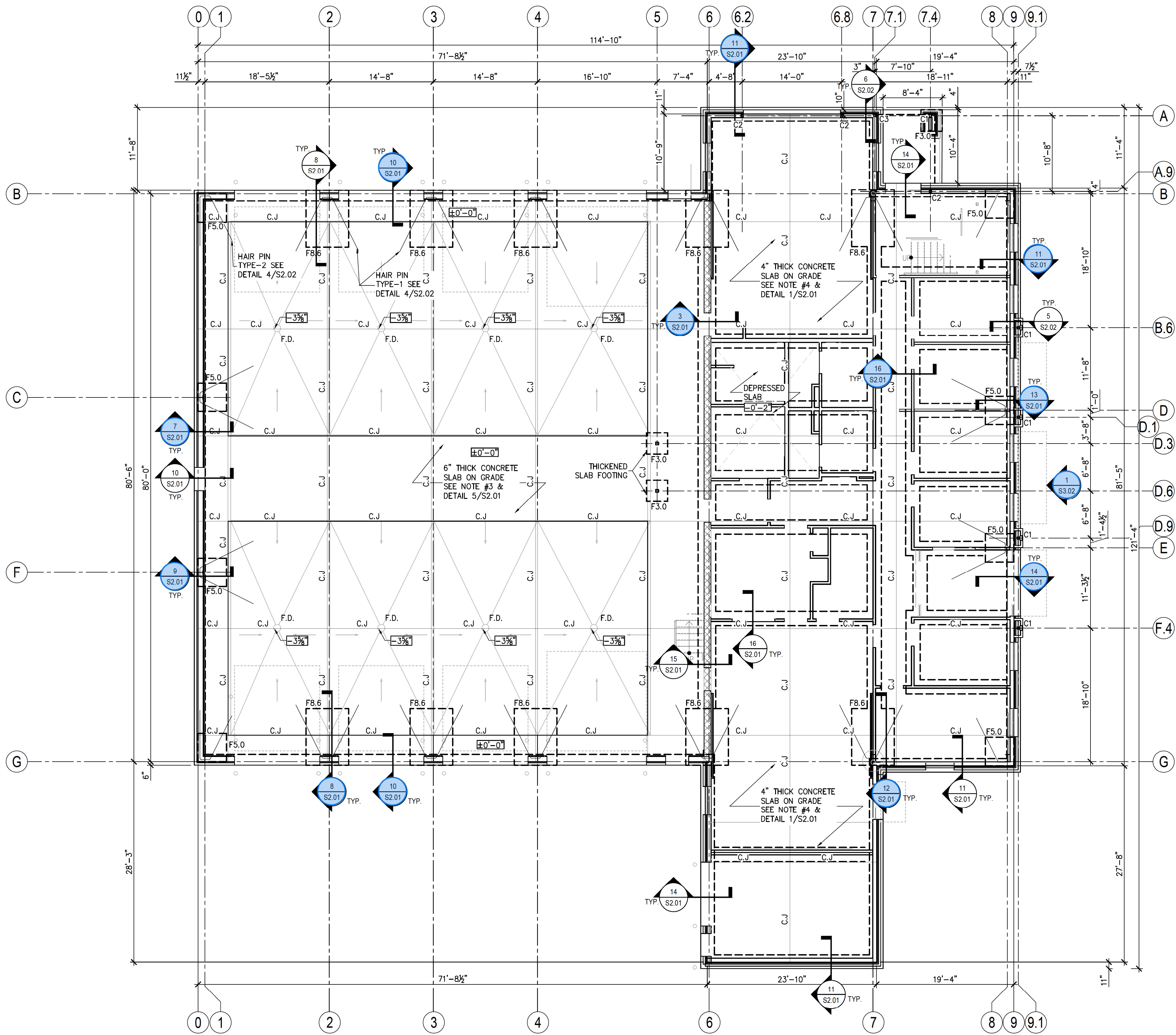




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### FOUNDATION PLAN

1/8" = 1'-0"

- FOUNDATION PLAN NOTES:**
- SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL STRUCTURAL NOTES & SCHEDULES.
  - TOP OF SLAB REFERENCE ELEVATION = 0'-0" UNLESS OTHERWISE NOTED. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR ACTUAL SITE ELEVATIONS.
  - CONCRETE FLOOR SLAB IS 6" THICK WITH 6 x 6 - W 2.9x W 2.9 WELDED WIRE FABRIC, PROVIDE 10 MIL VAPOR BARRIER AND 6" COMPACTED GRANULAR BASE UNDER SLAB. SEE DETAIL 5/S2.01
  - CONCRETE FLOOR SLAB IS 4" THICK WITH 6 x 6 - W 2.1x W 2.1 WELDED WIRE FABRIC, PROVIDE 10 MIL VAPOR BARRIER AND 4" COMPACTED GRANULAR BASE UNDER SLAB. SEE DETAIL 1/S2.01
  - SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY CONTROL JOINTS.
  - SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MASONRY OPENINGS NOT SHOWN.
  - FOR FOOTING, BASE PLATE AND ANCHOR BOLT DETAIL, SEE SCHEDULE ON SHEET S1.01, AND DETAIL ON S2.01, S2.02
  - SEE 4/S2.01 FOR SLAB CORNER REINFORCEMENT. U.N.O.

FOUNDATION PLAN LEGEND	
F4.0	DENOTES COLUMN CONCRETE SPREAD FOOTING WITH FOOTING MARK - SEE FOOTING SCHEDULE ON S1.01 FOR SIZE AND REINFORCING
C.J.	DENOTES SLAB ON GRADE CONSTRUCTION OR SAWCUT CONTROL JOINT - SEE DETAILS 3/S2.01 AND 2/S2.01 FOR ADDITIONAL INFORMATION
U.O.N.	DENOTES 'UNLESS OTHERWISE NOTED'
PEMB	DENOTES 'PRE-ENGINEERED METAL BUILDING'

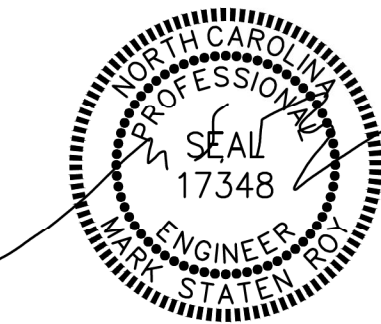
FOOTING SCHEDULE			
MARK	FTG. SIZE	REINFORCEMENT	REMARKS
F3.0	3'-0" x 3'-0" x 2'-0"	4 - #4 EACH WAY, TOP. 4 - #6 EACH WAY, BOTT.	-
F4.0	4'-0" x 4'-0" x 2'-0"	5 - #4 EACH WAY, TOP. 5 - #6 EACH WAY, BOTT.	-
F5.0	5'-0" x 5'-0" x 2'-0"	6 - #4 EACH WAY, TOP. 6 - #6 EACH WAY, BOTT.	-
F8.6	8'-0" x 6'-0" x 2'-0"	5 - #4 EACH L.W. TOP. 9 - #4 EACH S.W. TOP. 5 - #6 EACH L.W. BOTT. 9 - #6 EACH S.W. BOTT.	-

NOTE:  
1. S.W.=> SHORT WAY. 2. L.W.=> LONG WAY. 3. E.W.=> EACH WAY

COLUMN SCHEDULE				
MARK	COL. SIZE	BASE $\varnothing$ SIZE & TYPE	ANCH. BOLT DIA.	REMARKS
C1	HSS 4 x 4 x $\frac{1}{4}$	$\frac{3}{4}$ " x 10" x 0'-10" - B1	$\frac{3}{4}$ "	4 BOLTS
C2	HSS 4 x 4 x $\frac{1}{4}$	$\frac{3}{4}$ " x 5" x 0'-10" - B2	$\frac{3}{4}$ "	2 BOLTS
C3	HSS 4 x 4 x $\frac{1}{4}$	$\frac{3}{4}$ " x 7 $\frac{1}{2}$ " x 7 $\frac{1}{2}$ " - B3	$\frac{3}{4}$ "	2 BOLTS

NOTE:  
1. SEE DETAIL 7/S2.02 FOR ANCHOR BOLTS DETAILS & BASE PLATE TYPE.

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**FOUNDATION PLAN**

**S1.01**





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MEZZANINE FLOOR FRAMING PLAN

1/8" = 1'-0"

- MEZZANINE FLOOR FRAMING PLAN NOTES:
- SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL STRUCTURAL NOTES AND SCHEDULES.
  - ALL BUILDING DIMENSIONS ARE FROM FACE TO FACE OF STUD WALLS, U.N.O.
  - SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS, WALL OPENINGS, ETC.
  - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
  - FINISHED FLOOR ELEVATION IS +11'-4". ABOVE GROUND LEVEL SLAB. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR ACTUAL FINISHED FLOOR ELEVATION.
  - PROVIDE DOUBLE JOIST UNDER ALL WALLS PARALLEL TO FLOOR TRUSS/JOIST. U.N.O.
  - PROVIDE (2)HEADER AND (2)TRIMMER FOR ALL OPENING BIGGER THEN 2'-0"x2'-0". U.N.O.
  - SEE DETAIL 1 & 2/S2.01 FOR MASONRY CONSTRUCTION DETAILS.
  - ATTACH LVL BEAM TO CMU WALL WITH 'HGM' SIMPSON HEAVY DUTY FACE MOUNT JOIST HANGER. FILL BLOCK CORE SOLID AT HANGER LOCATION. U.N.O.

MEZZANINE FLOOR FRAMING PLAN LEGEND	
Ⓐ	DENOTES (4) 6"-16GA STUD PACK
Ⓑ	DENOTES (3) 6"-16GA STUD PACK
Ⓒ	DENOTES (3) 3/8"-16GA STUD PACK
Ⓓ	DENOTES (5) 6"-16GA STUD PACK
U.N.O.	DENOTES 'UNLESS NOTED OTHERWISE'

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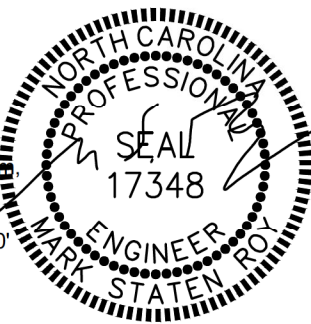
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SHEET NAME & NUMBER  
MEZZANINE FLOOR  
FRAMING PLAN

S1.02

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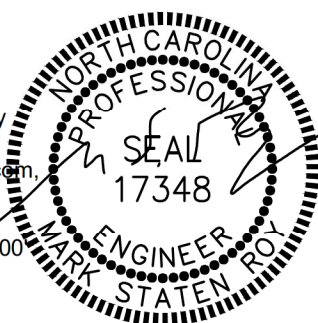
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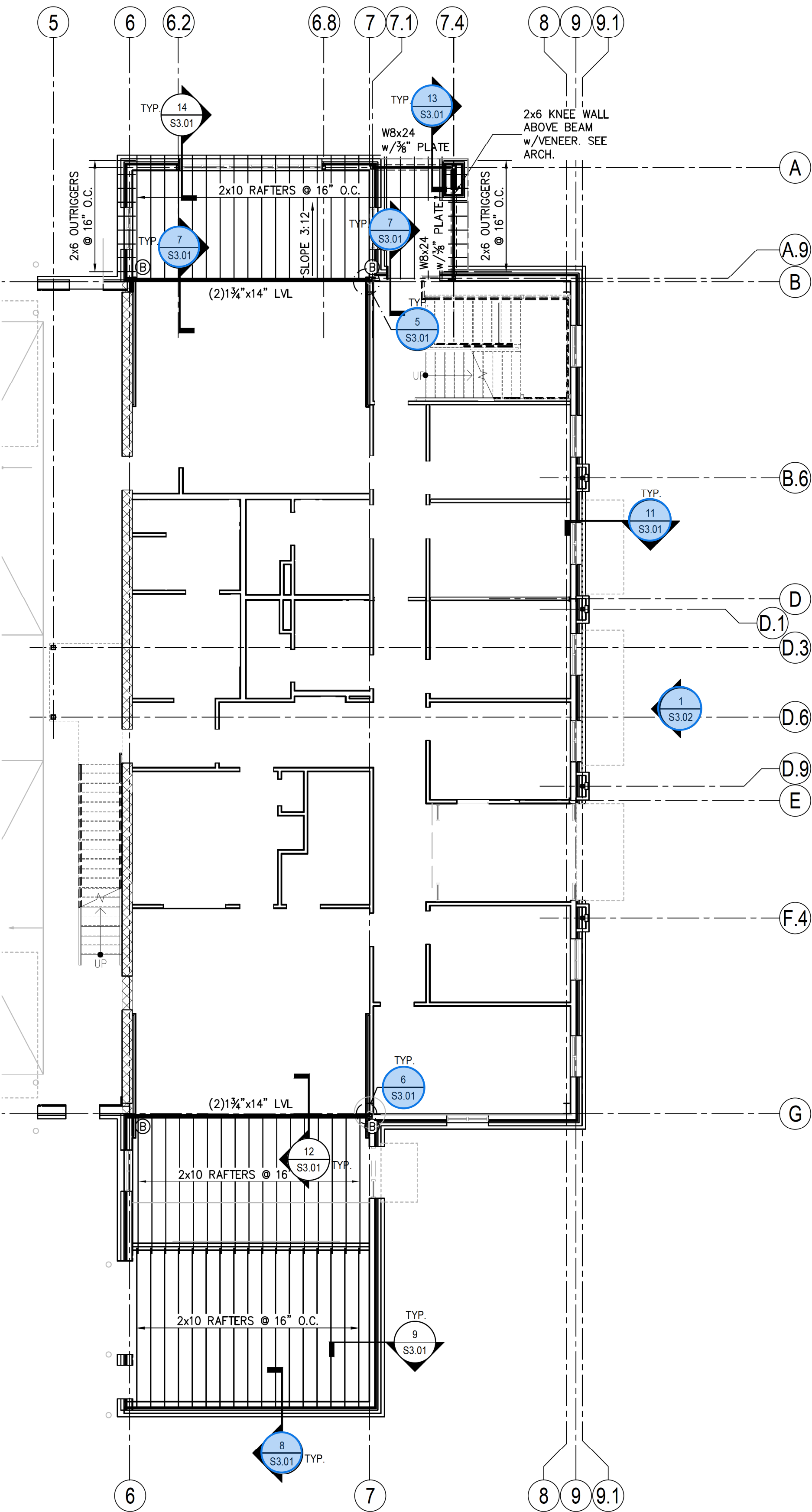
SHEET NAME & NUMBER  
SECOND FLOOR CEILING  
FRAMING AND ROOF  
FRAMING PLAN

S1.03

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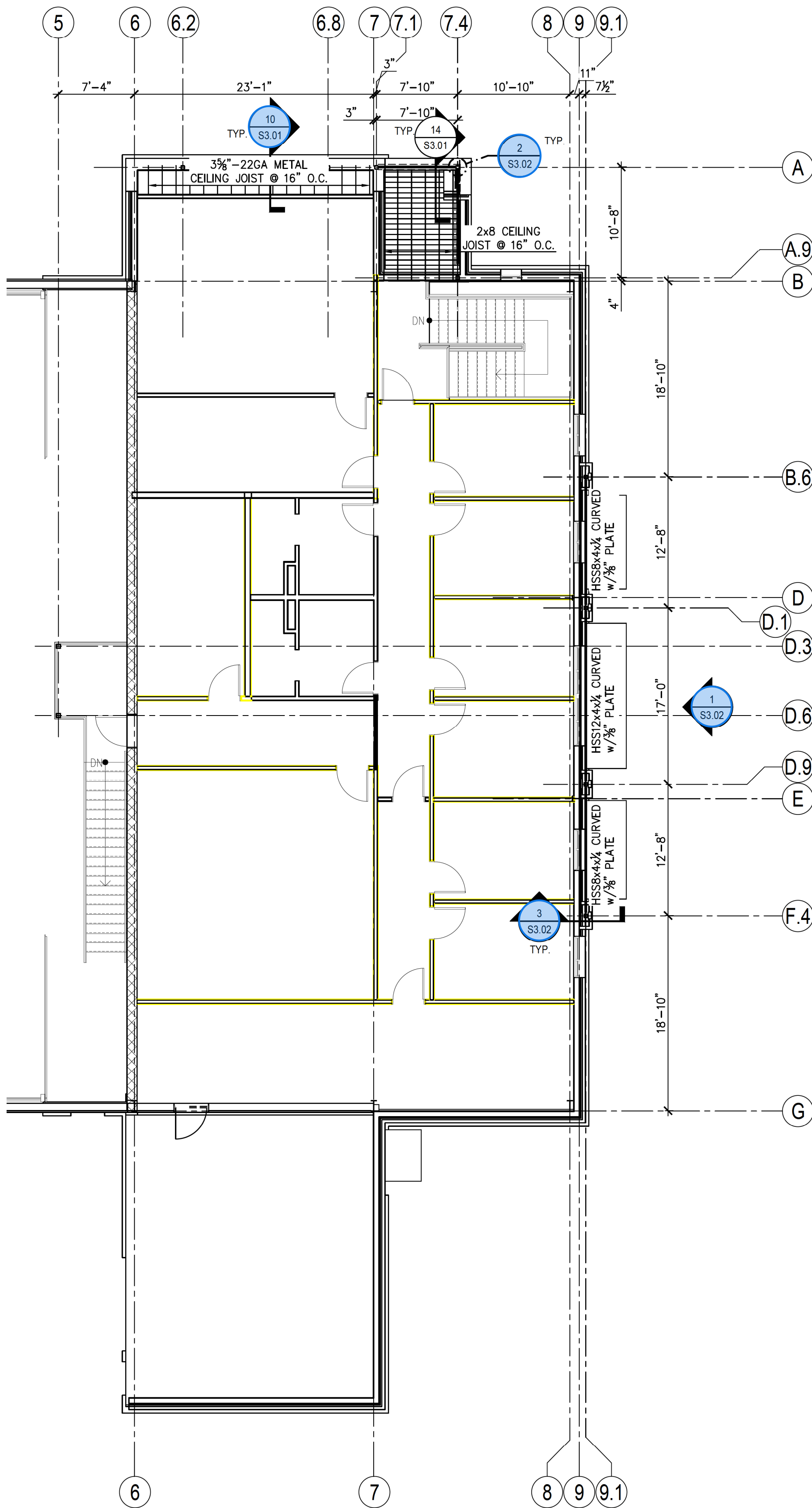
### LOW ROOF & ROOF FRAMING PLAN

1/8" = 1'-0"

#### ROOF FRAMING PLAN NOTES:

1. SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL STRUCTURAL NOTES AND SCHEDULES.
2. ALL BUILDING DIMENSIONS ARE FROM FACE TO FACE OF STUD WALLS, U.N.O.
3. SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS, WALL OPENINGS, ETC.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
5. PROVIDE DOUBLE JOIST UNDER ALL WALLS PARALLEL TO FLOOR TRUSS/JOIST. U.N.O.
6. PROVIDE (2)HEADER AND (2)TRIMMER FOR ALL OPENING BIGGER THEN 2'-0"x2'-0". U.N.O.

