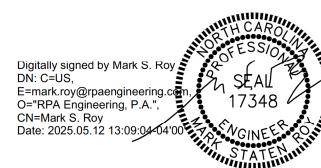
- STRUCTURAL NOTES AND SCHEDULES. 2. ALL BUILDING DIMENSIONS ARE FROM FACE TO FACE OF STUD WALLS, U.N.O.
- . SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS, WALL
- OPENINGS, ETC. . VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. 5. PROVIDE (2)HEADER AND (2)TRIMMER FOR ALL OPENING

ROOF FRAMING PLAN LEGEND				
Α	DENOTES (4) 6"-16GA STUD PACK			
В	DENOTES (3) 6"-16GA STUD PACK			
С	DENOTES (3) 3%"-16GA STUD PACK			
U.N.O.	DENOTES 'UNLESS NOTED OTHERWISE'			

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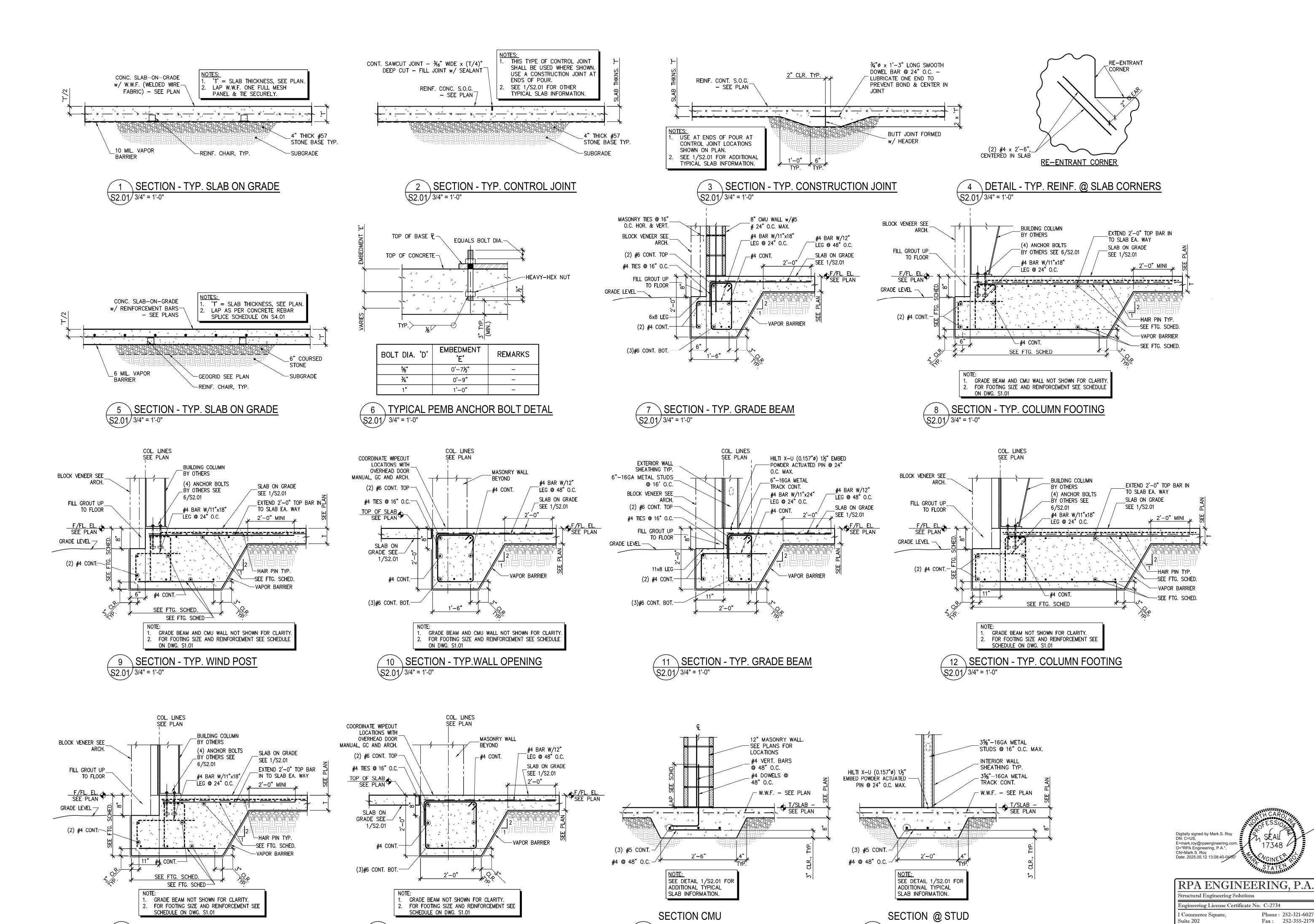
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15 WALL w/ THICKENED SLAB

\$2.01\\ 3/4" = 1'-0"

14 SECTION - TYP.WALL OPENING

\$2.01/3/4" = 1'-0"

SECTION - TYP. WIND POST

\$2.01/3/4" = 1'-0"

16 WALL w/ THICKENED SLAB

\$2.01\(\) 3/4" = 1'-0"

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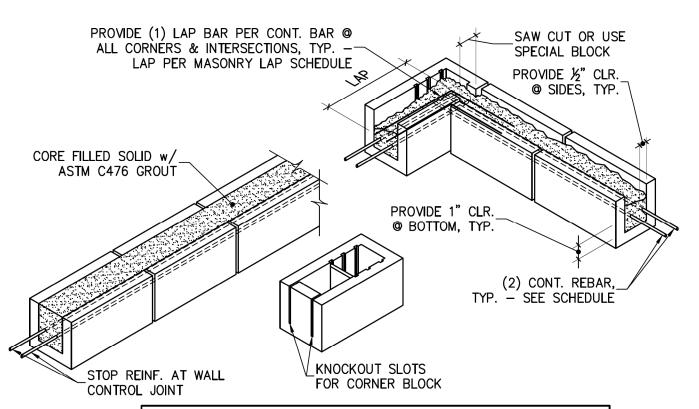
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SHEET NAME & NUMBER
FOUNDATION PLAN

S2.01

Washington, NC 27889

RPA Project No.: 2024223



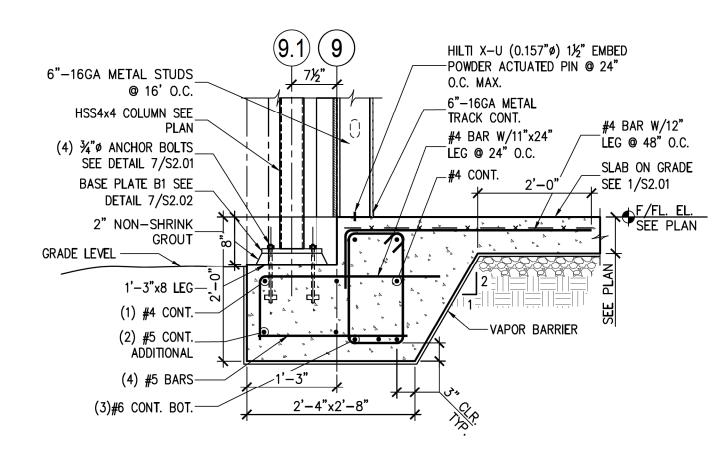
CMU BOND	BEAM CONT.	REINFORCING
CMU THK.	REINF.	REMARKS
8"	(2) # 5	-

NOTES:

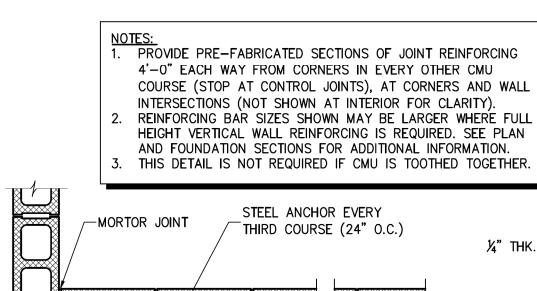
1. MATCH THICKNESS OF CORNER LAP BARS AND CONTINUOUS BARS.

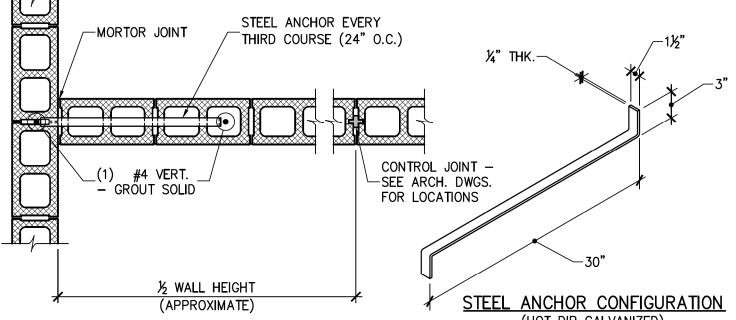
2. SEE MASONRY LAP SCHEDULE FOR LAP REQUIREMENTS AT CORNERS AND INTERSECTIONS.

1 DETAIL - TYP. BOND BEAM



5 SECTION - TYP. GRADE BEAM S2.02 3/4" = 1'-0"





2 PLAN DETAIL - TYPICAL INTERIOR CMU WALL REINFORCING \$2.02 3/4" = 1'-0"

6" METAL STUD

_LEG @ 24" O.C.

#4 BAR W/11"x24"

#4 BAR W/12"

_SLAB ON GRADE

__ SEE 1/S2.01

BEYOND

_#4 CONT.

COL. LINES ȘEE PLAN

6 SECTION - TYP. GRADE BEAM

HSS4x4 COLUMN SEE

SEE DETAIL 7/S2.01

BASE PLATE B2 SEE_

DETAIL 7/S2.02

FILL GROUT UP

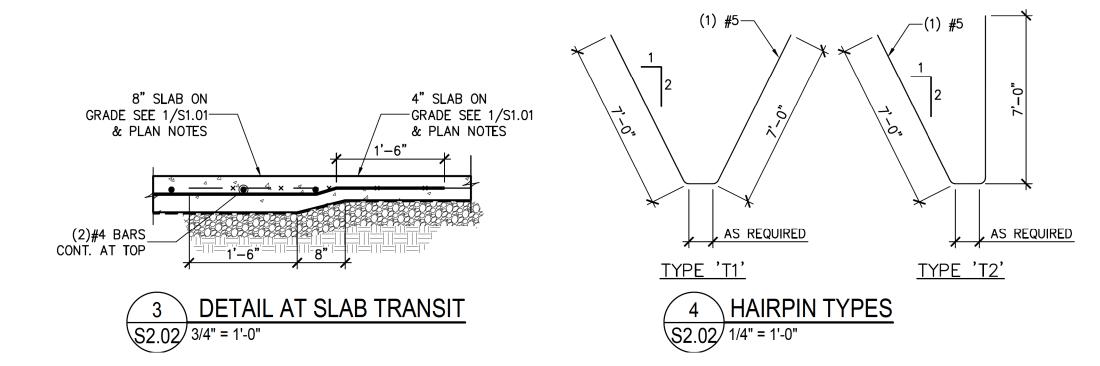
11x8 LEG-

\$2.01 3/4" = 1'-0"

(3)#6 CONT. BOT.-

2" NON-SHRINK

(2) 3/4" ANCHOR BOLTS_



_EQUALS BOLT DIA.

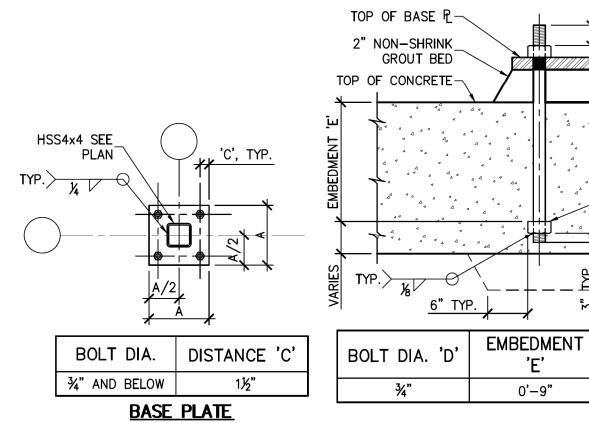
HEAVY-HEX

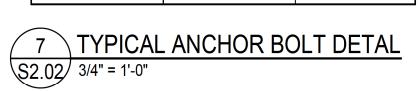
THICKENED SLAB OR

FOUNDATION

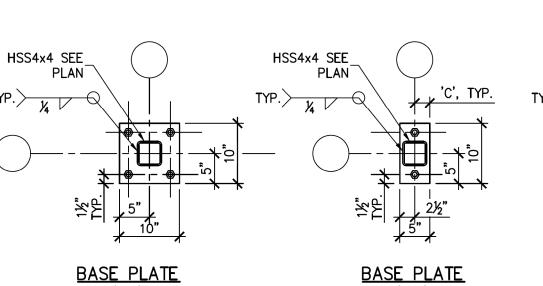
AS R**E**Q'D.

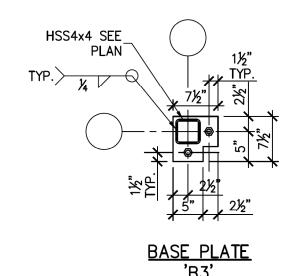
REMARKS





0'-9"





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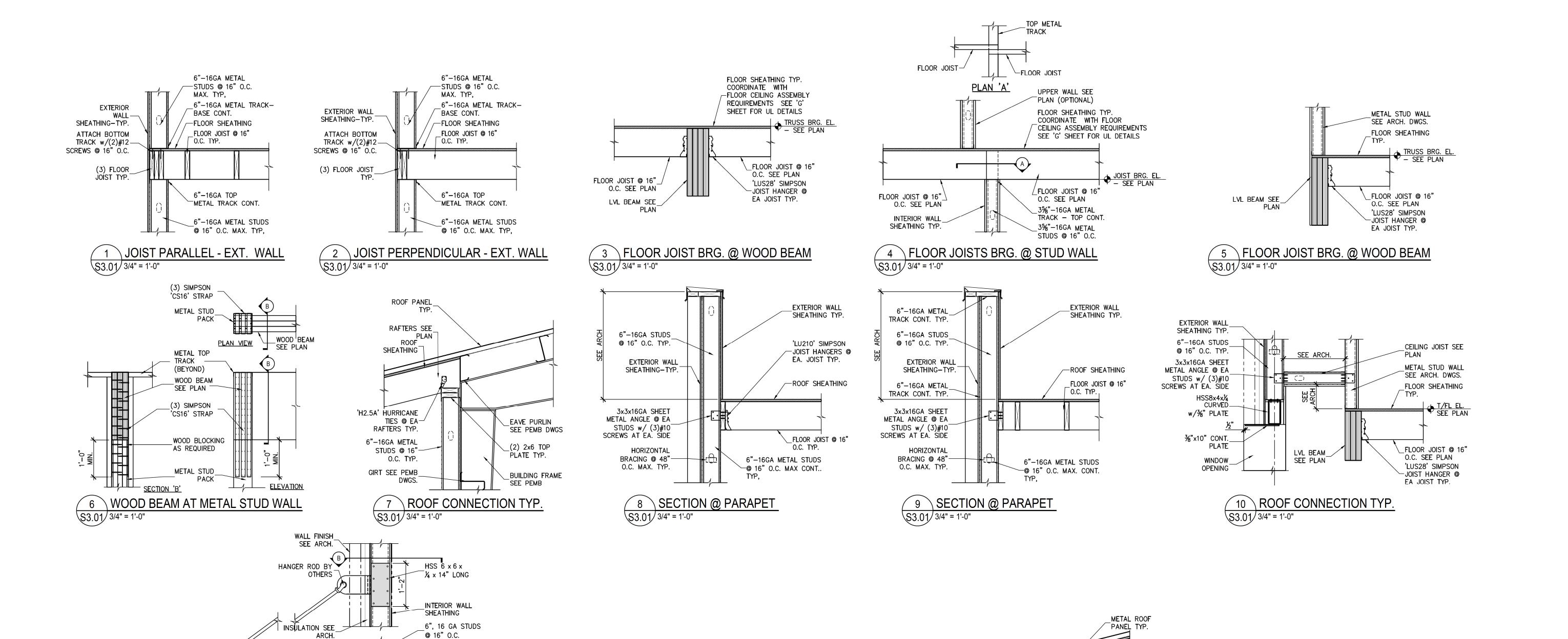
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MEZZANINE FLOOR FRAMING PLAN Phone: 252-321-6027 Fax: 252-355-2179

S2.02



ROOF SHEATHING

'LU28' SIMPSON

TOP/ 2ND FLOOR EL. +11'-4"

L4x4x¼"-12"_

ATTACH BOTTOM

SCREWS @ 16" O.C.

TRACK w/(2)#12—

20" LVL BEAM_

SEE PLAN

\$3.01\bigside 3/4" = 1'-0"

JOIST HANGER TYP.

6", 16 GA STUD

HSS, ATTACH w/

8-#12 SCREWS

HSS 6 x 6 x 1/4

x 14" LONG

EACH SIDE OF

RAFTERS SEE

TYP.

EXTERIOR WALL

SHEATHING TYP.

BUILDING FRAME

6"-16GA STUDS

__@ 16" O.C. TYP.

14" LVL BEAM

SEE PLAN

FLOOR JOIST

'LU28' SIMPSON

JOIST HANGER TYP.

SEE PLAN

12 SECTION AT FLOOR - ROOF

BEYOND SEE PLAN

SHEATHING TYP

BRICK VENEER

BRICK VENEER

EXTERIOR WALL

SHEATHING TYP.

6"-16GA STUDS

@ 16" O.C. TYP.

W8 BEAM SEE

¾"x12" CONT.

PLAN

PLATE

\$3.01 3/4" = 1'-0"

SEE ARCH.

SEE ARCH.

-RAFTERS SEE PLAN

'H2.5A' SIMPSON

EA. RAFTER TYP.