ROOF FRAMING PLAN LEGEND	
A	DENOTES (4) 6"-16GA STUD PACK
B	DENOTES (3) 6"-16GA STUD PACK
C	DENOTES (3) 3/8"-16GA STUD PACK
U.N.O.	DENOTES 'UNLESS NOTED OTHERWISE'



### SECOND FLOOR CEILING FRAMING PLAN

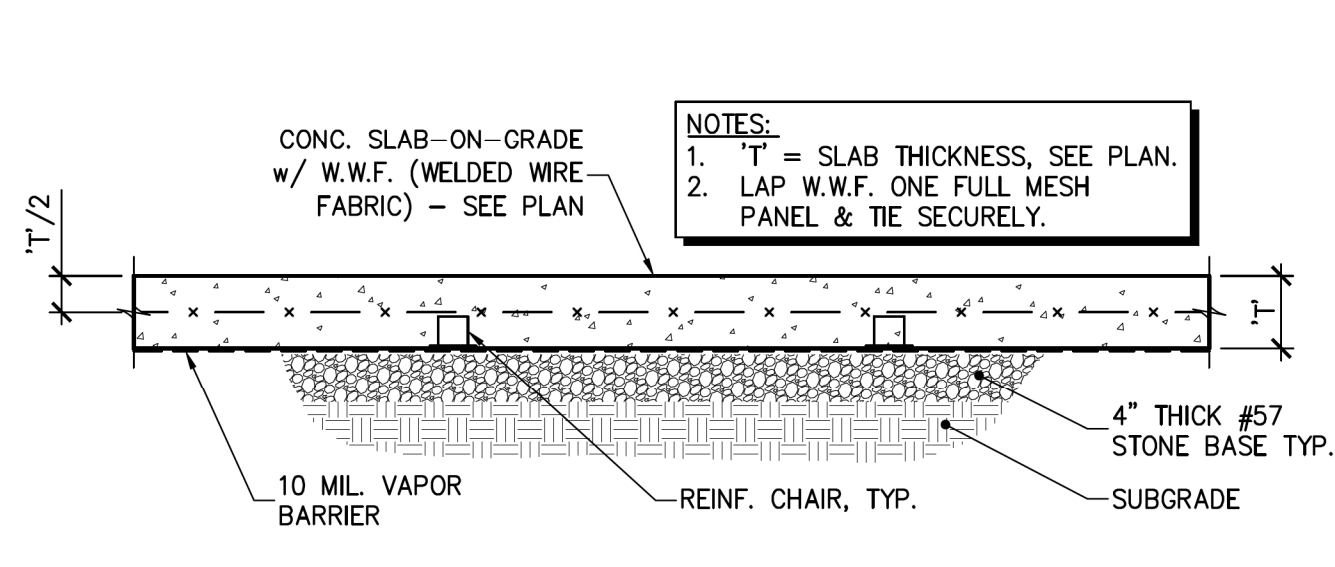
1/8" = 1'-0"

#### SECOND FLOOR CEILING FRAMING PLAN NOTES:

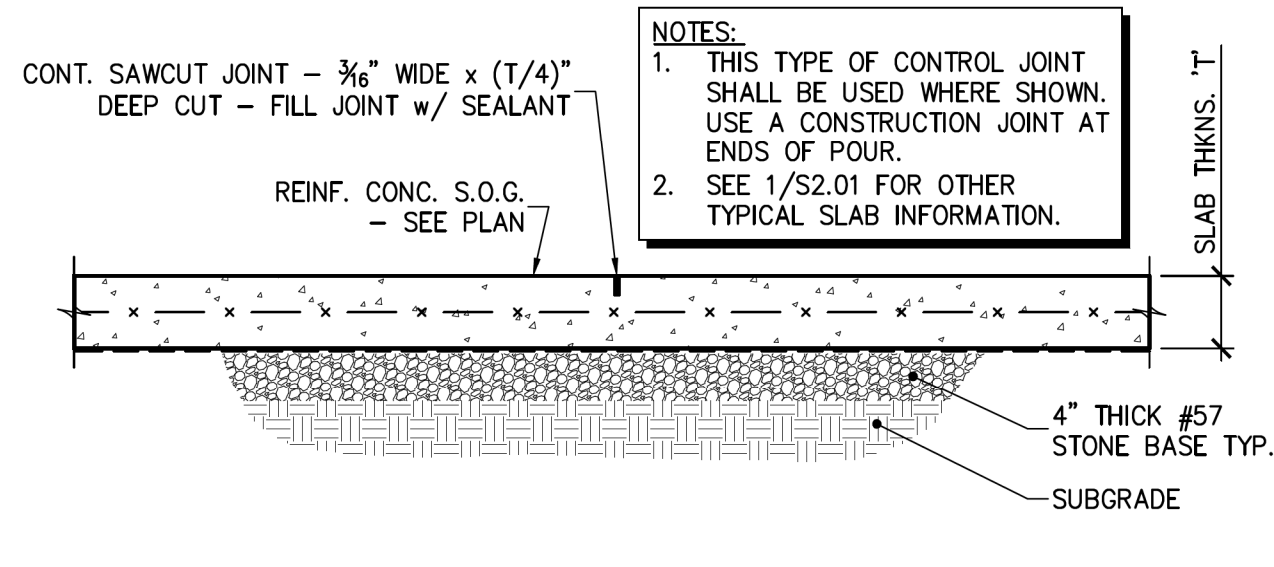
1. SEE SHEET S4.01 FOR DESIGN CRITERIA, GENERAL STRUCTURAL NOTES AND SCHEDULES.
2. ALL BUILDING DIMENSIONS ARE FROM FACE TO FACE OF STUD WALLS, U.N.O.
3. SEE ARCH DWGS FOR ADDITIONAL DIMENSIONS, WALL OPENINGS, ETC.
4. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
5. PROVIDE (2)HEADER AND (2)TRIMMER FOR ALL OPENING BIGGER THEN 2'-0"x2'-0". U.N.O.

ROOF FRAMING PLAN LEGEND	
A	DENOTES (4) 6"-16GA STUD PACK
B	DENOTES (3) 6"-16GA STUD PACK
C	DENOTES (3) 3/8"-16GA STUD PACK
U.N.O.	DENOTES 'UNLESS NOTED OTHERWISE'

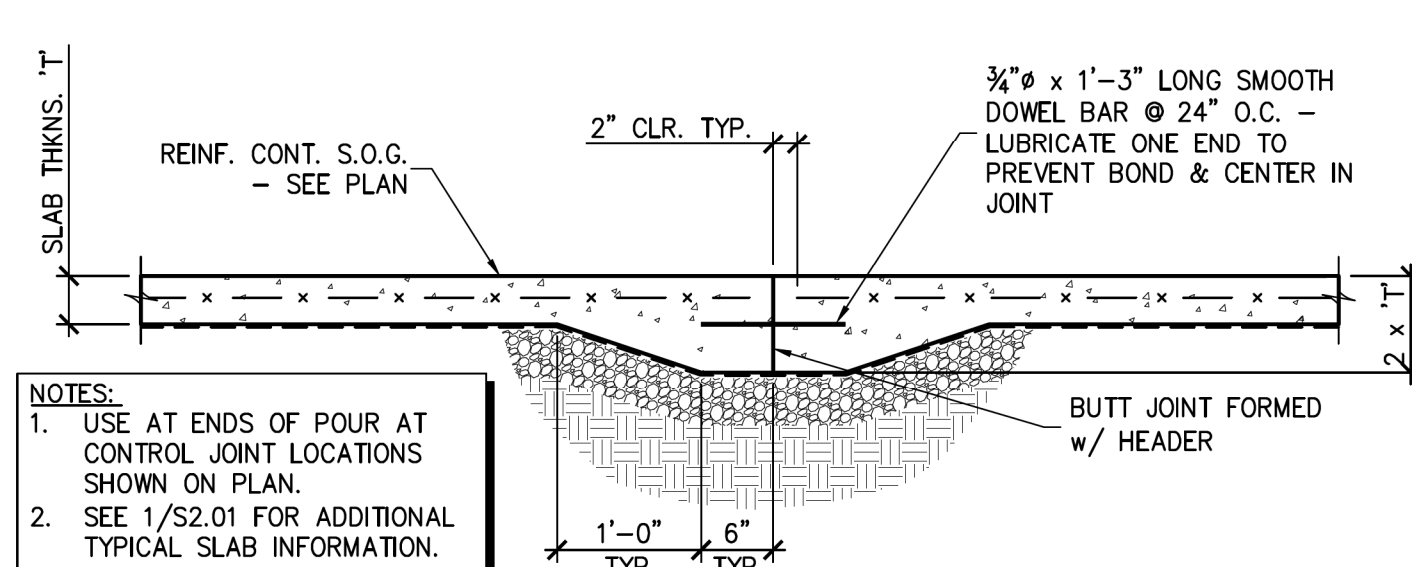




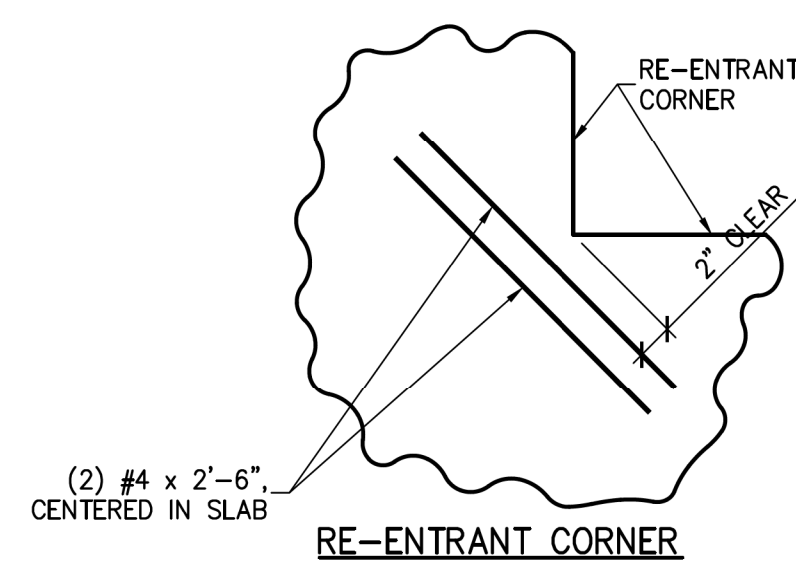
1 SECTION - TYP. SLAB ON GRADE  
S2.01/3/4" = 1'-0"



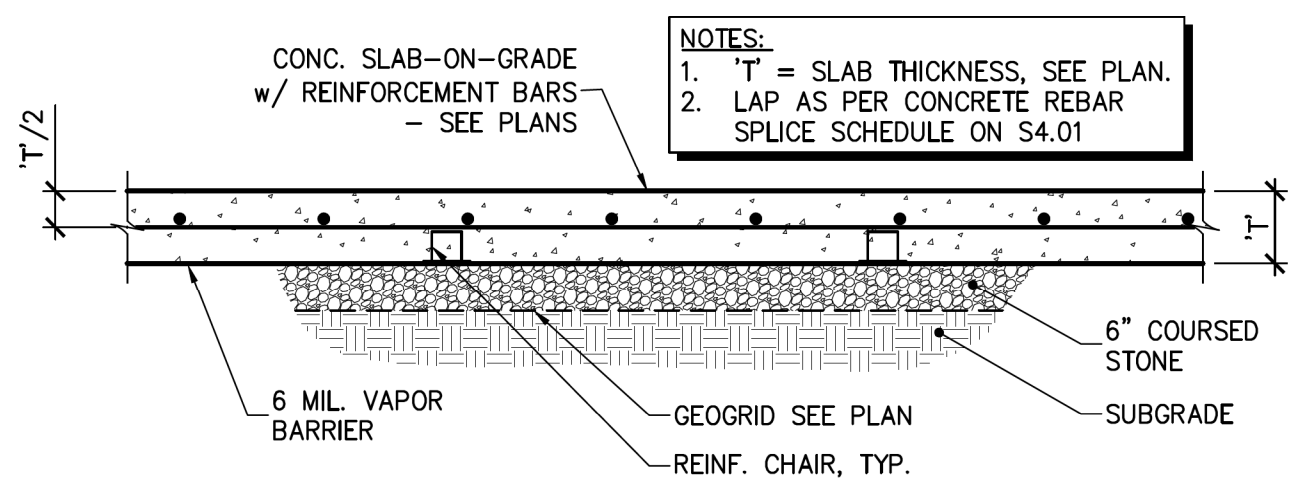
2 SECTION - TYP. CONTROL JOINT  
S2.01/3/4" = 1'-0"



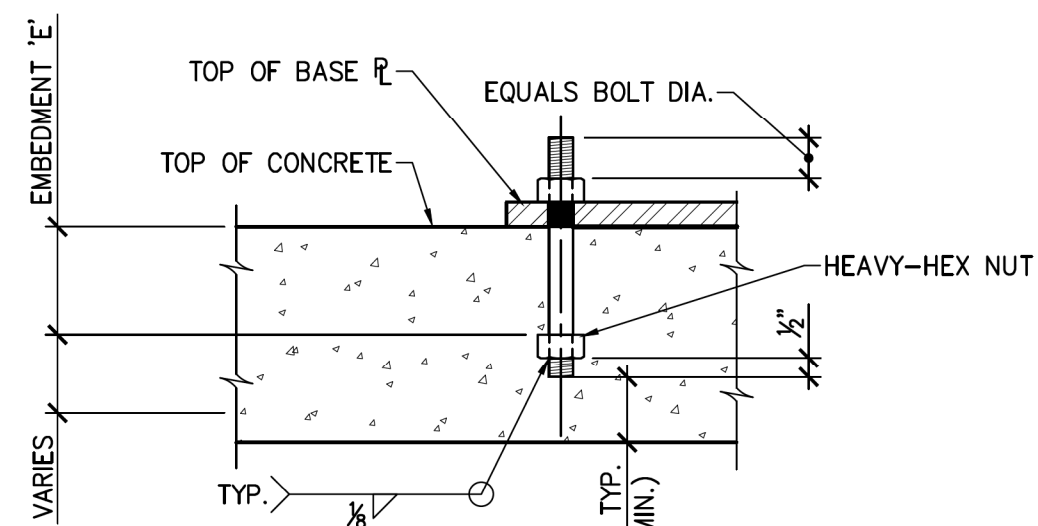
3 SECTION - TYP. CONSTRUCTION JOINT  
S2.01/3/4" = 1'-0"



4 DETAIL - TYP. REINF. @ SLAB CORNERS  
S2.01/3/4" = 1'-0"

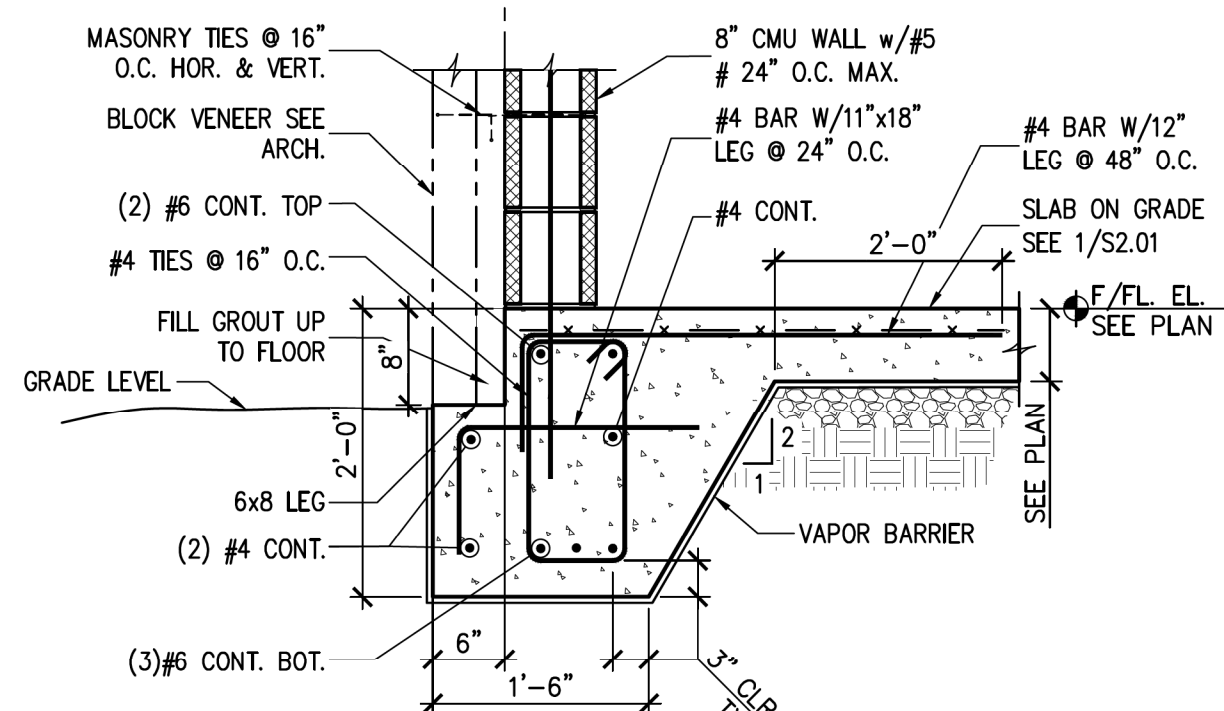


5 SECTION - TYP. SLAB ON GRADE  
S2.01/3/4" = 1'-0"

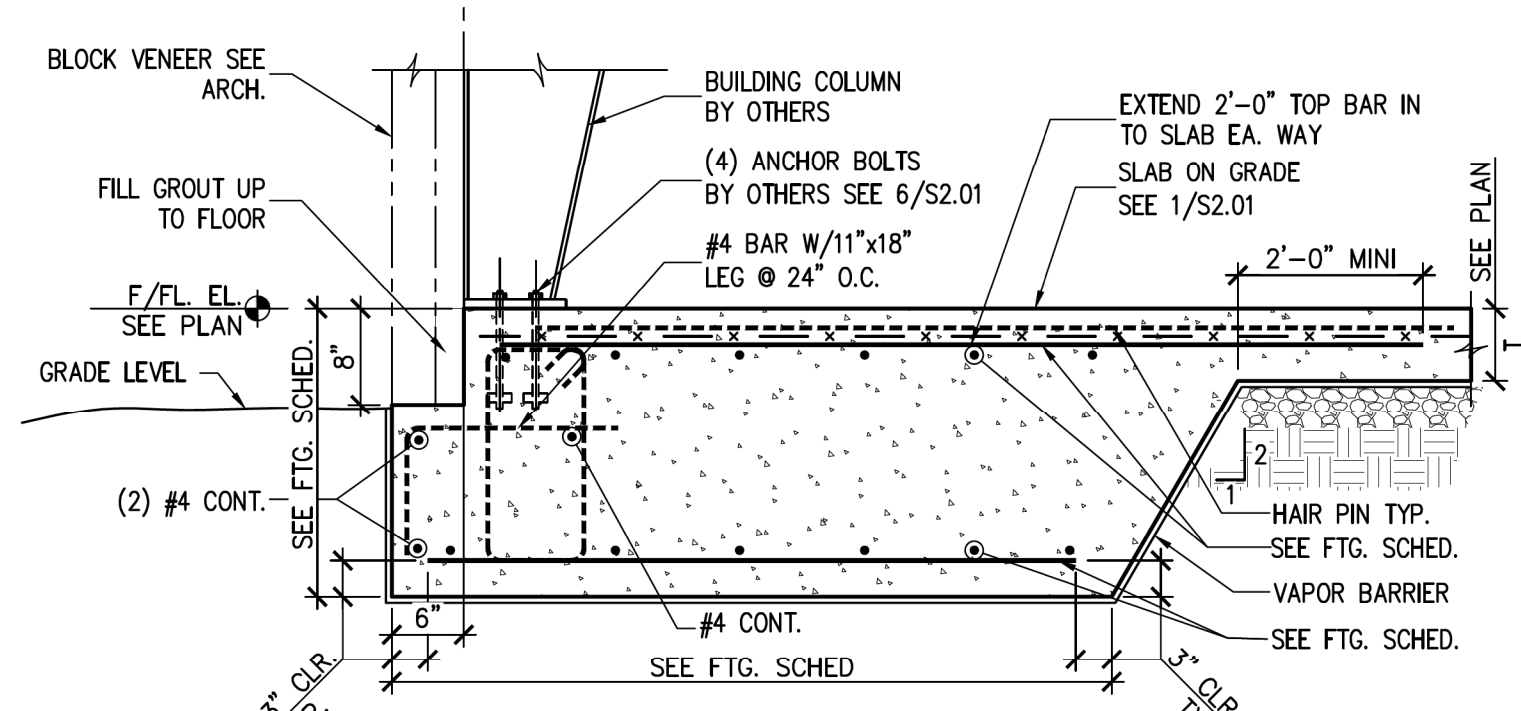


BOLT DIA. 'D'	EMBEDMENT 'E'	REMARKS
3/8"	0'-7 1/2"	-
3/4"	0'-9"	-
1"	1'-0"	-

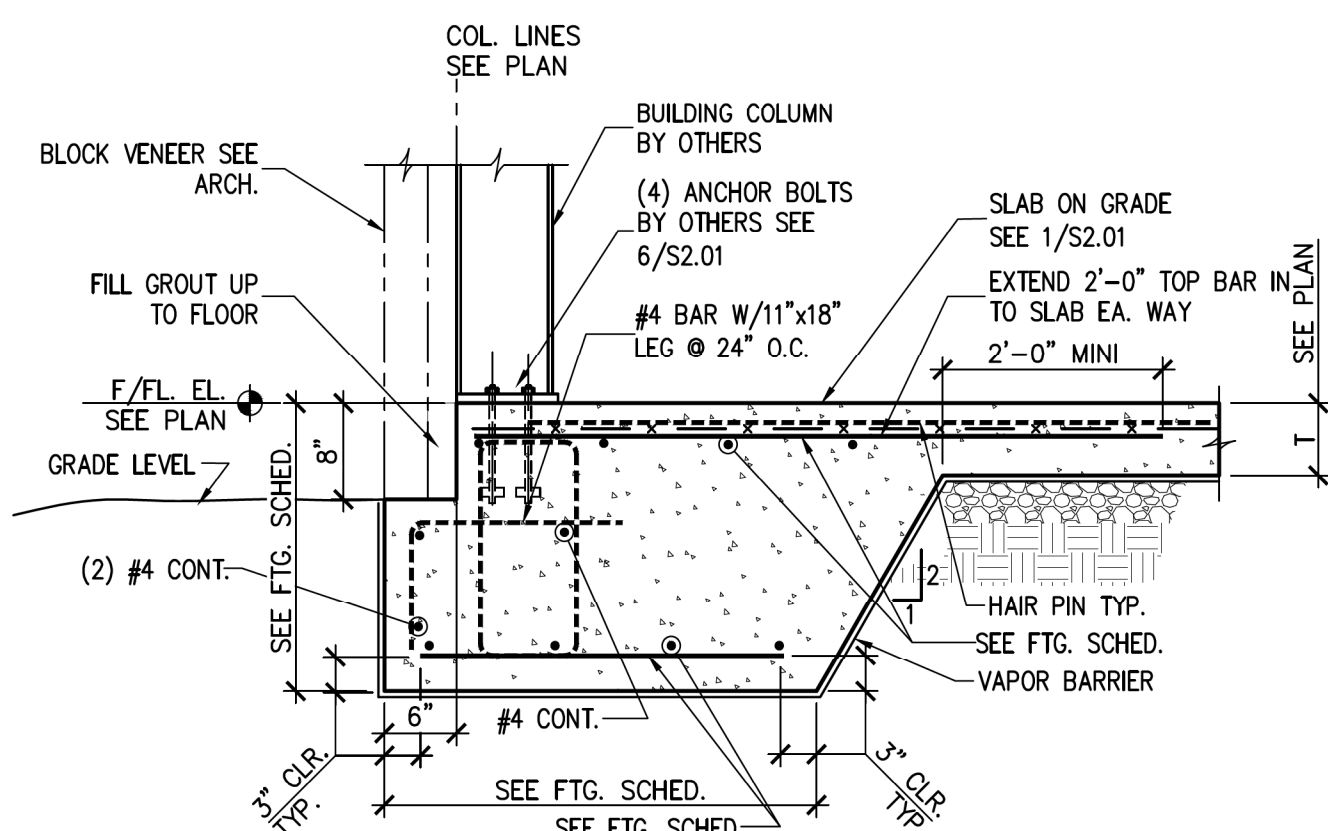
6 TYPICAL PEMB ANCHOR BOLT DETAL  
S2.01/3/4" = 1'-0"



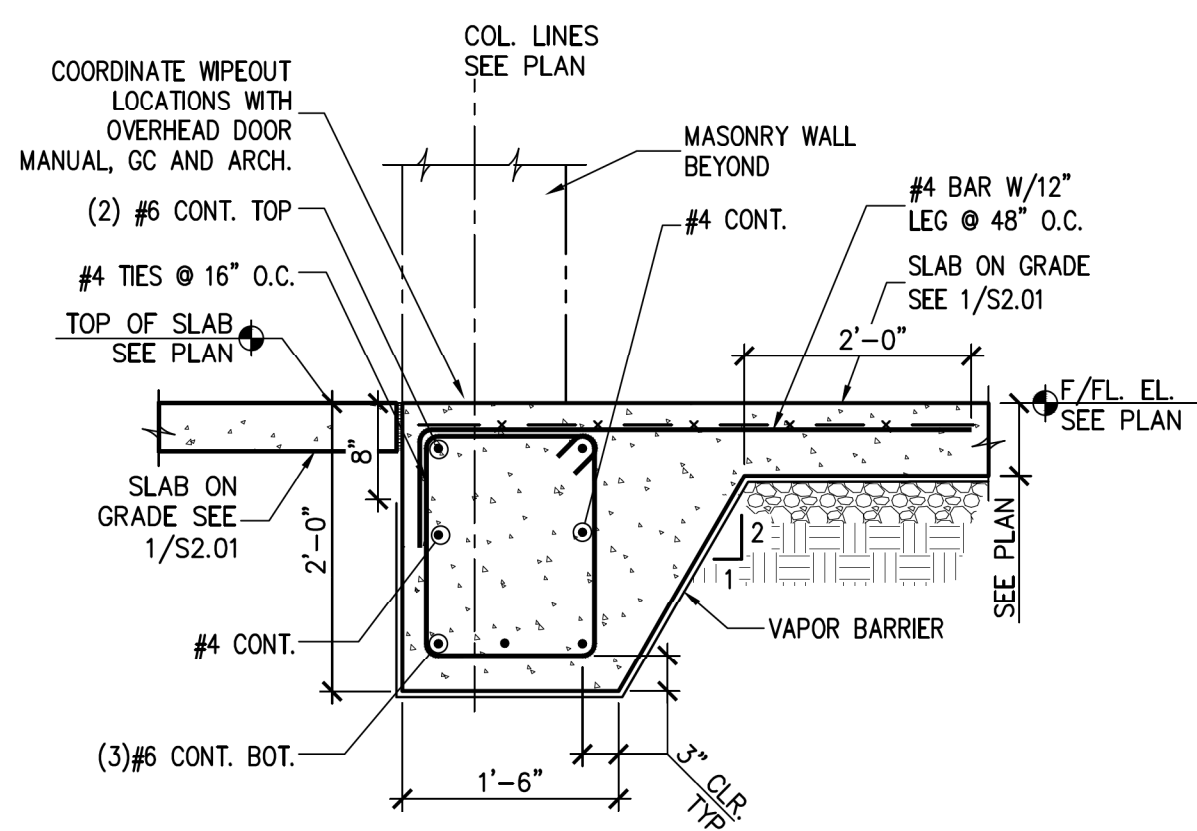
7 SECTION - TYP. GRADE BEAM  
S2.01/3/4" = 1'-0"



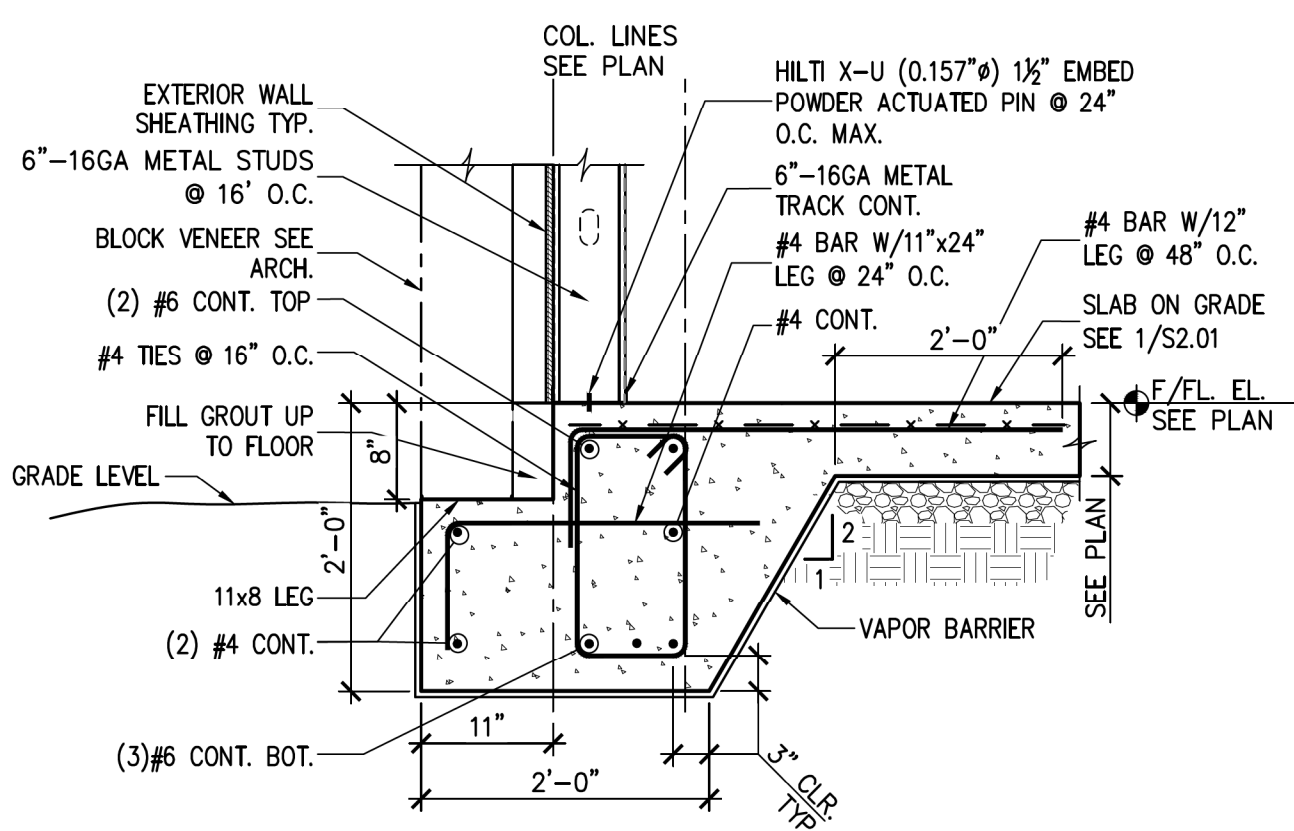
8 SECTION - TYP. COLUMN FOOTING  
S2.01/3/4" = 1'-0"