-HURRICANE TIES @

ATTACH TO BEAM

6"-16GA METAL TRACK

w/(2)#12 SELF TAPPING

(2)2x_ LEDGER CUT TO

w/1/2"ø THRU BOLTS @

FIT ATTACH TO BEAM

12" O.C. MAX.

CEILING JOIST

SEE PLAN

SECTION AT PORCH

WALL SHEATHING-

CANOPY BY

 $L 2 \times 2 \times \% \times 6$ " LONG

SEE CANOPY MNF.

FOR BOLT SIZE &-

\$3.01 3/4" = 1'-0"

SEE ARCHITECT

NOTES: 1. VERIFY WITH VENDOR

DRAWINGS.

OTHERS

P_ 7"x % x 6" LONG—

SPACING

PLAN SECTION 'A'

11 SECTION AT FRONT CANOPY

HSS 6 x 6 x

 $-\frac{1}{4} \times 14$ LONG

STEEL ANGLE SEE

-MASONRY VENEER

LINTEL SCHEDULE

SCHEDULE PLAN

L 3 × 2 × ¾ × _ 6" LONG

ዊ 4"x ½" x

w/EYE HOLE

SEE CANOPY

MNF. FOR-

BOLT SIZE

9" LONG — ⊀

PLAN SECTION 'B'

HEADER SEE

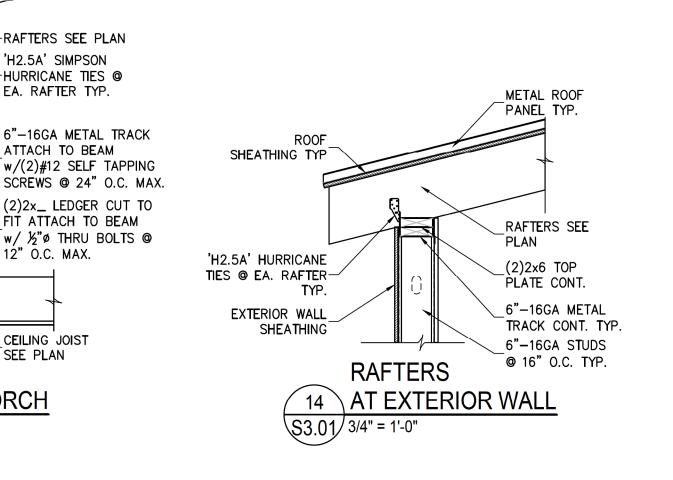
6", 16 GA STUD

SCREWS

_EACH SIDE OF HSS,

ATTACH w/ 8-#12

_HSS 6 x 6 x 1/4 x 14"





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> STATION FIRE

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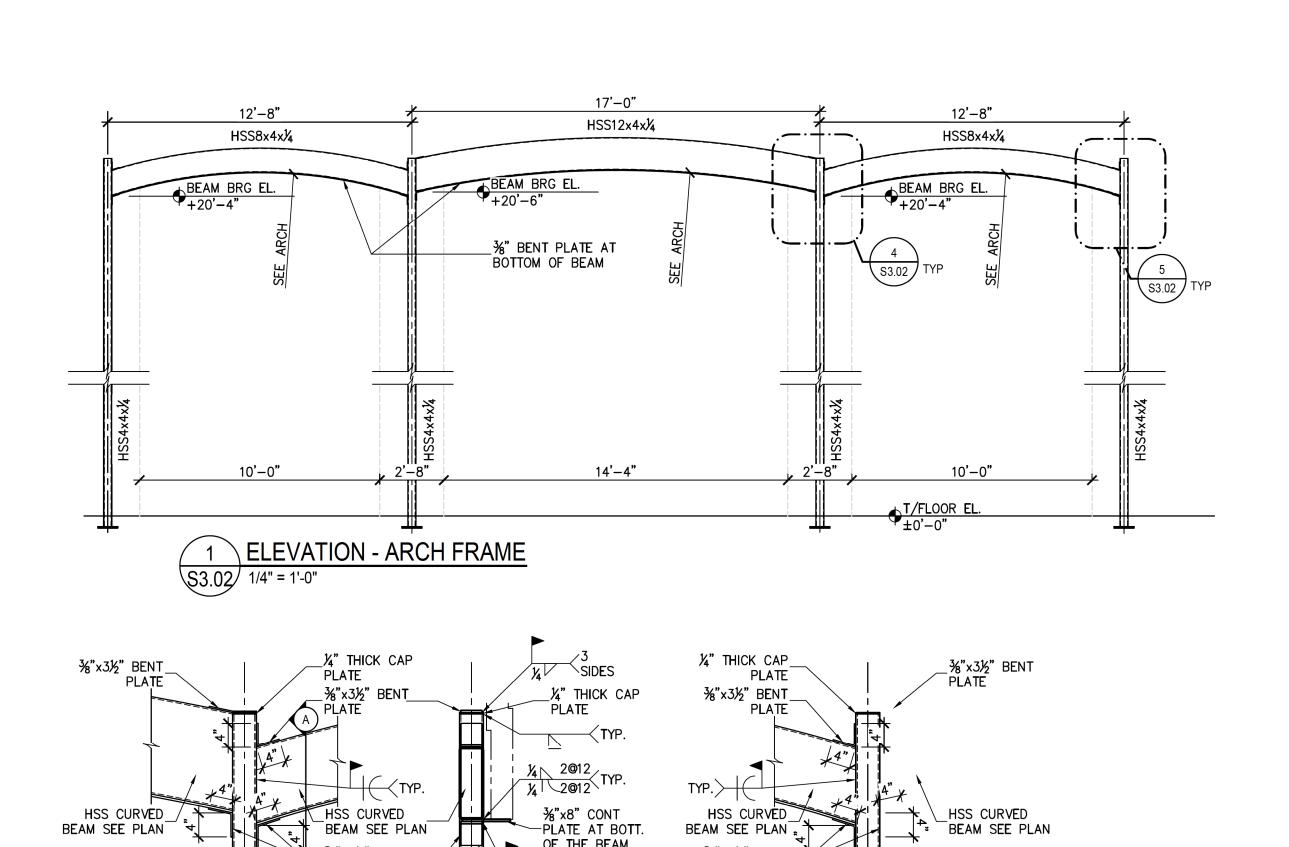
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FRAMING SECTIONS & **DETAILS** Phone: 252-321-6027

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S3.01

SHEET NAME & NUMBER



¾"x8" CONT
PLATE AT BOTT.

OF THE BEAM

SIDES

SECTION 'A'

¾"x3½" BENT

HSS CURVED

5 BEAM & COLUMN CONN.

HSS COLUMN_ SEE PLAN

\$3.02 3/4" = 1'-0"

HSS CURVED
BEAM SEE PLAN

HSS CURVED
BEAM SEE PLAN

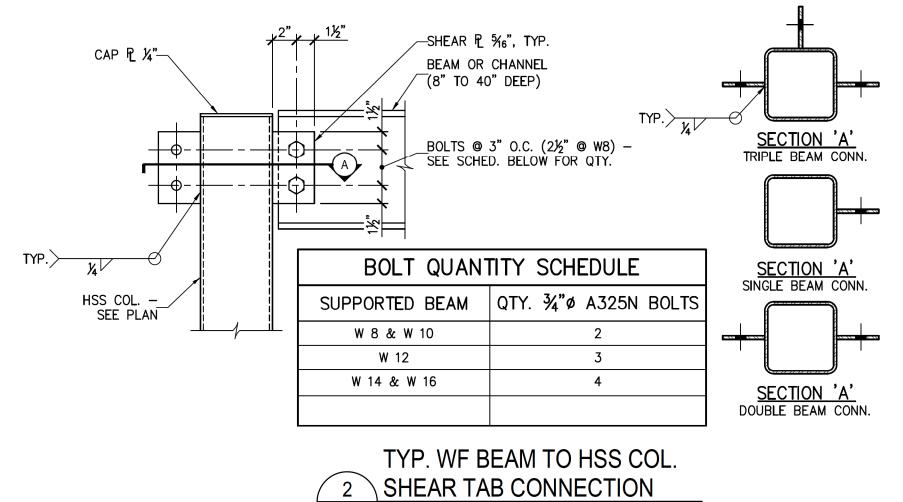
_HSS COLUMN _ SEE PLAN

HSS CURVED_______BEAM SEE PLAN _______

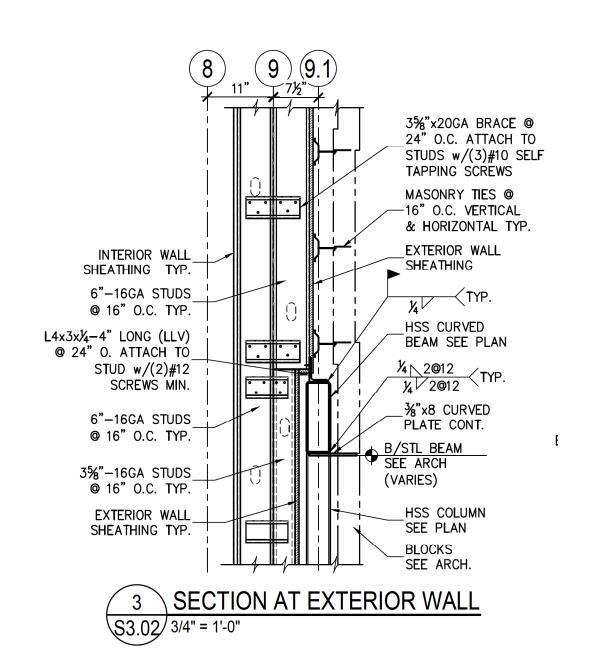
\$3.02 3/4" = 1'-0"

HSS CURVED

4 BEAM & COLUMN CONN.



\$3.02 N.T.S.