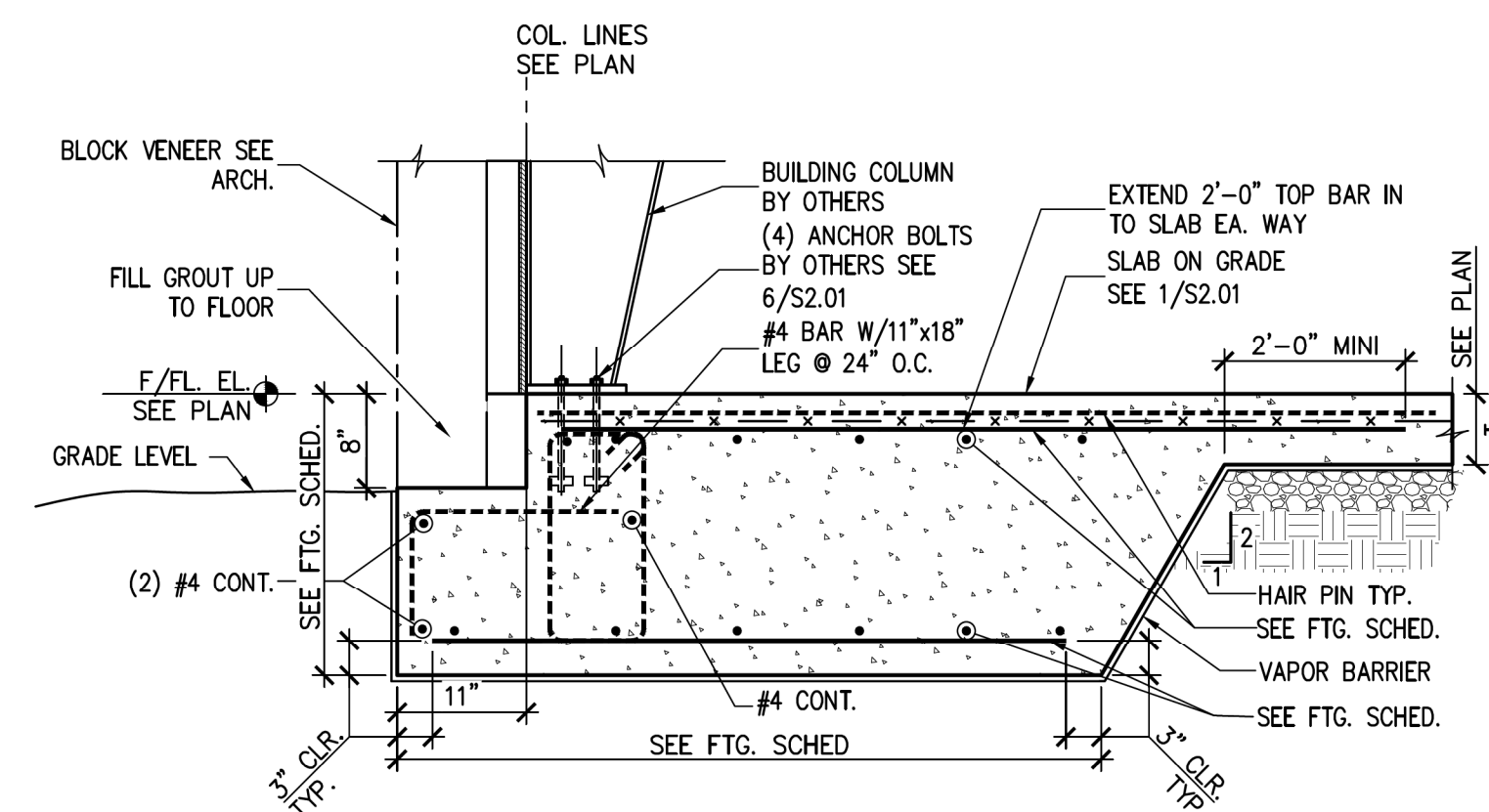
9 SECTION - TYP. WIND POST  
S2.01/3/4" = 1'-0"



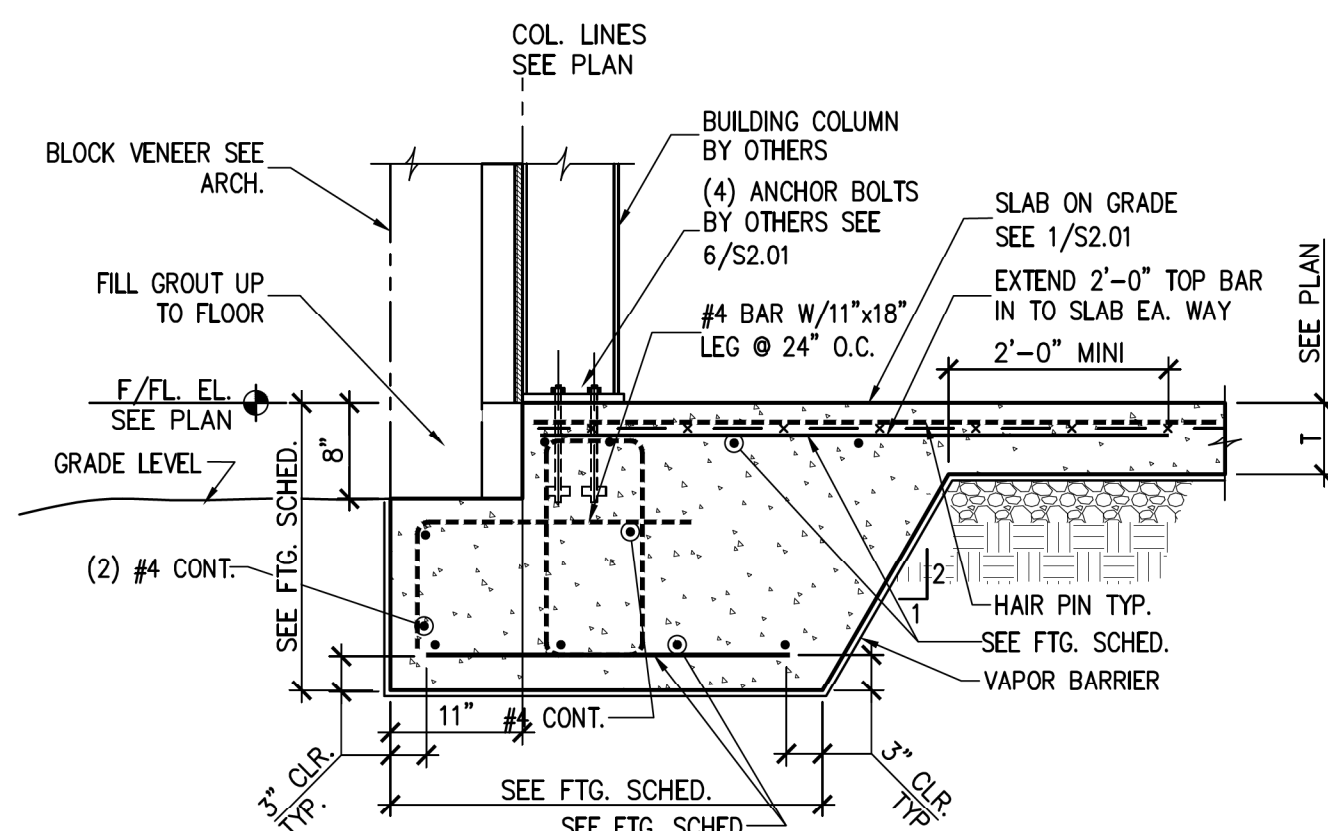
10 SECTION - TYP. WALL OPENING  
S2.01/3/4" = 1'-0"



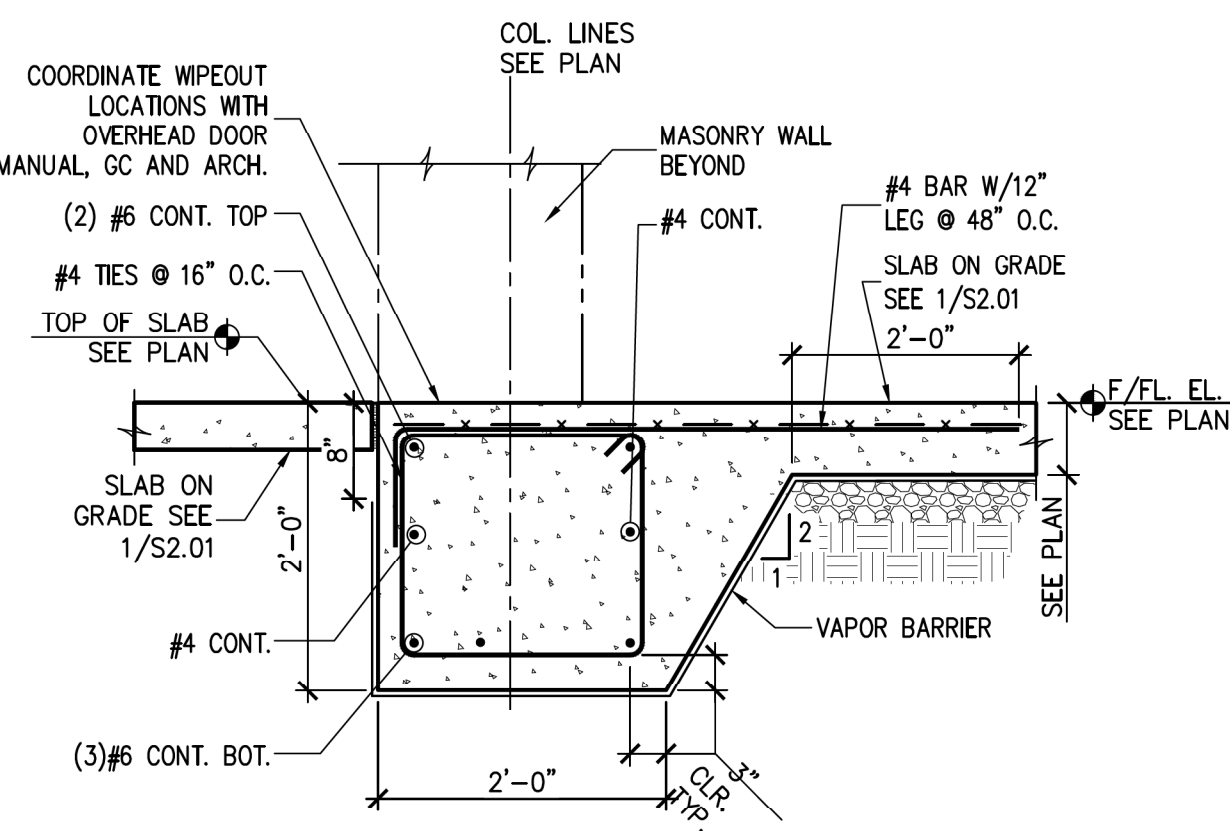
11 SECTION - TYP. GRADE BEAM  
S2.01/3/4" = 1'-0"



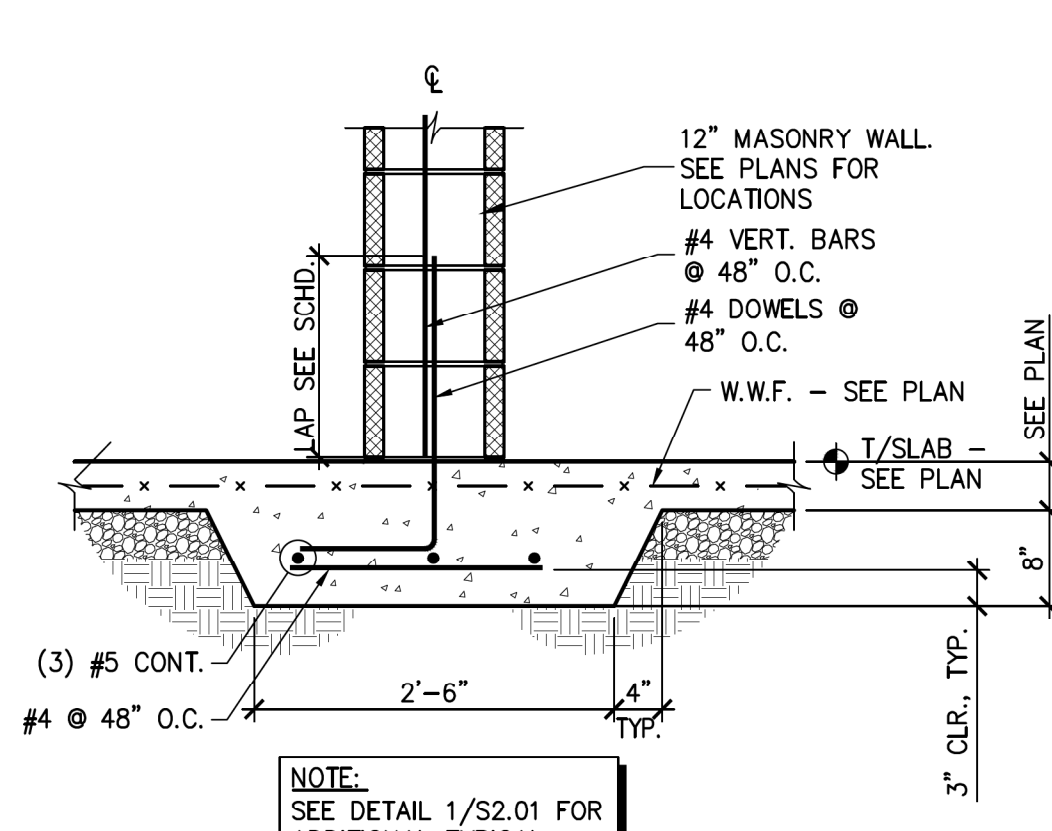
12 SECTION - TYP. COLUMN FOOTING  
S2.01/3/4" = 1'-0"



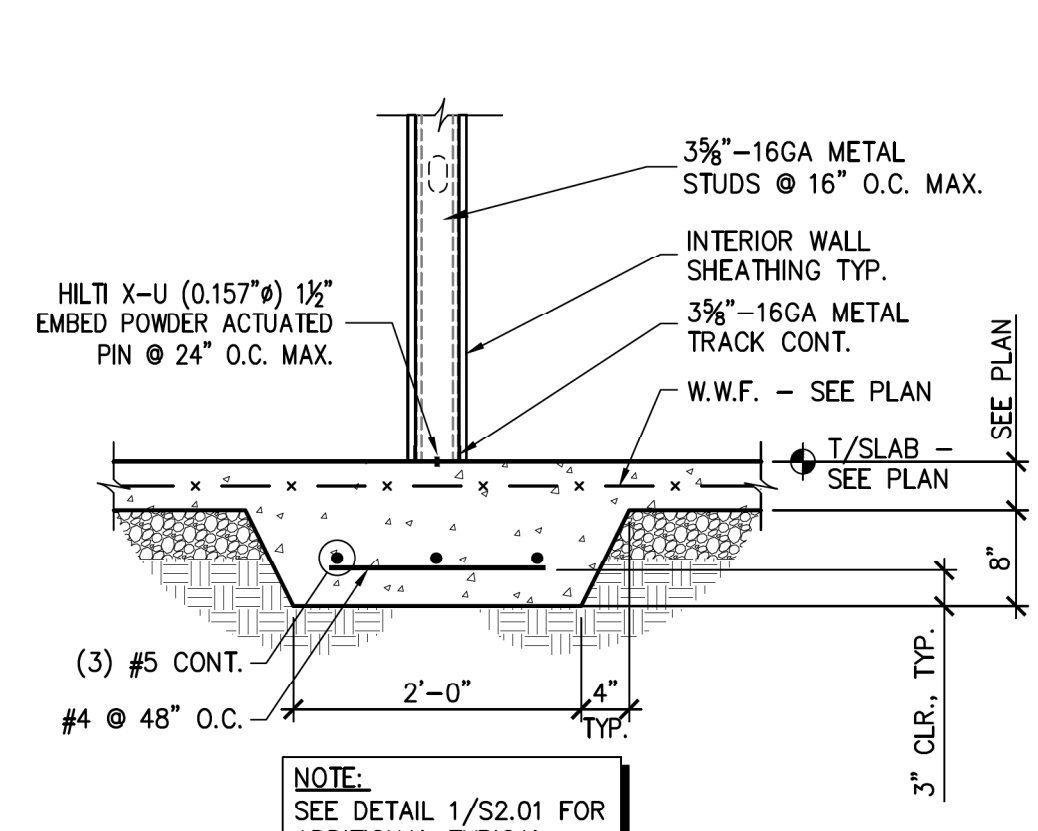
13 SECTION - TYP. WIND POST  
S2.01/3/4" = 1'-0"



14 SECTION - TYP. WALL OPENING  
S2.01/3/4" = 1'-0"



SECTION CMU WALL w/ THICKENED SLAB  
S2.01/3/4" = 1'-0"



SECTION @ STUD WALL w/ THICKENED SLAB  
S2.01/3/4" = 1'-0"



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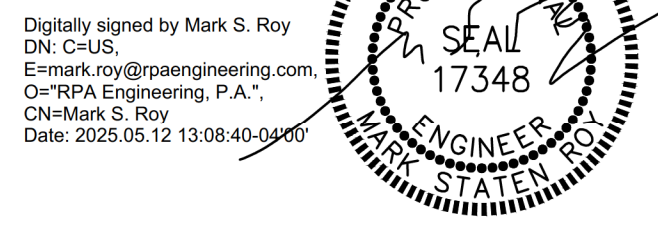
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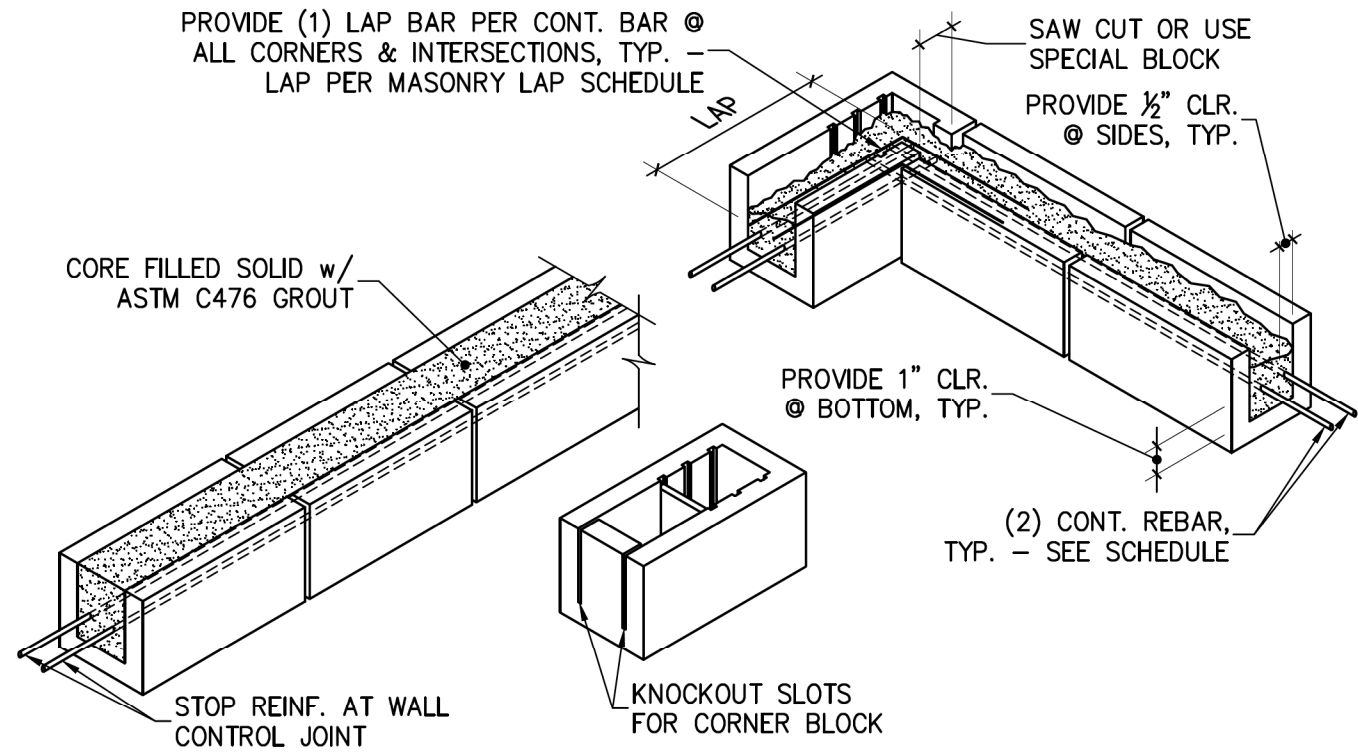
SHEET NAME & NUMBER  
FOUNDATION PLAN



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S2.01

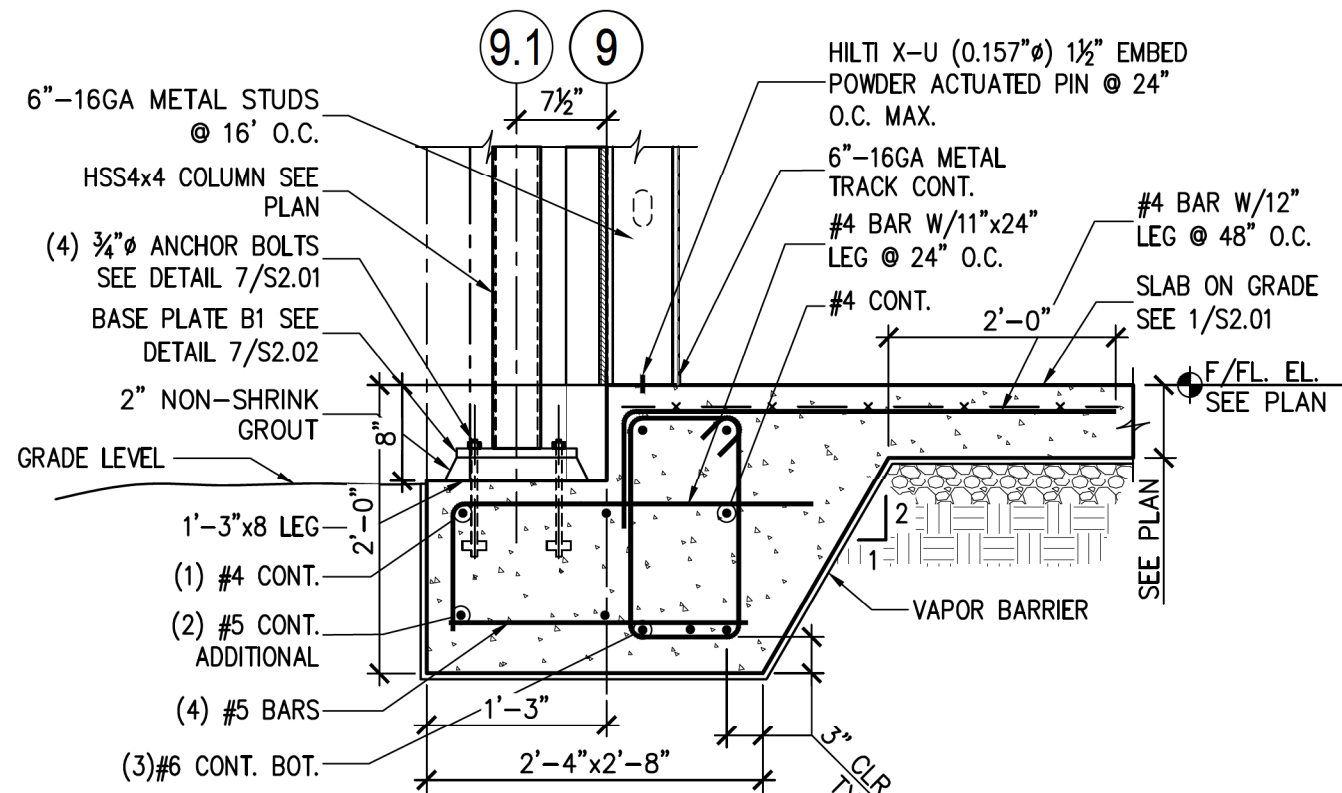




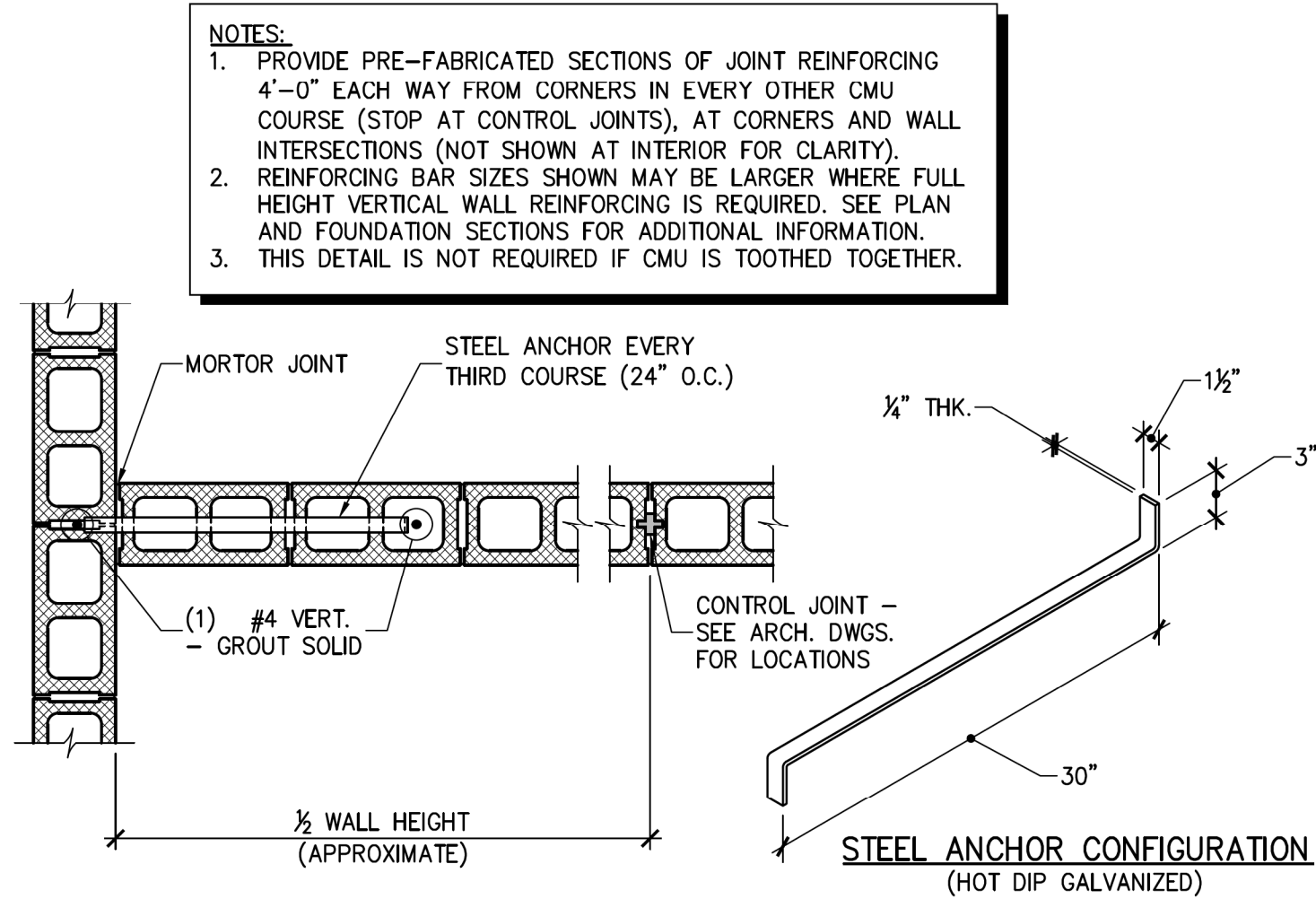
CMU BOND BEAM CONT. REINFORCING		
CMU THK.	REINF.	REMARKS
8"	(2) #5	-

NOTES:  
1. MATCH THICKNESS OF CORNER LAP BARS AND CONTINUOUS BARS.  
2. SEE MASONRY LAP SCHEDULE FOR LAP REQUIREMENTS AT CORNERS AND INTERSECTIONS.

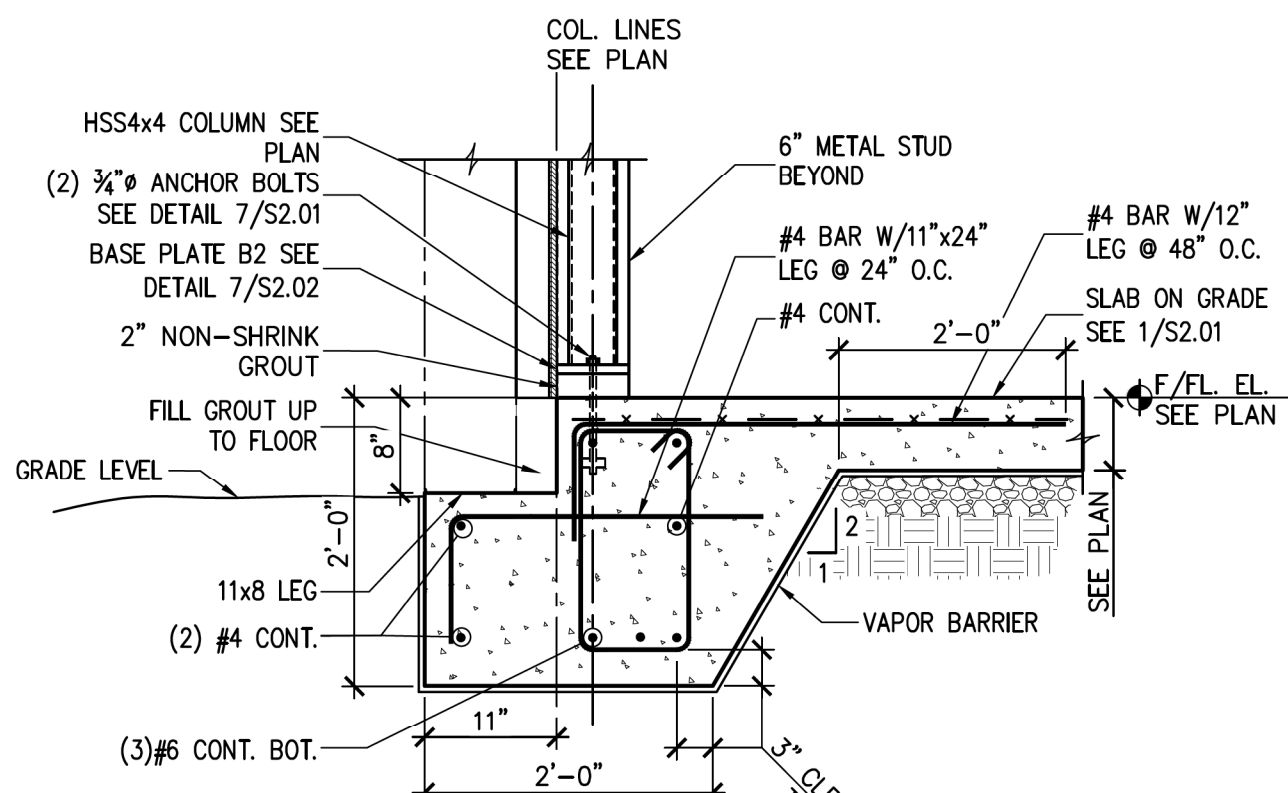
1 DETAIL - TYP. BOND BEAM  
S2.02 N.T.S.