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GENERAL STRUCTURAL NOTES:

- METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.
- THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS OR OPENINGS NOT HEREIN INDICATED.
- COORDINATE THESE DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS.
- 1.4. VERIFY ALL FLOOR AND ROOF OPENING SIZES AND LOCATIONS, EQUIPMENT PAD SIZES AND LOCATIONS,
- ANCHOR BOLT LAYOUTS, ETCETERA, WITH EQUIPMENT SELECTED. VERIFY BUILDING LOCATION AND ORIENTATION WITH OWNER AND LOT SETBACK REQUIREMENTS BEFORE ANY CONSTRUCTION IS STARTED ON THE PROJECT.
- CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION DIMENSIONS WHICH IMPACT NEW CONSTRUCTION PRIOR TO FABRICATING ANY REBAR, STEEL, TRUSSES, ETCETERA.
- DO NOT CUT, NOTCH, OR OTHERWISE MODIFY ANY STRUCTURAL MEMBERS UNLESS SPECIFICALLY INDICATED ON THE DRAWINGS WITHOUT APPROVAL OF THE ENGINEER OF RECORD.
- 1.8. CUTTING OF STEEL MEMBERS AND INSTALLATION OF HOLES IN STEEL MEMBERS SHALL BE DONE BY CUTTING OR DRILLING. DO NOT USE TORCHES FOR CUTTING UNLESS APPROVED BY THE ENGINEER OF RECORD.
- CONTRACTOR IS RESPONSIBLE FOR DESIGN AND INSTALLATION OF ALL SHORING REQUIRED TO SUPPORT NEW AND EXISTING STRUCTURAL ELEMENTS.

2. FOUNDATION

- ALL FOOTINGS SHALL BE ON UNDISTURBED SOIL OR 98% COMPACTED FILL PER ASTM D698.
- 2.2. NO FOOTINGS OR SLABS SHALL BE POURED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, ICE OR LOOSE MATERIAL.
- 2.3. EXCAVATIONS FOR FOOTINGS SHALL HAVE THE SIDES AND BOTTOMS TEMPORARILY LINED WITH 6 MIL. POLYETHYLENE IF PLACEMENT OF CONCRETE DOES NOT OCCUR WITHIN 24 HRS OF THE EXCAVATION OF THE
- 2.4. ADVERSE FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION SUCH AS SOFT SOILS, ORGANIC MATTER,
- ETCETERA, SHALL BE REPORTED TO THE ENGINEER BEFORE FURTHER CONSTRUCTION IS ATTEMPTED. IF UNDERMINING OF FOOTINGS OCCURS, FILL VOIDS WITH LEAN CONCRETE MIX. DO NOT ATTEMPT TO

3. REINFORCED CONCRETE MASONRY

REPLACE AND RECOMPACT SOIL.

- LOAD-BEARING MASONRY PIERS OR WALLS, FOUNDATION WALLS, AND ANY OTHER MASONRY SO DESIGNATED ON THE DRAWINGS, ARE CONSIDERED TO BE STRUCTURAL MASONRY.
- COMPRESSIVE STRENGTH OF MASONRY UNITS: SOLID CLAY UNITS ______ 8250 PSI
- CONCRETE UNITS ____ ____ 1900 PSI ON NET AREA
- MINIMUM NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY (Fm) IS 1,900 PSI. MORTAR SHALL BE TYPE 'S' ASTM C270.
- 3.4. GROUT FOR REINFORCED MASONRY SHALL BE FINE GROUT ASTM C476. MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 3000 PSI. MAXIMUM HEIGHT TO WHICH MASONRY SHALL BE LAID BEFORE FILLING IS 6'-0". PROVIDE CLEANOUT OPENINGS AT THE BOTTOM OF EACH GROUT LIFT. CLEANOUT OPENINGS SHALL BE PROVIDED AT EACH CELL TO BE FILLED WITH GROUT.
- 3.5. REINFORCING GRADE AND DETAILS FOR MASONRY, SHALL BE AS THAT FOR CONCRETE. TIE IN REBAR IN POSITION, AND PLACE CONCRETE AROUND REINFORCING DURING CONSTRUCTION OF MASONRY. DO NOT PUSH REINFORCING DOWN INTO PREVIOUSLY PLACED GROUT FILL. SET BOLTS SIMILARLY. TIE WYTHES WITH HORIZONTAL REINFORCING AS SPECIFIED.
- 3.6. ALL CELLS BELOW GRADE SHALL BE FULLY GROUTED WITH MASONRY GROUT.
- PROVIDE HORIZONTAL JOINT REINFORCING AT 16" O.C. UNLESS OTHERWISE NOTED. 3.8. PLACE ALL VERTICAL REINFORCING BARS IN CENTERS OF BLOCK CELLS UNLESS OTHERWISE NOTED.
- 3.9. FILL ALL CELLS, AT VERTICAL REINFORCING, FULL HEIGHT WITH MASONRY GROUT.
- 3.10. CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS IN MASONRY. SEE ARCHITECTURAL
- MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SIZE AND LOCATION OF OPENINGS.
- 3.11. ALL MASONRY WORK PERFORMED, SHALL BE IN ACCORDANCE WITH ACI/ASCE 530. MASONRY CONSTRUCTION AND MATERIALS USED, SHALL CONFORM TO ALL REQUIREMENTS OF THESE CONTRACT DOCUMENTS.
- 3.12. UNLESS OTHERWISE SHOWN, MASONRY WALLS SHALL HAVE VERTICAL CONTROL JOINTS AT A MAXIMUM SPACING OF 40'-0" ON CENTER FOR BRICK AND OF 25'-0" FOR CMU. THE JOINT SHALL BE FORMED USING PVC MATERIAL CONFORMING TO ASTM D2287, TYPE PVC 654-4. COORDINATE LOCATION OF JOINTS WITH
- 3.13. PLACE A CONTINUOUS HORIZONTAL CMU BOND BEAM AT EACH FLOOR, AND AT THE TOP OF THE WALL, AND AT INTERMEDIATE LOCATIONS AS REQUIRED TO PROVIDE A MAXIMUM VERTICAL SPACING OF 12'-0", UNLESS OTHERWISE NOTED ON THE PLAN.