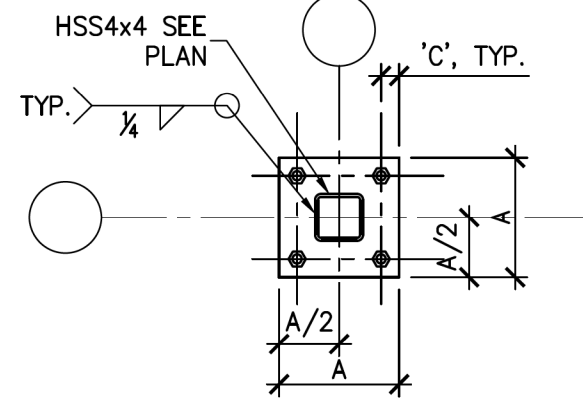
5 SECTION - TYP. GRADE BEAM  
S2.02 3/4" = 1'-0"



2 PLAN DETAIL - TYPICAL INTERIOR CMU WALL REINFORCING  
S2.02 3/4" = 1'-0"

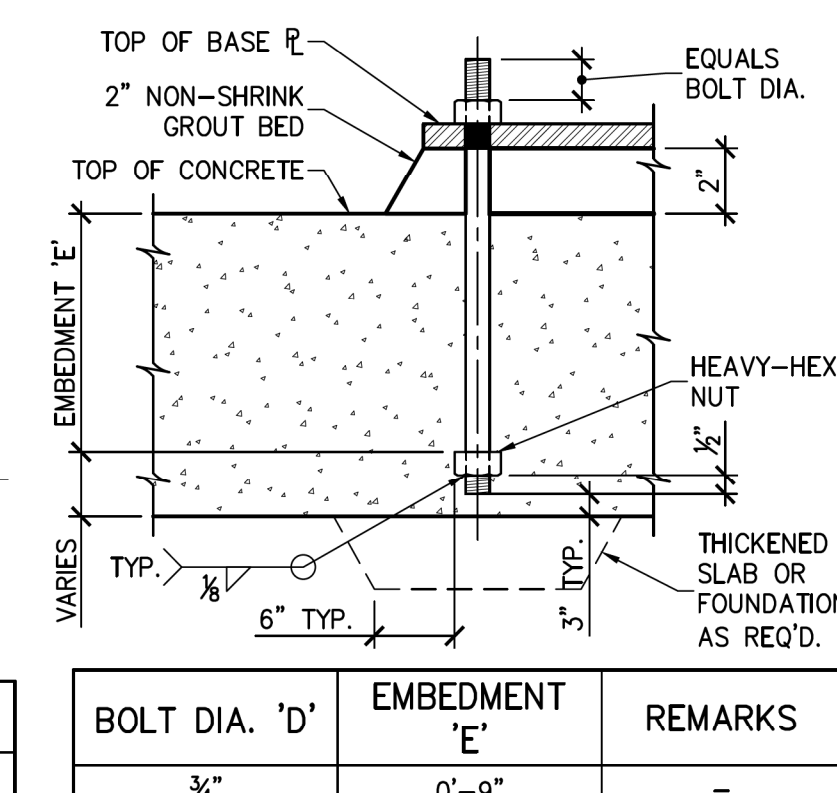


6 SECTION - TYP. GRADE BEAM  
S2.01 3/4" = 1'-0"



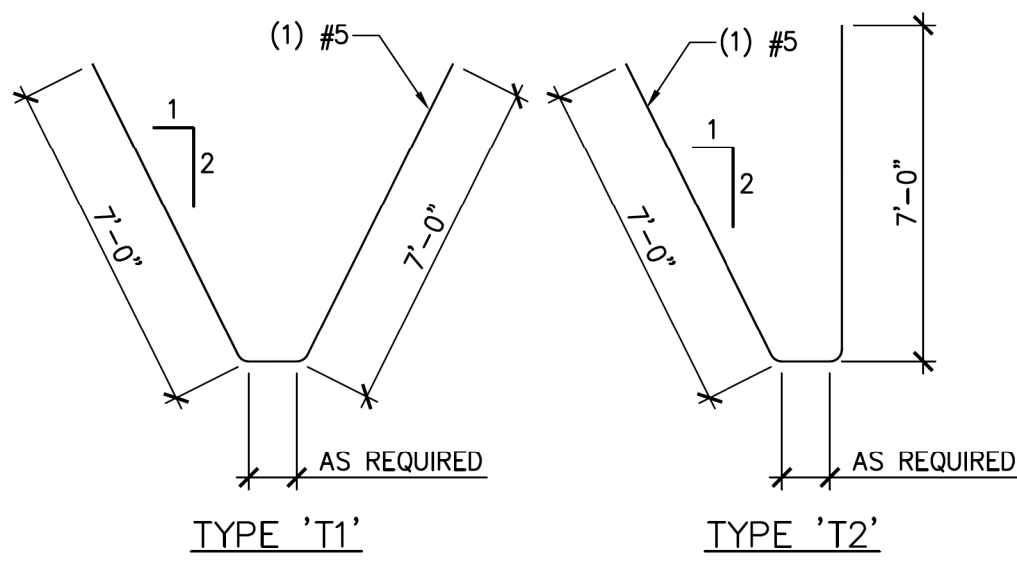
BOLT DIA.	DISTANCE 'C'
3/4" AND BELOW	1 1/2"

BASE PLATE

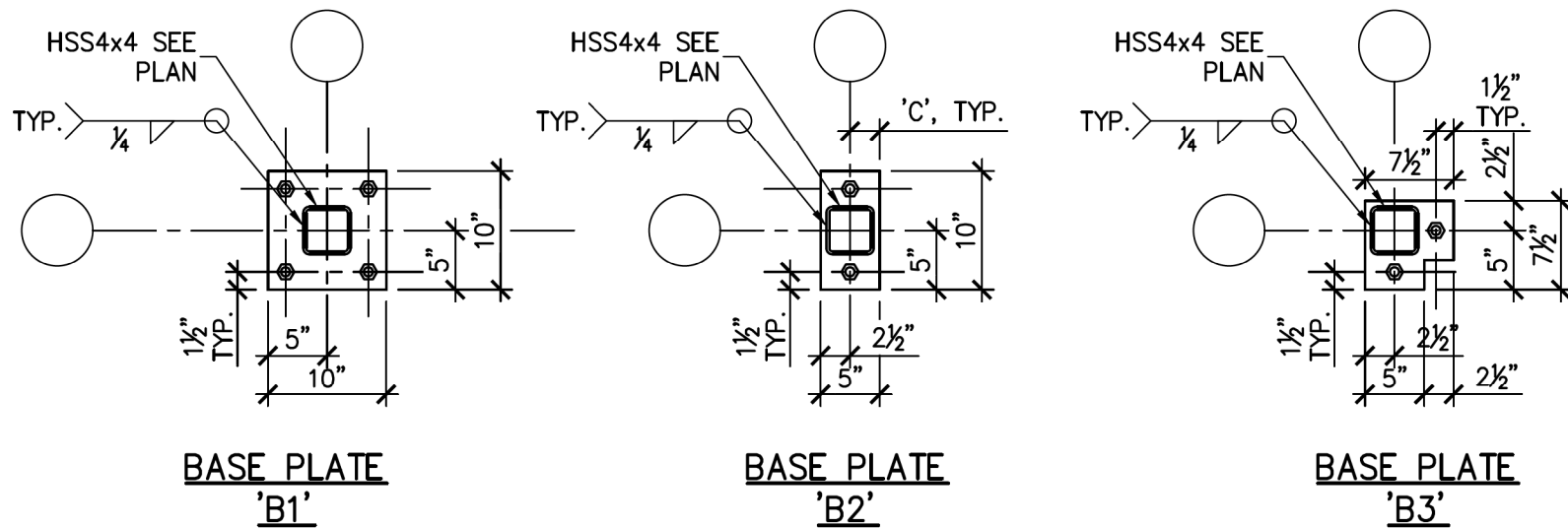


BOLT DIA. 'D'	EMBEDMENT 'E'	REMARKS
3/4"	0'-9"	-

7 TYPICAL ANCHOR BOLT DETAL  
S2.02 3/4" = 1'-0"



4 HAIRPIN TYPES  
S2.02 1/4" = 1'-0"



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O=RPA Engineering, P.A.,  
CN=Mark S. Roy  
Date: 2025.05.12 13:06:20-04'00'



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PROJECT #: 24008

ISSUE DATE: 04.30.2025

PHASE:

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SHEET NAME & NUMBER

MEZZANINE FLOOR  
FRAMING PLAN

S2.02

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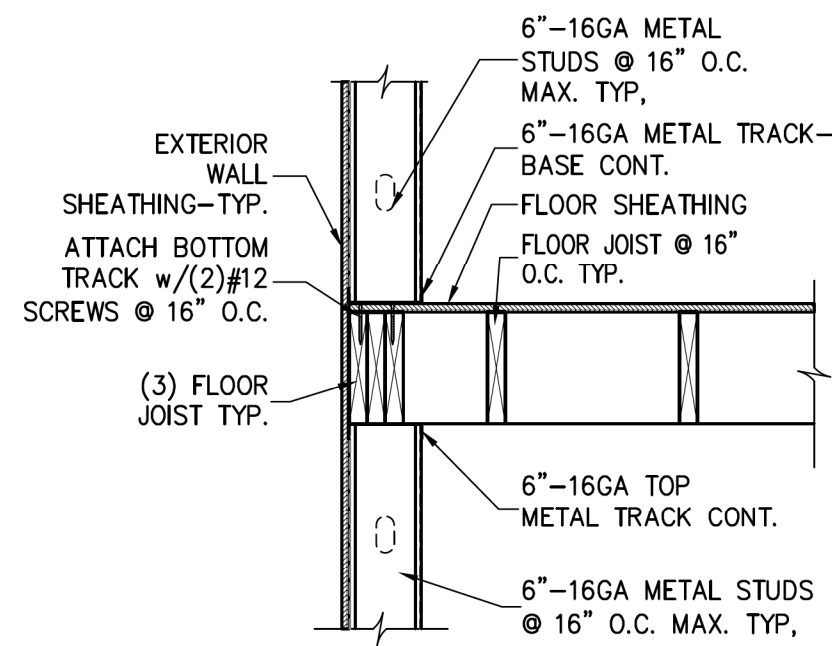


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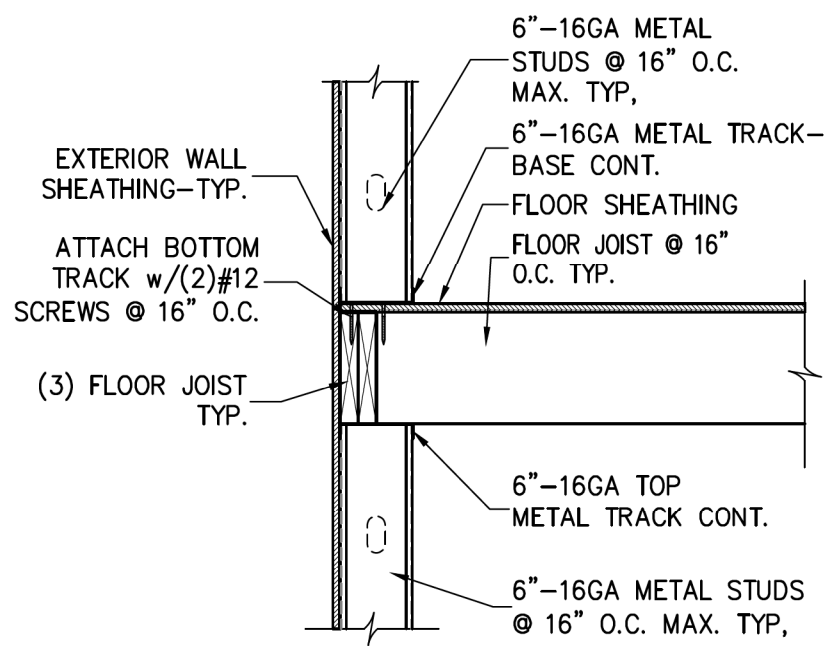
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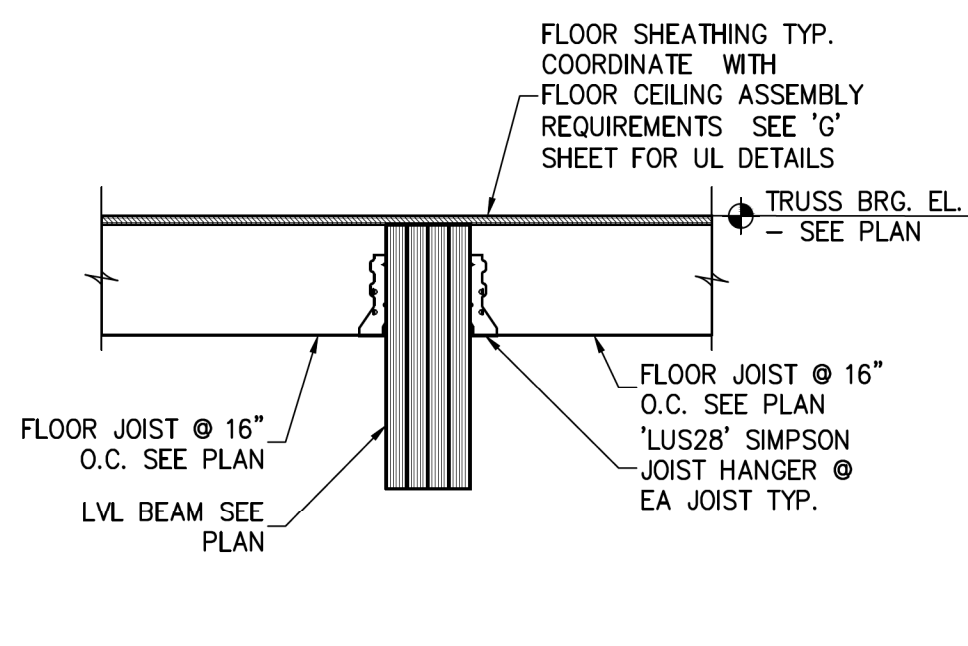
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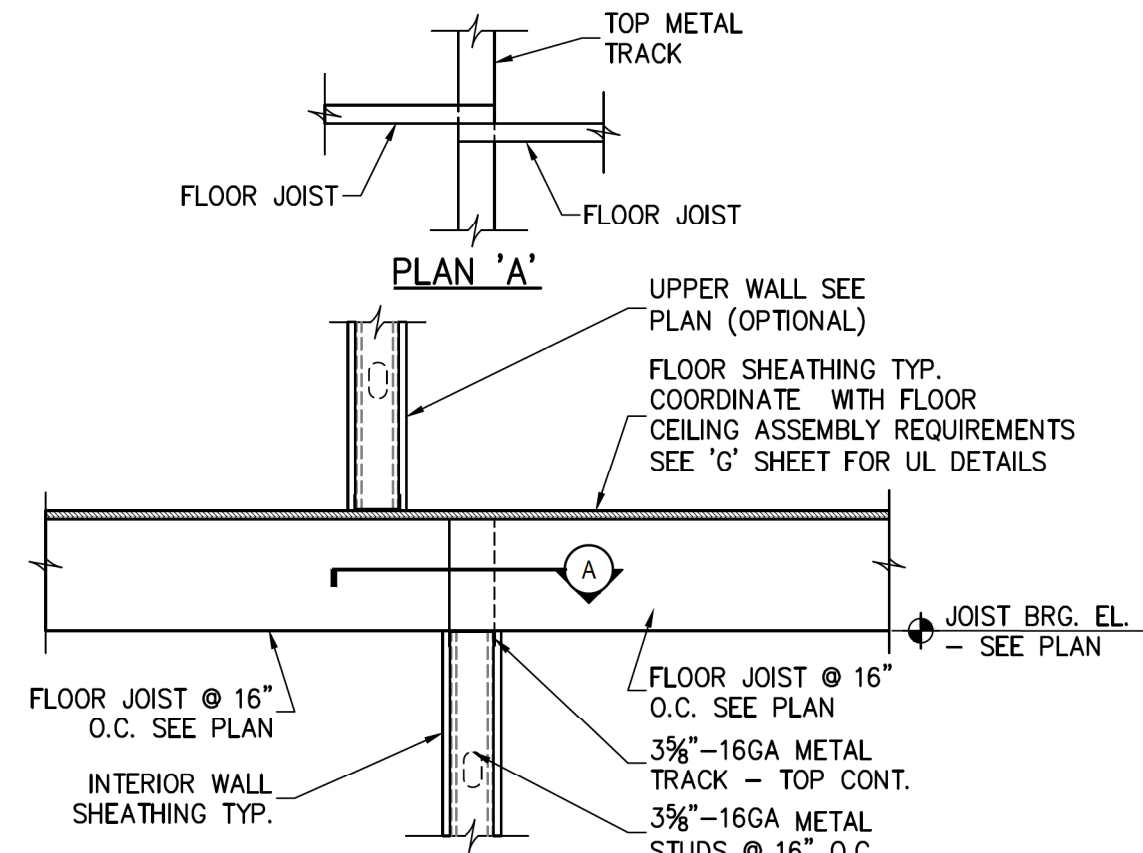
1 JOIST PARALLEL - EXT. WALL  
S3.01 3/4" = 1'-0"



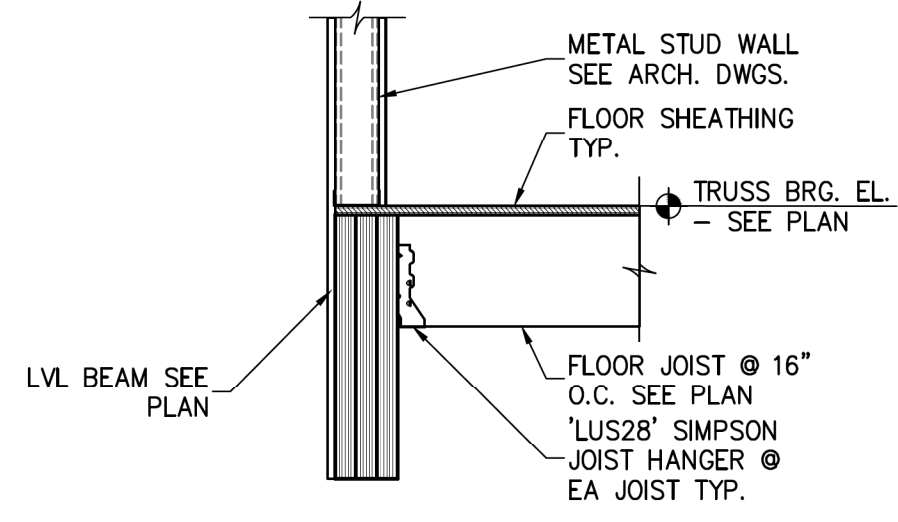
2 JOIST PERPENDICULAR - EXT. WALL  
S3.01 3/4" = 1'-0"



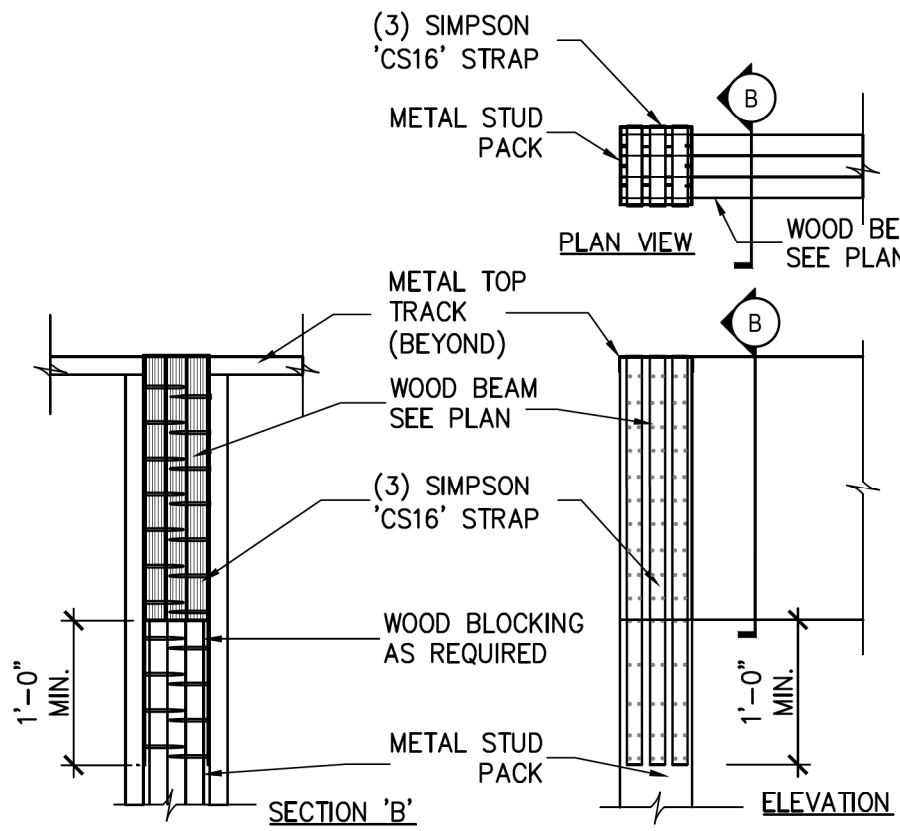
3 FLOOR JOIST BRG. @ WOOD BEAM  
S3.01 3/4" = 1'-0"



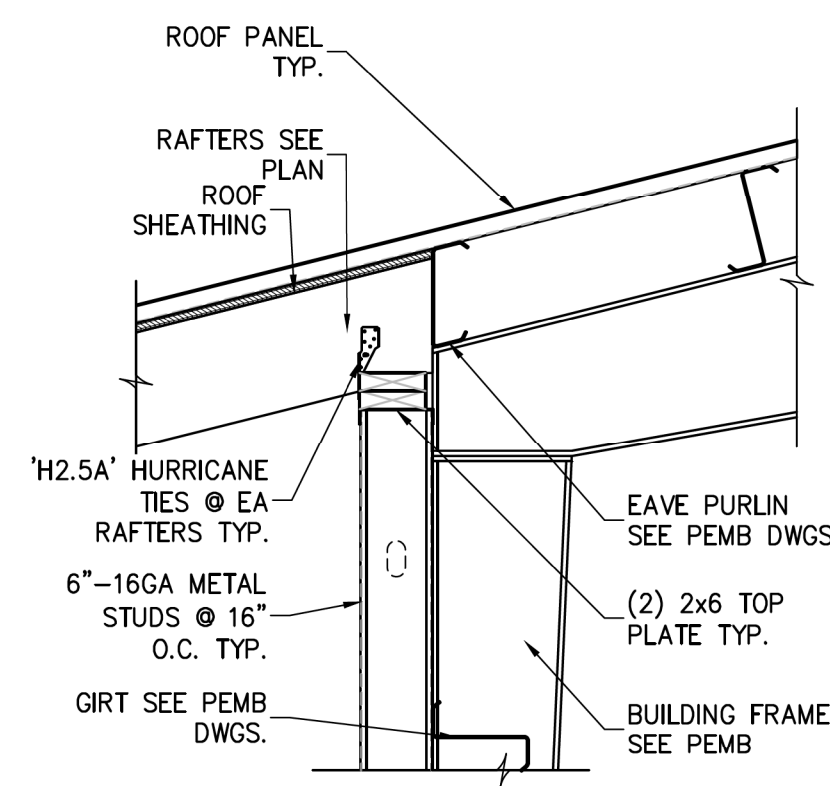
4 FLOOR JOISTS BRG. @ STUD WALL  
S3.01 3/4" = 1'-0"



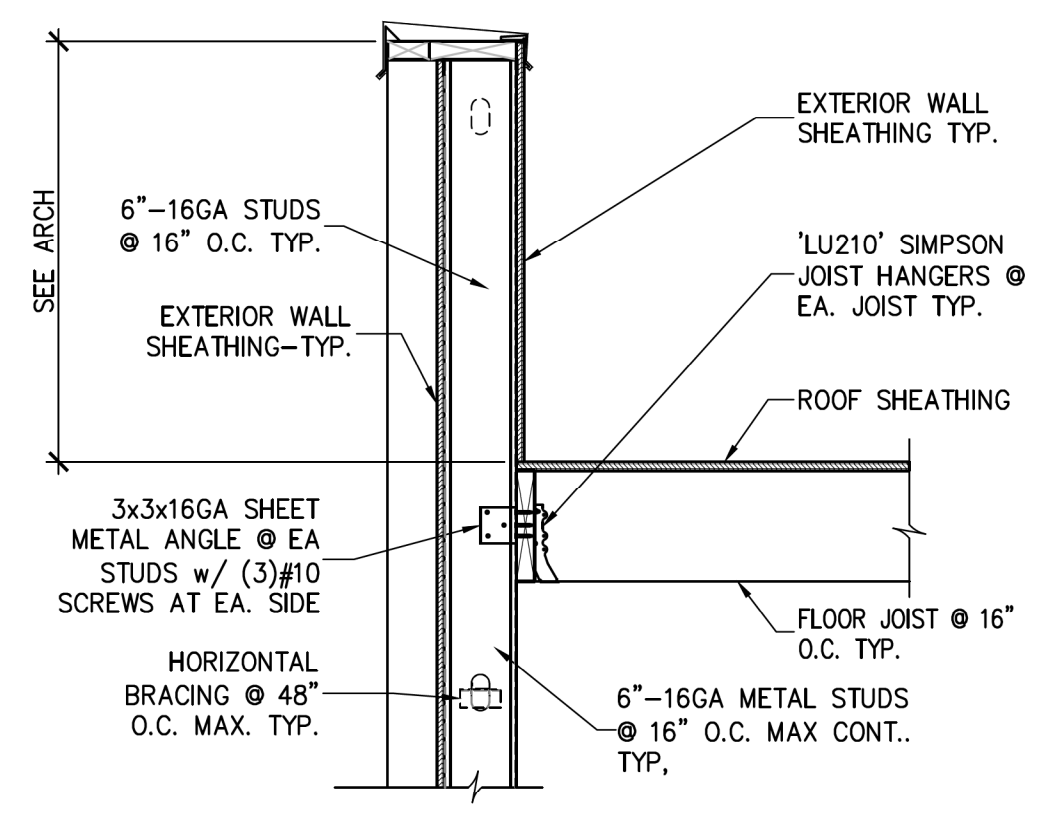
5 FLOOR JOIST BRG. @ WOOD BEAM  
S3.01 3/4" = 1'-0"



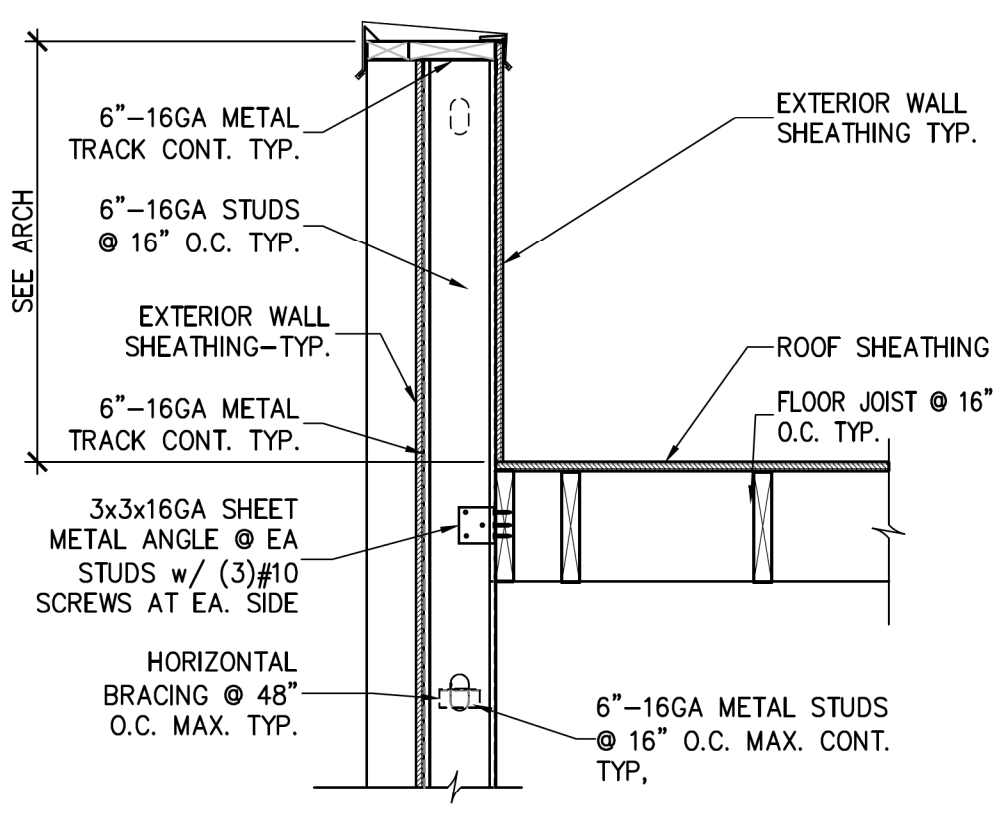
6 WOOD BEAM AT METAL STUD WALL  
S3.01 3/4" = 1'-0"



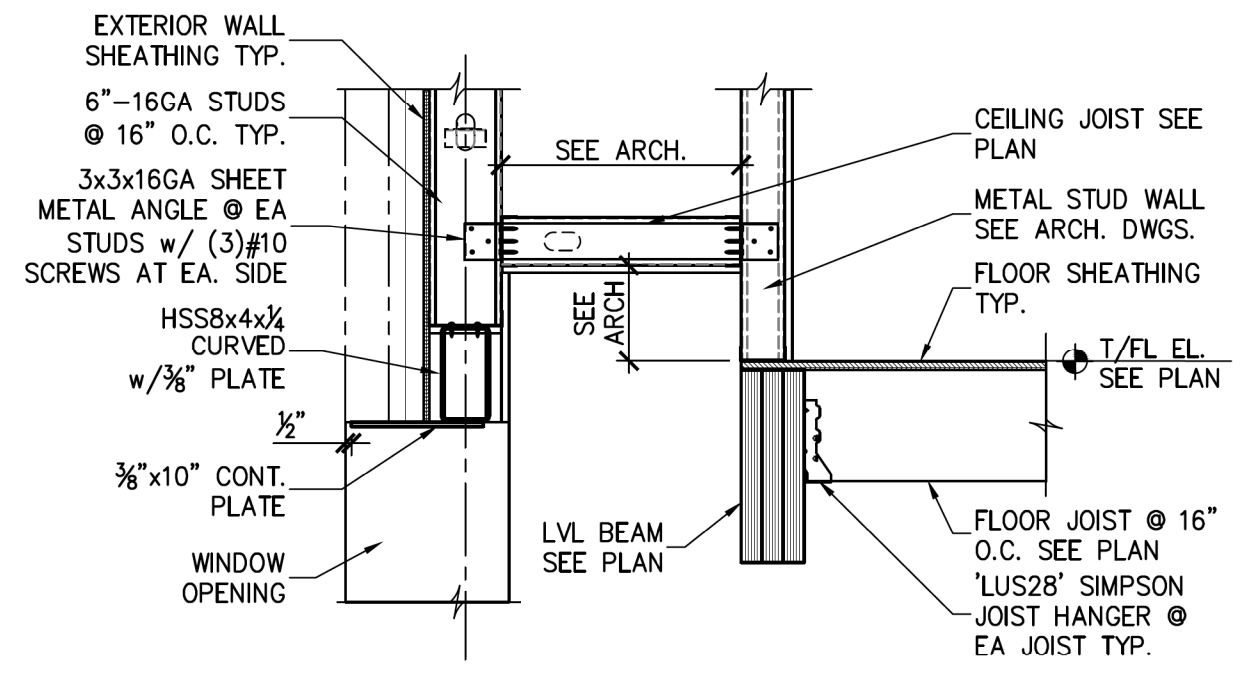
7 ROOF CONNECTION TYP.  
S3.01 3/4" = 1'-0"



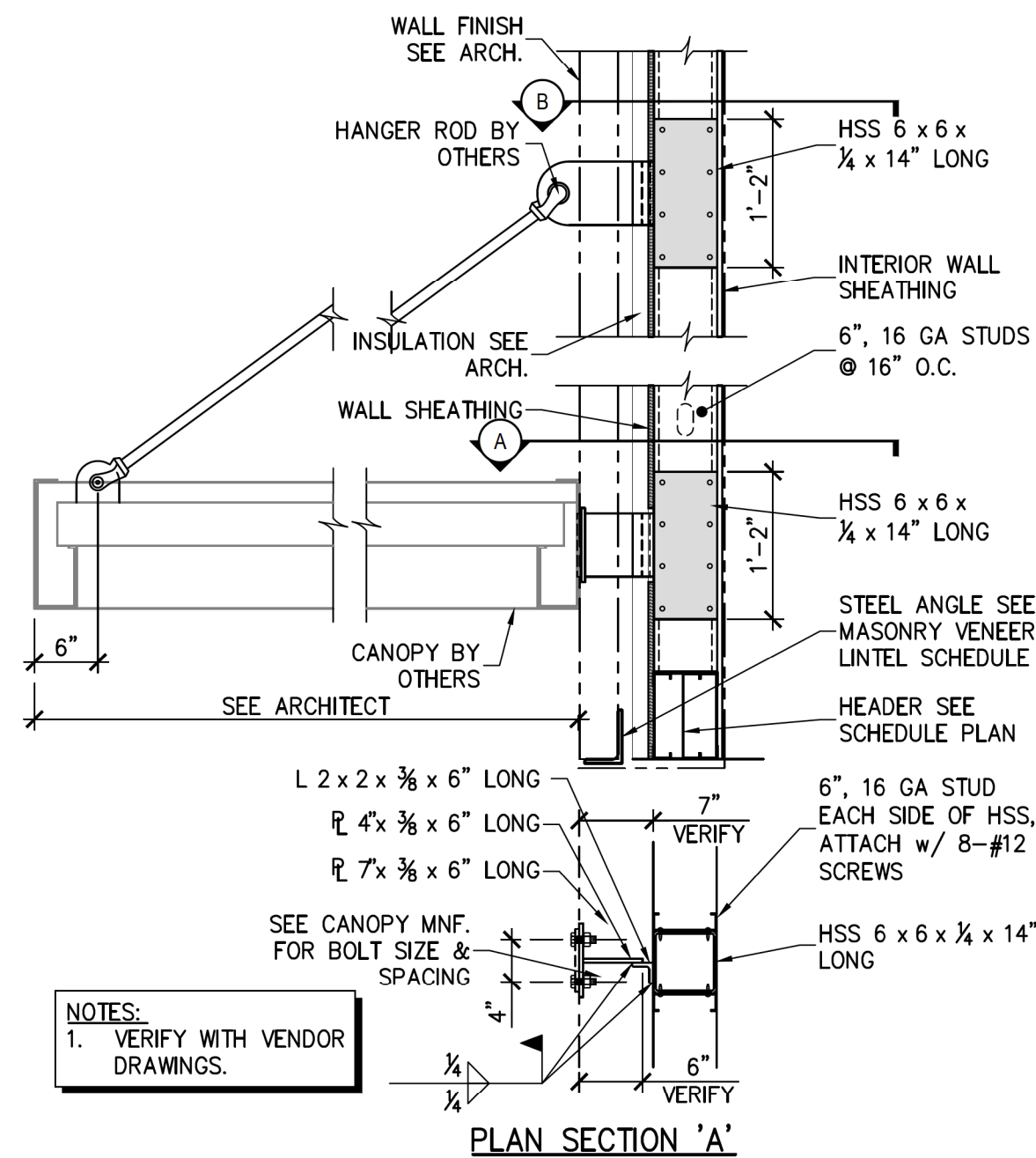
8 SECTION @ PARAPET  
S3.01 3/4" = 1'-0"