- ALL PLACED CONCRETE, SHALL HAVE NORMAL WEIGHT COARSE AGGREGATES UNLESS OTHERWISE NOTED, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) AT 28 DAYS AS SHOWN ON THE CONCRETE MATERIALS SCHEDULE.
- GROUT FOR BASE PLATES SHALL BE NON-METTALIC, NON-SHRINKABLE GROUT, AND SHALL HAVE A
- MINIMUM SPECIFIED COMPRESSIVE STRENGTH, AT 28 DAYS, OF 5000 PSI. 4.3. NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
- 4.4. CHAMFER ALL EXPOSED EXTERNAL CORNERS OF CONCRETE WITH 3/4" x 45 DEGREE CHAMFER, UNLESS OTHERWISE NOTED.
- HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCING SHALL BE CONTINUOUS, AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED, WITH A CLASS B TENSION SPLICE, AT CORNERS AND INTERSECTIONS. TOP BAR CRITERIA SHALL APPLY IF 12" OR MORE OF FRESH CONCRETE IS PLACED BELOW BAR.
- 4.6. SEE ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING / DAMPPROOFING DETAILS. 4.7. ALL DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, UNLESS OTHERWISE NOTED ON THE
- DRAWINGS 4.8. SEE ARCHITECTURAL DRAWINGS FOR TYPE AND LOCATION OF FLOOR FINISHES.
- 4.9. SEE MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS FOR ADDITIONAL WALL / SLAB OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 4.10. ALL REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60. 4.11. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- 4.12. DETAIL AND FABRICATE REINFORCING STEEL IN ACCORDANCE WITH THE ACI DETAILING MANUAL.
- 4.13. IN-PLACE REINFORCING STEEL, SHALL BE REVIEWED BY THE ENGINEER PRIOR TO PLACEMENT OF CONCRETE. 4.14. AT CORNERS AND INTERSECTIONS, PROVIDE BARS OF THE SAME NUMBER AND SIZE AS THE LONGITUDINAL
- BARS IN THE FOOTING. 4.15. CONCRETE MATERIALS SHALL BE AS FOLLOWS:
- 4.15.1. USE TYPE I/II PORTLAND CEMENT CONFORMING TO ASTM C150
- 4.15.2. AGGREGATE SHALL CONFORM TO ASTM C33 (FINE AND COURSE AGGREGATES)
- 4.15.3. AIR ENTRAINING ADMIXTURE SHALL CONFORM TO ASTM C260 4.15.4. PLASTICIZER CAN BE USED TO IMPROVE WORKABILITY IF REQUIRED
- 4.16. CONCRETE MIX DESIGN: 4.16.1. MAXIMUM WATER/CEMENT RATIO - 0.50 FOR SLAB, 0.55 FOR FOOTINGS AND OTHER CONCRETE UNLESS
- OTHERWISE NOTED.
- SLUMP SHALL BE 4 INCHES TO 6 INCHES (WITHOUT PLASTICIZER) AIR ENTRAINMENT SHALL BE 4% TO 6% (EXTERIOR CONCRETE)
- 4.17. CONCRETE SLAB SHALL BE CURED USING A WATER-BASED CURING COMPOUND. CURING COMPOUND SHALL BE APPLIED TO ALL HORIZONTAL SURFACES. ONCE THE SURFACE WATER DISSIPATES AND THE SURFACE IS NOT MARRED BY WALKING, APPLY PER MANUFACTURER'S SPECIFICATIONS.
- 4.18. CONDUCT SLUMP, AIR, AND STRENGTH TESTS OF CONCRETE IN ACCORDANCE WITH THE FOLLOWING
- 4.18.1. SECURE SAMPLES IN ACCORDANCE WITH "METHOD OF SAMPLING FRESH CONCRETE" (ASTM C 172). MOLD AND CURE FIVE SPECIMENS FROM EACH SAMPLE IN ACCORDANCE WITH "METHOD OF MAKING ANS CURING CONCRETE COMPRESSION AND FLEXURE SPECIMENS IN THE FIELD" (ASTM C 31). FIVE SPECIMENS COMPRISE ONE TEST. TEST TWO SPECIMENS AT 7 DAYS (ASTM C 39). TEST TWO SPECIMENS AT 28 DAYS IN ACCORDANCE WITH "METHOD OF TEST FOR COMPRESSIVE STRENGTH OF MOLDED CONCRETE CYLINDERS" (ASTM C 39). TEST EVALUATION SHALL BE CONDUCTED IN ACCORDANCE WITH
- PROVISIONS OF ACI 318-05. KEEP ONE SPECIMEN IN RESERVE MAKE ONE STRENGTH TEST FOR EACH 100 CUBIC YARDS OR FRACTION THEREOF FOR EACH MIX DESIGN OF CONCRETE PLACED IN ONE DAY, EXCEPT THAT IN NO CASE SHALL A GIVEN MIX DESIGN BE REPRESENTED BY LESS THAN THREE TESTS.

5. PRE-ENGINEERED METAL BUILDINGS

- CONFIGURATION, COLUMN LOCATIONS, EAVE HEIGHTS, ROOF SLOPE, ETCETERA, SHALL BE AS SHOWN ON THE DRAWINGS. SHOULD BUILDING MANUFACTURER WISH TO FURNISH A SYSTEM THAT WILL DIFFER FROM THAT SHOWN, WRITTEN APPROVAL SHALL BE OBTAINED FROM THE ARCHITECT/ENGINEER OF RECORD PRIOR
- 5.2. BUILDING DESIGN AND LOAD APPLICATION SHALL CONFORM TO THE CURRENT NORTH CAROLINA STATE BUILDING CODE. THE COLLATERAL LOAD SHALL NOT BE USED TO REDUCE THE EFFECTS OF WIND LOADS ON THE BUILDING. REFER TO THE 'GENERAL' SECTION OF THE STRUCTURAL NOTES FOR ADDITIONAL LOADING INFORMATION.
- 5.3. THE METAL BUILDING FRAMES SHALL BE DESIGNED SUCH THAT THE MAXIMUM HORIZONTAL DRIFT DUE TO WIND AND SEISMIC LOADING SHALL SATISFY AN H /180 CRITERIA. THE MAXIMUM VERTICAL DEFLECTION OF PRIMARY AND SECONDARY FRAMING MEMBERS SHALL BE WITHIN THE TOLERANCES PROSCRIBED BY THE NC STATE BUILDING CODE. MANUFACTURER SHALL VERIFY THAT THE DEFLECTION CRITERIA ARE COMPATIBLE WITH EXTERIOR AND INTERIOR FINISHES SUPPORTED BY THE METAL BUILDING STRUCTURE.
- THE FOOTING DESIGN IS BASED UPON AN ASSUMED LOADING OF THE METAL BUILDING SUPER-STRUCTURE. THE FOUNDATIONS SHALL NOT BE CONSTRUCTED UNTIL THE STRUCTURAL ENGINEER HAS REVIEWED THE ACTUAL DESIGN REACTIONS SUPPLIED BY THE MANUFACTURER.

6. COLD FORMED METAL FRAMING

- 6.1. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE AS SHOWN ON THE DRAWINGS AND AS REQUIRED BY THE MANUFACTURER'S SPECIFICATIONS.
- 6.2. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) 'SPECIFICATION FOR THE DESIGN OF COLD FORMED STRUCTURAL MEMBERS', LATEST
- 6.3. ALL STRUCTURAL MEMBERS SHALL BE FORMED OF CORROSION RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENT OF ASTM-A446, WITH A MINIMUM YIELD STRENGTH OF 40 KSI.
- ALL STRUCTURAL MEMBERS SHALL BE ZINC COATED AND CONFORM TO ASTM-A525. INSTALL JACK AND KING STUDS AT ALL WINDOW AND DOOR OPENINGS IN EXTERIOR WALLS AND INTERIOR
- LOAD-BEARING WALLS PER THE BOX BEAM HEADER SCHEDULE. ALL EXTERIOR STUD WALLS ARE LOAD BEARING UNLESS OTHERWISE NOTED. SEE ARCHITECTURAL DETAILS FOR ADDITIONAL INFORMATION ON CONNECTIONS. ALL EXTERIOR WALL STUDS SHALL BE 6", 16 GA., SPACED AT 16" O.C., UNLESS OTHERWISE NOTED. STUD TRACK GAUGE SHALL MATCH THE STUD GAUGE SPECIFIED UNLESS OTHERWISE NOTED.
- 6.7. BOTTOM TRACK SHALL BE ATTACHED WITH 'HILTI X-U' POWDER ACTUATED FASTENERS (0.157" SHANK DIAMETER) WITH 1¾" EMBEDMENT AT 12" O.C.
- 6.8. ALL STUDS INSTALLED BELOW STEEL BEAMS OR OTHER LOAD BEARING STRUCTURAL MEMBERS SHALL BE ATTACHED WITH A CONTINUOUS DEFLECTION TRACK OR DEFLECTION CLIPS EQUIVALENT TO 'VERTITRACK' OR 'VERTICLIP' FROM 'THE STEEL NETWORK'.

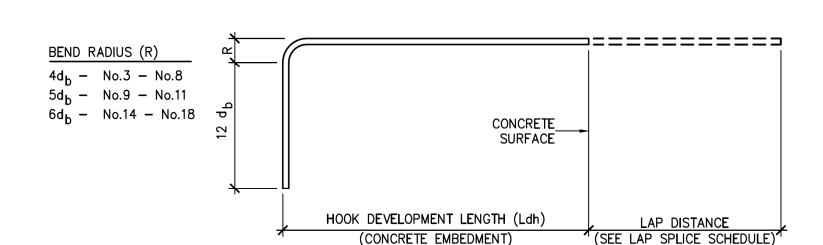
7. WOOD FRAMING

- 7.1. ALL STRUCTURAL WOOD MEMBERS SHALL BE No. 2 SOUTHERN YELLOW PINE, 19% MAXIMUM MOISTURE CONTENT, UNLESS OTHERWISE NOTED. INTERIOR NON BEARING PARTITIONS MAY BE No. 2 SPRUCE (SPF) ALL WOOD FRAMING, DIRECTLY EXPOSED TO WEATHER, OR IN DIRECT CONTACT WITH MASONRY, SOIL OR
- CONCRETE, SHALL BE PRESSURE TREATED, UNLESS OTHERWISE NOTED. 7.3. ALL LVLs, DIRECTLY EXPOSED TO WEATHER, OR IN DIRECT CONTACT WITH MASONRY, SOIL OR CONCRETE SHALL BE EXTERIOR GRADE, UNLESS NOTED OTHERWISE
- ALL METAL CONNECTORS SHALL BE HOT DIP GALVANIZED. INSTALL ALL CONNECTORS PER THE MANUFACTURER'S RECOMMENDATIONS. METAL CONNECTOR DESIGNATIONS INDICATED ON PLANS, ARE FOR 'SIMPSON STRONG—TIE' ANCHORS. ANCHORS FROM OTHER MANUFACTURERS MAY BE USED, PROVIDED THEY HAVE EQUIVALENT STRENGTH.
- 7.5. ALL NAILED CONNECTIONS SHALL BE IN ACCORDANCE WITH NORTH CAROLINA STATE BUILDING CODE TABLE <u> 2304.9.1, — FASTENING SCHEDULE,</u> UNLESS OTHERWISE NOTED.
- FRAMING CONNECTIONS THAT ARE BOLTED OR SCREWED, SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF <u>THE NATIONAL DESIGN SPECIFICATION FOR WOOD.</u>
- 7.7. PROVIDE STUDS AND HEADERS AT ALL EXTERIOR WALLS AND INTERIOR BEARING WALLS AS FOLLOWS, UNLESS OTHERWISE NOTED:

<u>OPENING WIDTH</u>	<u>STUDS</u>	<u>HEADER</u>
0'-0" TO 6'-0"	2 KING STUDS, 1 JACK STUD	(2) 2 x 10 @ 2 x 4 WALL
		(3) 2 x 10 @ 2 x 6 WALL
6'-1" TO 8'-0"	2 KING STUDS, 2 JACK STUDS	(2) 2 x 10 @ 2 x 4 WALL
		(3) 2 x 10 @ 2 x 6 WALL
8'-1" TO 12'-0"	3 KING STUDS, 2 JACK STUDS	(2) 2 x 12 @ 2 x 4 WALL
		(3) 2 x 12 @ 2 x 6 WALL

8. WOOD DECKING/SHEATHING

- 8.1. WALL SHEATHING SHALL BE $^{1}\%_{2}$ " PLYWOOD OR ORIENTED STRAND BOARD (OSB), UNLESS OTHERWISE NOTED. ATTACH WALL SHEATHING TO FRAMING WITH 10d NAILS AT 4" O.C. AT PANEL EDGES AND 12" O.C. AT INTERIOR MEMBERS. PROVIDE SOLID BLOCKING AT PANEL EDGES (48" O.C.).
- 8.2. ROOF SHEATHING SHALL BE 13/2" PLYWOOD OR ORIENTED STRAND BOARD (OSB), UNLESS OTHERWISE NOTED. ATTACH ROOF SHEATHING TO FRAMING WITH 8d NAILS AT 4" O.C. AT PANEL EDGES AND 12" O.C. AT
- 8.3. SUB-FLOOR SHALL CONSIST OF 3/4" TONGUE AND GROOVE PLYWOOD UNLESS OTHERWISE NOTED. FASTEN WITH 8d NAILS AT 6" O.C. AT PANEL EDGES, AND AT 12" O.C. AT INTERIOR SUPPORTS. THE FLOOR SHEATHING TO COORDINATE WITH UL FLOOR CEILING ASSEMBLY AS OUTLINED IN 'G' SERIES.



. CONCRETE IS NORMAL WEIGHT

CONCRETE. IF LIGHTWEIGHT

LENGTHS IN TABLE BY 1.3.

NOT BE MET.

TO NOT BE MET.

REDUCTION OF EXCESS

FOR 180° HOOKS ALSO.

 $d_h = BAR DIAMETER$

CONCRETE IS USED, MULTIPLY

2. BAR YIELD STRENGTH (fy) IS 60 KSI.

4. TIE OR STIRRUP REQUIREMENTS OF

REINFORCEMENT IS NOT TAKEN.

SIDE COVER REQUIREMENTS OF ACI

SECTION 25.4.3.2 ARE ASSUMED TO

ACI SECTION 25.4.3.2 ARE ASSUMED

HOOK DEVELOPMENT LENGTH IS VALID

	STANDARD HOOKS IN TENSION (PER ACI 318-02) HOOK DEVELOPMENT LENGTH Ldh (INCHES)				
BAR SIZE	f'c 3000 psi	f'c 4000 psi	f'c 5000 psi		
#3	9	7	7		
#4	11	10	9		
#5	14	12	11		
#6	17	15	13		
#7	19	17	15		
#8	22	19	17		
#9	25	22	19		
#10	28	24	22		
#11	31	26	24		

CONCRETE MATERIALS SCHEDULE			
LOCATION MINIMUM COMPRESSIVE STRENGTH (AT 28 DAYS)		REMARKS	
FOUNDATIONS 3000 PSI		_	
SLAB ON GRADE	4000 PSI	_	
EQUIPMENT PADS	3000 PSI	_	
CONCRETE FOR MASONRY CORES, BOND BEAMS	3000 PSI	ASTM C476 GROUT	
MISCELLANEOUS	3000 PSI	-	

CONCRETE REBAR SPLICE SCHEDULE					
BAR	LAP LENGTH (in.)				
SIZE	f'c = 3000 psi	f'c = 4000 psi	f'c = 5000 psi		
#4	22	19	17		
# 5	28	24	21		
#6	32	29	26		
#7	48	42	37		
#8	55	48	43		
#9	62	54	48		
#10	68	60	53		
#11	76	66	59		

- 1. CONCRETE IS NORMAL WEIGHT CONCRETE. IF LIGHTWEIGHT CONCRETE IS USED, MULTIPLY LENGTHS IN TABLE BY 1.3.
- 2. BAR YIELD STRENGTH (fy) IS 60 KSI. 3. BAR SPACING AND COVER REQUIREMENTS OF ACI SECTION 25.4.2.2 ARE
- ASSUMED TO BE MET. IF NOT, MULTIPLY LENGTHS IN TABLE BY 1.5. 4. REDUCTION OF EXCESS REINFORCEMENT NOT TAKEN.
- 5. IF MORE THAN 12" OF FRESH CONCRETE IS CAST IN MEMBER BELOW HORIZONTAL SPLICE, MULTIPLY LENGTHS IN TABLE BY 1.3.

EXPOSED CONCRETE FINISH SCHEDULE			
AREA	FINISH	COMMENTS	
EXTERIOR CONCRETE PAVEMENT, SIDEWALKS	COARSE BROOM	SEE NOTE 1	
SLAB ON GRADE	TROWEL	SEE NOTE 1	
EXT. EQUIP. PADS	COARSE BROOM	SEE NOTE 1	
_	_	_	

1. SEE SPECIFICATIONS SECTION, '033000 CAST-IN-PLACE CONCRETE', FOR ADDITIONAL INFORMATION.

MASONRY VENEER LINTEL SCHEDULE			
OPENING DIMENSION	ANGLE SIZE	ORIENTATION	
0'-0" THRU 4'-0"	L 4 × 4 × 1/6	N/A	
4'-1" THRU 6'-0"	L4 x 4 x %	N/A	
6'-1" THRU 8'-0"	L6 × 4 × 1/6	LLV	
8'-1" THRU 10'-0"	L6 x 4 x 3%	LLV	
10'-1" THRU 12'-0"	L7 x 4 x %	LLV	
_	_	_	

FULL HEIGHT 'KING'

'JACK' STUDS - SEE_

SCHED. FOR QTY.

SCHED. FOR QTY.

STUDS - SEE

OPENING

WIDTH

0'-0" TO 4'-0"

4'-1" TO 8'-0"

8'-1" TO 12'-0"

FOR ADDITIONAL INFORMATION.

- PROVIDE LOOSE STEEL ANGLE LINTELS FOR ALL MASONRY VENEER OPENINGS, PER ABOVE DATA UNLESS NOTED OTHERWISE.
- PROVIDE MINIMUM OF 8" BEARING FOR ALL LINTELS UNLESS NOTED OTHERWISE.
- COORDINATE HORIZONTAL LEG SIZE WITH ARCHITECTURAL DRAWINGS. SIZES MAY NEED TO BE CHANGED TO ACCOMMODATE AIR SPACE, INSULATION AND OTHER WALL COMPONENTS.

-TOP TRACK

JACK STUD

QUANTITY

-CRIPPLE STUD

'A' ELEVATION

BOX BEAM HEADER -

KING STUD

QUANTITY

SEE DETAIL A/SX.X

BOX HEADER BEAM SCHEDULE

I. USE INFORMATION IN THIS SCHEDULE UNLESS OTHERWISE NOTED ON PLAN.

2. SEE GENERAL STRUCTURAL NOTES, SECTION FOR 'COLD FORMED METAL FRAMING'

\S4.01/ 3/4" = 1'-0"

FASTEN w/ #8

SCREWS @ 24"

QTY. OF STUDS

HORIZ. MEMBER

QTY. & CONFIG.

 $(3) 6" \times 16 GA.$

(2) 8" x 16 GA.

(3) 8" x 16 GA.

OR

(2) 10" x 16 GA.

(3) 10" x 16 GA.

\ TYP. METAL STUD BOX BEAM DETAIL

STRUCTURAL DESIGN CRITERIA: **DESIGN LOADS:** ROOF DEAD LOAD ROOF MEMBRANE & INSULATION 4 PSF SHEATHING ROOF FRAMING PIPING, DUCT, ETC. 1.2. LIVE LOADS

	1ST FLOOR LIVE LOAD 100 PSF
	2ND FLOOR LIVE LOAD 40 PSF (OFFICE
1.3.	SNOW LOAD
	GROUND SNOW LOAD = 10 PSF (MAYSVILLE, NC)
	SNOW LOAD IMPORTANCE FACTOR: $I = 1.25$
	SNOW EXPOSURE FACTOR $= 1.0$
	SNOW THERMAL FACTOR = 1.0
	ROOF SNOW LOAD = 10 PSF
	BASIC DESIGN ROOF SNOW LOAD = 12.5 PSF
1.4.	WIND LOAD
	•

BASIC WIND SPEED: Vult = 150 MPH (MAYSVILLE, NC) RISK CATEGORY: ____ I WIND EXPOSURE CATEGORY: 'B' (ASCE 7-10) WIND BASE SHEAR (FOR MWFRS): $Vx = __K Vy = __K$ INTERNAL PRESSURE COEFFICIENT: ±0.55

5 PSF

1.5. SEISMIC LOADS (N.C. STATE BLDG. CODE): SEISMIC IMPORTANCE FACTOR: I = 1.0RISK CATEGORY: ____ I

SEISMIC DESIGN CATEGORY: SEISMIC DESIGN CATEGORY: ___ A ___ B __X C ___ D

MAPPED SPECTRAL RESPONSE ACCELERATION: Ss _13.7 % g S1 _6.7 % g SPECTRAL RESPONSE COEFFICIENTS: SDS 14.6 % SD1 10.6 % SEISMIC RESPONSE COEFFICIENT: Cs ___

MIN (FOR UPLIFT)

SEE PRE-ENGINEERED METAL

BUILDING DRAWINGS FOR

INFORMATION NOT SHOWN.

2 PSF

2 PSF

3 PSF

ROOF LIVE LOAD - ALL AREAS GREATER OF 20 PSF MINIMUM OR SNOW LOAD. LIVE LOAD

REDUCTION CAN BE USED IN ACCORDANCE WITH 2018 NCBC, SECTION 1607.10

SITE CLASSIFICATION: ___ A ___ B ___ C __X D BASIC STRUCTURAL SYSTEM: ___ BEARING WALL _____ DUAL w/ SPECIAL MOMENT FRAME BUILDING FRAME ____ DUAL w/ INTERMEDIATE R/C OR SPECIAL STEEL

X MOMENT FRAME _____ INVERTED PENDULUM

SEISMIC BASE SHEAR $Vx = __K Vy = __K$

ANALYSIS PROCEDURE: ___ SIMPLIFIED _X EQUIVALENT LATERAL FORCE ___ MODAL LATERAL DESIGN CONTROL: ____ EARTHQUAKE 1.6. ALL DESIGN LOADS ARE PER NORTH CAROLINA STATE BUILDING CODE 2018 EDITION.

FOUNDATION DESIGN CRITERIA:

TOP & BOTTOM

(2-MEMBER)

JACK STUDS

('JAMB') - SEE-

SCHED. FOR QTY.

LATERAL LOAD RESISTANCE.

MINIMUM FOOTING BEARING DEPTH BELOW GRADE IS 12 INCHES. FOUNDATION DESIGN IS BASED ON A PRESUMPTIVE MAXIMUM ALLOWABLE SOIL BEARING CAPACITY OF 1.500 PSF.

WIND LOADS CONTROL THE LATERAL LOAD DESIGN. THE BUILDING UTILIZES SHEAR WALLS FOR

FASTEN w/ #8

-SCREWS @ 24'

SEE SCHED. FOR

FULL HEIGHT 'KING'

-STUDS - SEE SCHED.

QTY. OF STUDS

FOR QTY.

54 MIL. P

ATTACHED w/(4)

#10 SCREWS EACH

(4) #10 SCREWS TO

BOX BEAM HEADER

AT EA. JACK STUD

DN: C=US.

CN=Mark S. Rov

Digitally signed by Mark S. Roy

E=mark.roy@rpaengineering.c

Date: 2025.05.12 13:07:22-04'

RPA Project No.: 2024223

O="RPA Engineering, P.A.",

∽KING STUD, AND

(3-MEMBER)

O.C., TYP.

CONTRACTOR SHALL FIELD VERIFY THE SOIL BEARING CAPACITY PRIOR TO START OF

ARCHITECTURE

114 E. 3rd STREET: GREENVILLE, NC 27858 p:1.252.270.5330 www.INTREPIDarchitecture.com

> S S **>**

IVICE AND AS SUCH SHALL REMAIN THE PROPERTY OF THE ARCHITECT. THEY REPARED FOR A SPECIFIC PROJECT AND SHALL NOT BE USED IN CONJUNCT ANY OTHER PROJECTS WITHOUT PRIOR WRITTEN PERMISSION OF THE ARCH © INTREPID Architecture, PA 2023

DATE

DRAWN BY: RPA/GBP RPA/MSR PROJECT #: 24008 ISSUE DATE: 04.30.2025 PHASE:

REVISIONS:

DESC:

RPA ENGINEERING, P.A. ructural Engineering Solutions Engineering License Certificate No. C-2734 l Commerce Square Suite 202 Washington, NC 27889

CONSTRUCTION DOCUMENTS SHEET NAME & NUMBER

FRAMING SECTIONS & DETAILS Phone: 252-321-6027 Fax: 252-355-2179

Schedule of Inspection and Testing Agencies This Statement of Special Inspections / Quality Assurance Plan includes the following building systems: Spray Fire Resistant Material X Soils and Foundations X Cast-in-Place Concrete X Wood Construction Precast Concrete Exterior Insulation and Finish System X Masonry Mechanical & Electrical Systems Structural Steel Architectural Systems X Cold-Formed Steel Framing Special Cases

Signature

Special Inspection Agencies	Firm	Address, Telephone, e-mail	
Special Inspection Coordinator (SI)	Mark S. Roy, PE RPA Engineering, PA	102 Regency Blvd, Suite A1 Greenville, NC 27834 252-321-6027	
Structural Engineer of Record (SER)	Mark S. Roy, PE RPA Engineering, PA	102 Regency Blvd, Suite A1 Greenville, NC 27834 252-321-6027	
Architect of Record (AR)	DANI HOFF, AIA INTREPID ARCHITECTURE	114 E 3th street, Greenville NC 27858 701-388-0422	
Testing Agency (TA)	TBD	TBD	
5. Testing Agency			
6. Other			

Qualifications of Inspectors and Testing Technicians

The qualifications of all personnel performing Special Inspection and testing activities are subject to the approval of the Building Official. The credentials of all Inspectors and testing technicians shall be provided if requested.

Key for Minimum Qualifications of Inspection Agents:

When the Registered Design Professional in Responsible Charge deems it appropriate that the individual performing a stipulated test or inspection have a specific certification or license as indicated below, such designation shall appear below the Agency Number on the Schedule.

Structural Engineer - a licensed SE or PE specializing in the design of building structures PE/GE Geotechnical Engineer - a licensed PE specializing in soil mechanics and foundations Engineer-In-Training - a graduate engineer who has passed the Fundamentals of

Engineering examination American Concrete Institute (ACI) Certification

Concrete Field Testing Technician - Grade 1 ACI-CCI Concrete Construction Inspector ACI-LTT Laboratory Testing Technician - Grade 1&2 ACI-STT Strength Testing Technician

American Welding Society (AWS) Certification

Certified Welding Inspector AWS/AISC-SSI Certified Structural Steel Inspector

American Society of Non-Destructive Testing (ASNT) Certification

Non-Destructive Testing Technician - Level II or III.

International Code Council (ICC) Certification

ICC-SMSI Structural Masonry Special Inspector ICC-SWSI Structural Steel and Welding Special Inspector **ICC-SFSI** Spray-Applied Fireproofing Special Inspector ICC-PCSI Prestressed Concrete Special Inspector ICC-RCSI Reinforced Concrete Special Inspector

National Institute for Certification in Engineering Technologies (NICET)

NICET-CT Concrete Technician - Levels I, II, III & IV NICET-ST Soils Technician - Levels I, II, III & IV NICET-GET Geotechnical Engineering Technician - Levels I, II, III & IV

Exterior Design Institute (EDI) Certification

EDI-EIFS EIFS Third Party Inspector

Quality Assurance Plan

Quality Assurance for Seismic Resistance

Seismic Design Category Quality Assurance Plan Required (Y/N) Description of seismic force resisting system and designated seismic systems: Steel rigid frame and diagonal bracing

Quality Assurance for Wind Requirements

150 mph Basic Wind Speed (3 second gust) Wind Exposure Category Quality Assurance Plan Required (Y/N)

Description of wind force resisting system and designated wind resisting components: Masonry and wood shear walls

Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated above must submit a Statement of Responsibility.

Special Inspections General Notes

- 1. The inspection and testing agencies shall be engaged by the owner's agent and not by the contractor or sub-contractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the
- Building Official, prior to commencing work. 2. Each Special Inspection Agency shall submit a Final Report of Special Inspections to the Special Inspection Coordinator at the conclusion of the project.

Schedules of Special Inspection Services

	SOIL	S AND FOUNDATIONS
ITEM	QUALIFICATIONS	SCOPE
1. SHALLOW FOUNDATIONS	ТА	INSPECT SOILS BELOW FOOTINGS FOR ADEQUATE BEARING CAPACITY AND CONSISTENCY WITH GEOTECHNICAL REPORT. INSPECT REMOVAL OF UNSUITABLE MATERIAL AND PREPARATION OF SUBGRADE PRIOR TO PLACEMENT OF CONTROLLED FILL.
2. CONTROLLED STRUCTURAL FILL	TA	PERFORM SIEVE TESTS (ASTM D422 & D1140) AND MODIFIED PROCTOR TESTS (ASTM D1557) OF EACH SOURCE OF FILL MATERIAL. INSPECT PLACEMENT, LIFT THICKNESS AND COMPACTION OF CONTROLLED FILL. TEST DENSITY OF EACH LIFT OF FILL BY NUCLEAR METHODS (ASTM D2922) OR MODIFIED PROCTOR TESTS (ASTM 101557) VERIFY EXTENT AND SLOPE OF FILL PLACEMENT.
3. DEEP FOUNDATIONS	ТА	N/A
4. LOAD TESTING		NOT APPLICABLE
5. OTHER		

CAST-IN-PLACE CONCRETE		
ITEM	QUALIFICATIONS	SCOPE
1. DESIGN MIX	SER	REVIEW MIX SUBMITTAL
2. MATERIAL CERTIFICATION	SER	REVIEW CERTIFICATES OF COMPLIANCE FOR CONCRETE MATERIALS, REINFORCING, ADMIXTURES, AND ACCESSORIES.
3. REINFORCEMENT INSTALLATION	SER/TA	PERFORM FIELD INSPECTION OF REINFORCEMENT PLACEMENT.
4. ANCHOR RODS	SER/TA	INSPECT SIZE, POSITIONING AND EMBEDMENT OF ANCHOR RODS.
5. CONCRETE PLACEMENT	TA (ACI-CCI ICC-RCSI)	INSPECT PLACEMENT OF CONCRETE. VERIFY THAT CONCRETE CONVEYANCE AND DEPOSITING AVOIDS SEGREGATION OR CONTAMINATION. VERIFY THAT CONCRETE IS PROPERLY CONSOLIDATED.
6. SAMPLING AND TESTING OF CONCRETE	TA (ACI-CFTT ACI-STT)	TEST CONCRETE COMPRESSIVE STRENGTH (ASTM C31 & C39), SLUMP (ASTM C143), AIR-CONTENT (ASTM C231 OR C173) AND TEMPERATURE (ASTM C1064).
7. CURING AND PROTECTION	TA (ACI-CCI ICC-RCSI)	INSPECT CURING, COLD WEATHER PROTECTION AND HOT WEATHER PROTECTION PROCEDURES.
8. FABRICATION DRAWINGS	SER	REVIEW REINFORCING STEEL FABRICATION DRAWINGS FOR COMPLIANCE WITH DESIGN DRAWINGS.
9. OTHER		

COLD-FORMED STEEL FRAMING		
ITEM	QUALIFICATIONS	SCOPE
1. MEMBER SIZE	SER	PERIODIC FIELD REVIEW.
2. MATERIAL THICKNESS	SER	PERIODIC FIELD REVIEW.
3. MATERIAL PROPERTIES	SER	REVIEW CERTIFICATE OF COMPLIANCE.
4. MECHANICAL CONNECTIONS	TA	PERIODIC FIELD REVIEW OF CONNECTIONS.
5. WELDING	TA	PERIODIC VISUAL INSPECTION OF FIELD WELDS.
6. FRAMING DETAILS	SER/TA	PERIODIC FIELD REVIEW.
7. OTHER		

REINFORCED CONCRETE MASONRY		
ITEM	QUALIFICATIONS	SCOPE REQUIRED INSPECTION LEVEL: X 1 _ 2
1. MATERIAL CERTIFICATION	SER	REVIEW CERTIFICATES OF COMPLIANCE FOR CONCRETE MASONRY UNITS, MORTAR, GROUT, REINFORCING, AND ACCESSORIES.
2. MIXING OF MORTAR AND GROUT	TA (ICC-SMSI)	PERIODIC INSPECTION OF PROPORTIONING, MIXING AND RETEMPERING OF MORTAR AND GROUT.
3. INSTALLATION OF MASONRY	SER/TA (ICC-SMSI)	PERIODIC INSPECTION OF SIZE, LAYOUT, BONDING AND PLACEMENT OF MASONRY UNITS.
4. REINFORCEMENT INSTALLATION	SER/TA (ICC-SMSI)	PERIODIC INSPECTION OF PLACEMENT, POSITIONING AND LAPPING OF REINFORCING STEEL.
5. GROUTING OPERATIONS	TA (ICC-SMSI)	PERIODIC INSPECTION OF PLACEMENT AND CONSOLIDATION OF GROUT. INSPECT MASONRY CLEAN-OUTS FOR HIGH-LIFT GROUTING.
6. WEATHER PROTECTION	TA (ICC-SMSI)	PERIODIC INSPECTION OF COLD WEATHER PROTECTION AND HOT WEATHER PROTECTION PROCEDURES. VERIFY THAT WALL CAVITIES ARE PROTECTED AGAINST PRECIPITATION.
7. EVALUATION OF MASONRY STRENGTH	TA (ICC-SMSI)	TEST COMPRESSIVE STRENGTH OF MORTAR AND GROUT CUBE SAMPLES (ASTM C780).
8. ANCHORS AND TIES	SER/TA (ICC-SMSI)	PERIODIC INSPECTION OF SIZE, LOCATION, SPACING AND EMBEDMENT OF DOWELS, ANCHORS AND TIES.
9. OTHER		

STRUCTURAL STEEL		
ITEM	QUALIFICATIONS	SCOPE
FABRICATOR CERTIFICATION / QUALITY CONTROL PROCEDURES	SER	REVIEW SHOP FABRICATION AND QUALITY CONTROL PROCEDURES. FABRICATOR SHALL SUBMIT CERTIFICATE OF COMPLIANCE.
2. MATERIAL CERTIFICATION	SER	REVIEW CERTIFIED MILL TEST REPORTS FOR STRUCTURAL STEEL; REVIEW CERTIFICATES OF COMPLIANCE FOR BOLTS, NUTS, AND WASHERS; REVIEW CERTIFICATES OF COMPLIANCE FOR WELD FILLER MATERIAL.
3. BOLTING	SER/TA	VISUALLY INSPECT HIGH-STRENGTH BOLTED CONNECTIONS. VERIFY SNUGTIGHT BOLTED CONNECTIONS HAVE ALL CONNECTED PARTS DRAWN TOGETHER.
4. WELDING	TA (AWS-CWI)	VISUALLY INSPECT ALL FIELD WELDS PER AWS D1.1 OR AWS D1.3 AS APPLICABLE. PROVIDE PERIODIC INSPECTION OF SINGLE PASS FILLET WELDS.
	(ASNT)	PROVIDE CONTINUOUS INSPECTION AND 100% TESTING OF ALL PARTIAL AND FULL PENETRATION GROOVE WELDS. VERIFY SIZE AND LENGTH OF FILLET WELDS.
5. SHEAR CONNECTIONS	TA (AWS/AISC-SSI) (ICC-SWSI)	INSPECT SIZE, NUMBER, POSITIONING AND WELDING OF SHEAR CONNECTORS. INSPECT SUDS FOR FULL 360 DEGREE FLASH. RING TEST ALL SHEAR CONNECTORS WITH A 3 LB HAMMER. BEND TEST ALL QUESTIONABLE STUDS TO 15 DEGREES.
6. STRUCTURAL DETAILS	SER	INSPECT STEEL FRAME FOR COMPLIANCE WITH STRUCTURAL DRAWINGS, INCLUDING BRACING, MEMBER CONFIGURATION AND CONNECTION DETAILS.
7. METAL DECK	TA (AWS-CWI)	INSPECT WELDING AND SIDE-LAP FASTENING OF METAL ROOF AND FLOOR DECK.
8. FABRICATION DRAWINGS	SER	REVIEW STRUCTURAL STEEL FABRICATION DRAWINGS FOR COMPLIANCE WITH DESIGN DRAWINGS.
9. OTHER		

WOOD CONSTRUCTION		
ITEM	QUALIFICATIONS	SCOPE
FABRICATOR CERTIFICATION / QUALITY CONTROL PROCEDURES	SER	REVIEW SHOP FABRICATION AND QUALITY CONTROL PROCEDURES. FABRICATOR SHALL SUBMIT CERTIFICATE OF COMPLIANCE.
2. MATERIAL GRADING	SER	VERIFY SPECIES AND GRADE OF WOOD FRAMING MEMBERS.
3. CONNECTIONS	SER	REVIEW SUBMITTALS FOR BOLTS AND OTHER CONNECTORS.
4. DIAPHRAGMS AND SHEARWALLS	SER	INSPECT SIZE, CONFIGURATION, BLOCKING AND FASTENING OF SHEARWALLS AND DIAPHRAGMS. VERIFY PANEL GRADE AND THICKNESS.
5. PREFABRICATED WOOD TRUSSES	SER	REVIEW WOOD TRUSS FABRICATION/ERECTION DRAWINGS.
6. PERMANENT TRUSS BRACING	SER	REVIEW PERMANENT TRUSS BRACING FOR COMPLIANCE WITH TRUSS FABRICATION/ERECTION DRAWINGS.
7. OTHER		



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STATION

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