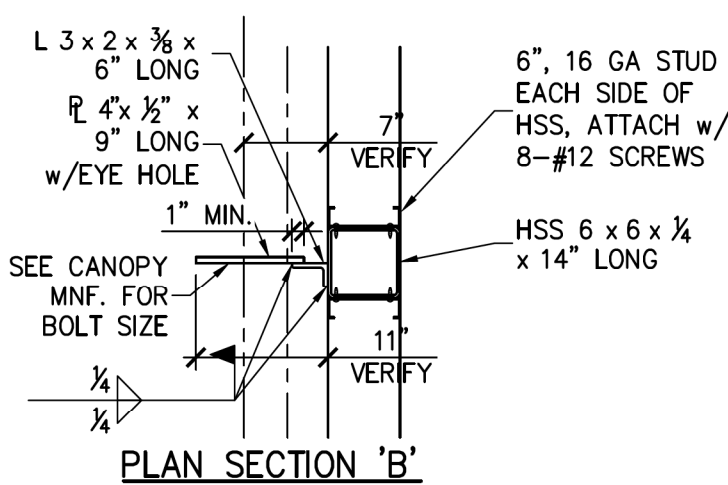
9 SECTION @ PARAPET  
S3.01 3/4" = 1'-0"



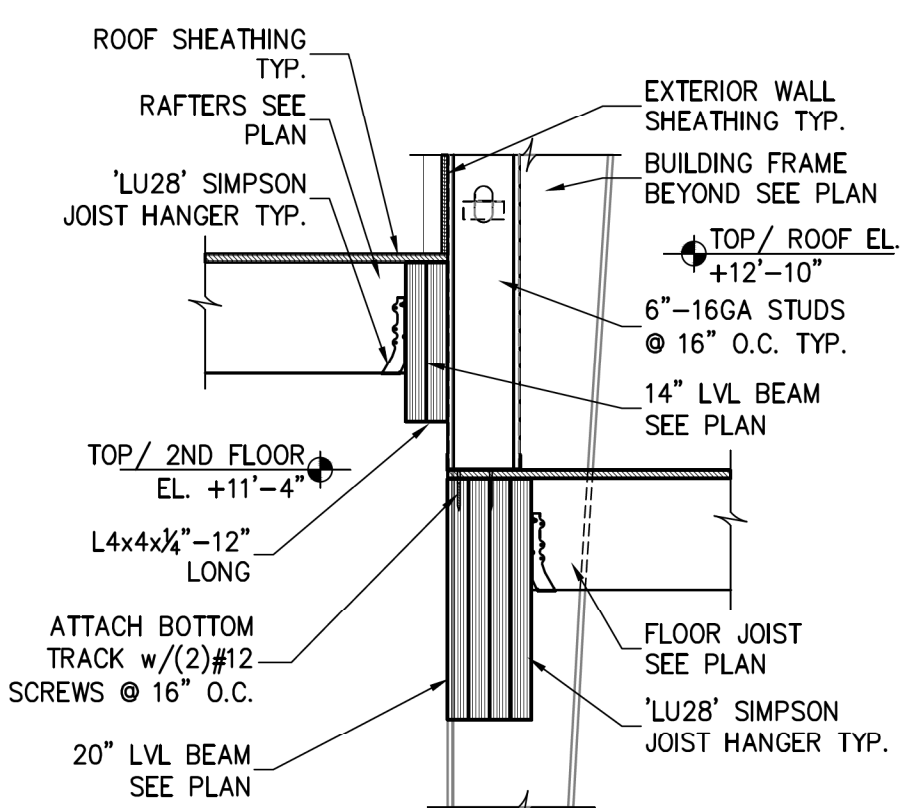
10 ROOF CONNECTION TYP.  
S3.01 3/4" = 1'-0"



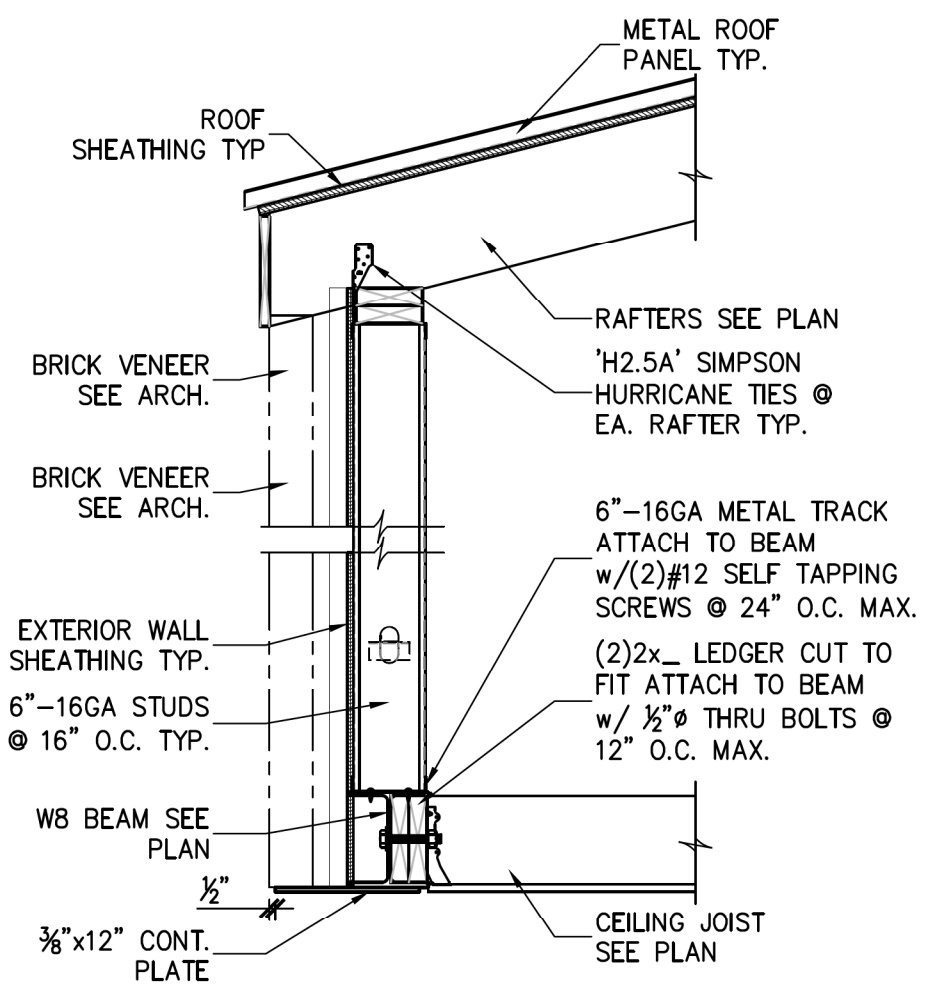
11 SECTION AT FRONT CANOPY  
S3.01 3/4" = 1'-0"



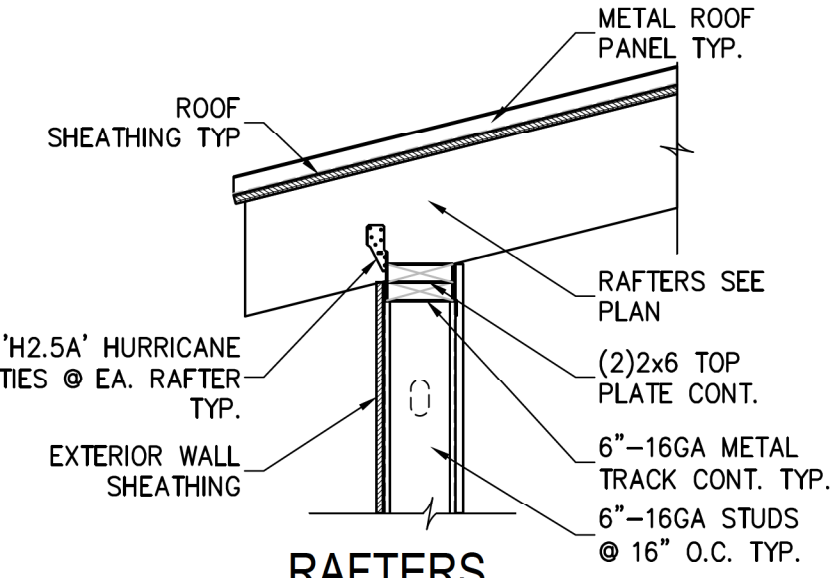
12 SECTION AT FLOOR - ROOF  
S3.01 3/4" = 1'-0"



13 SECTION AT PORCH  
S3.01 3/4" = 1'-0"



14 RAFTERS AT EXTERIOR WALL  
S3.01 3/4" = 1'-0"

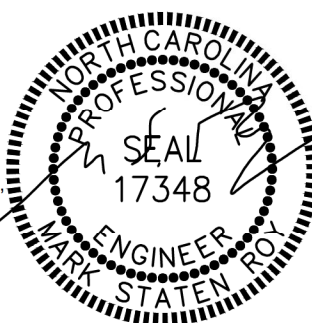


NOTES:  
1. VERIFY WITH VENDOR  
DRAWINGS.

THESE DRAWINGS AND THE ACCOMPANYING SPECIFICATIONS ARE INSTRUMENTS OF SERVICE AND AS SUCH ARE THE SOLE PROPERTY OF THE ARCHITECT. THEY HAVE BEEN PREPARED FOR A SPECIFIC PROJECT AND SHALL NOT BE USED IN CONNECTION WITH ANY OTHER PROJECT WITHOUT PRIOR WRITTEN PERMISSION OF THE ARCHITECT.

REVISIONS:  
# | DESC: | DATE

Digitally signed by Mark S. Roy  
DN: CN=US,  
E=mark.roy@rpaengineering.com,  
O=RPA Engineering, P.A.,  
CN=Mark S. Roy  
Date: 2025.05.12 13:08:01-0400



DRAWN BY: RPA/GBP RPA/MSR  
PROJECT #: 24008  
ISSUE DATE: 04.30.2025  
PHASE:  
CONSTRUCTION DOCUMENTS

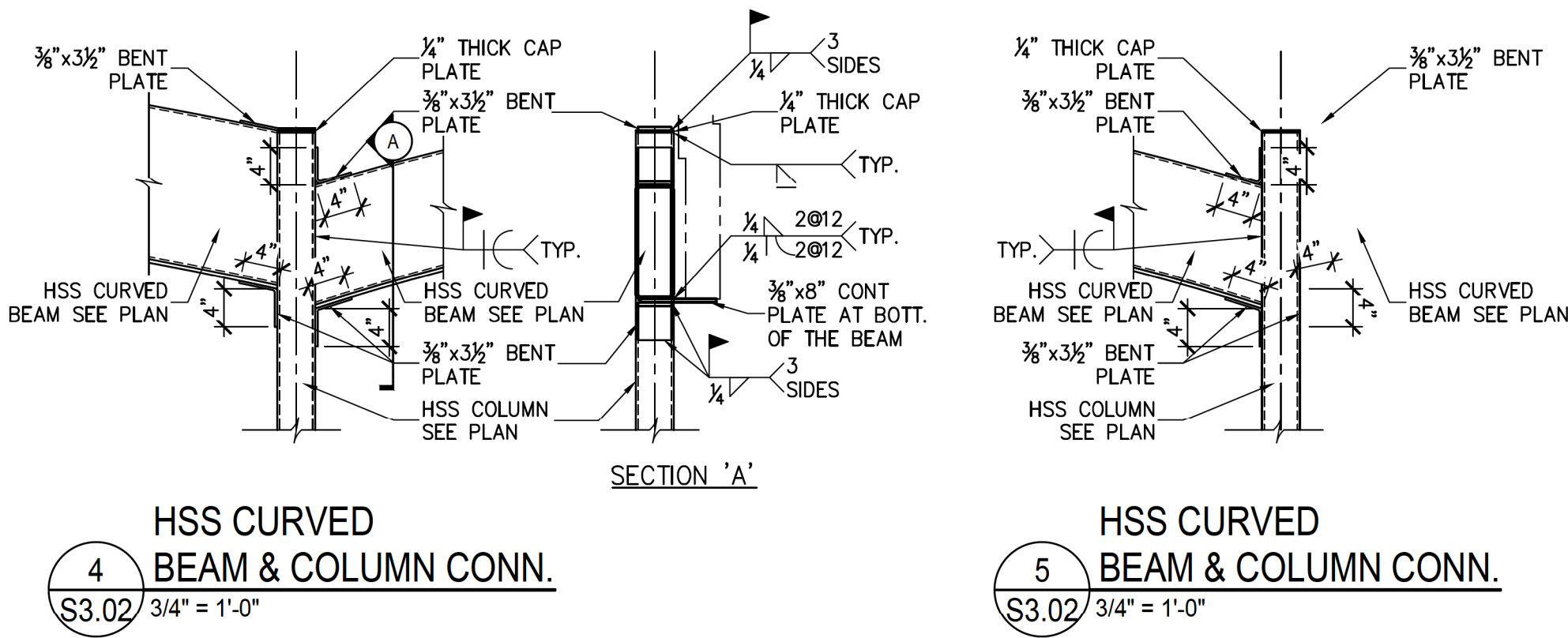
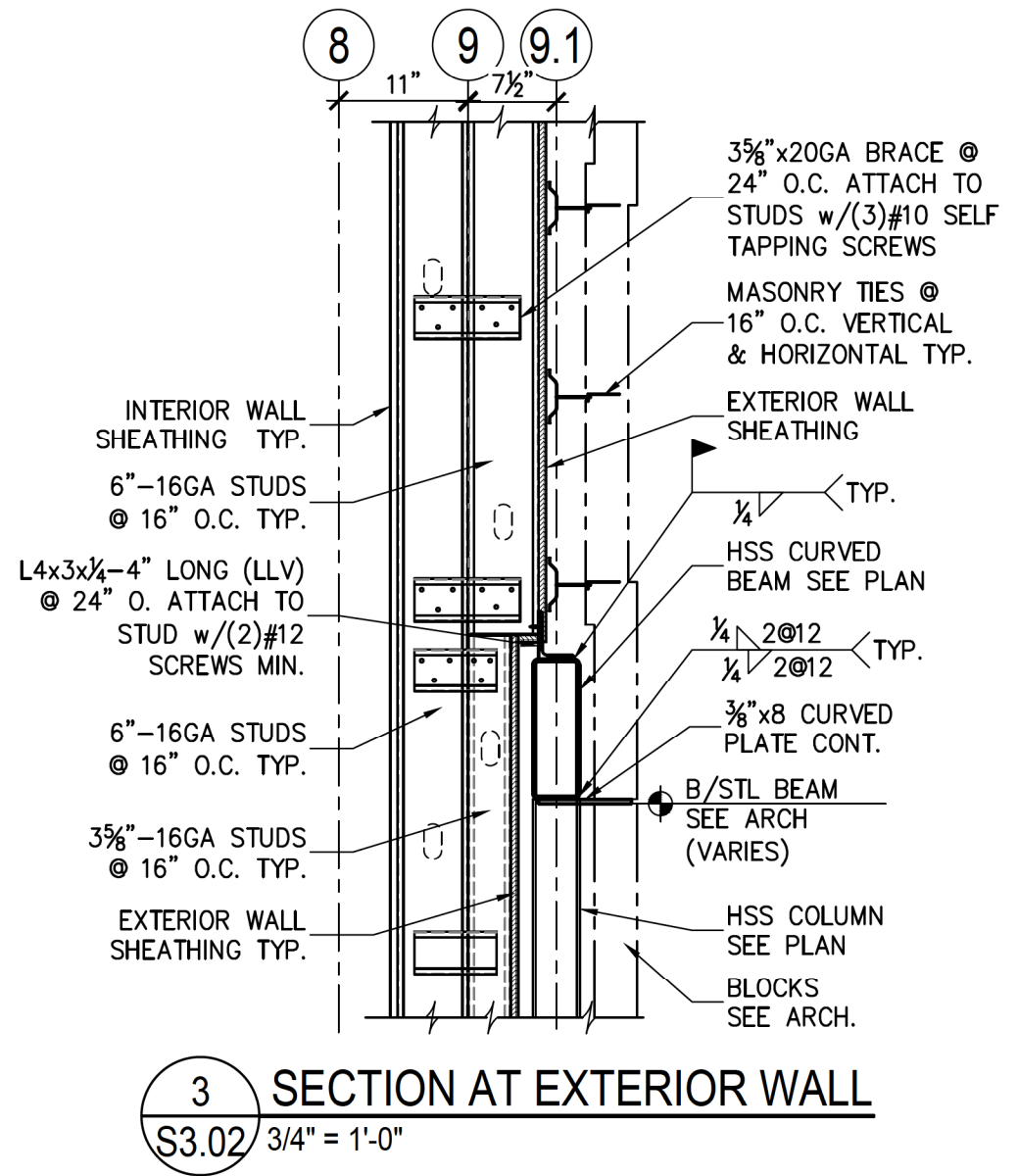
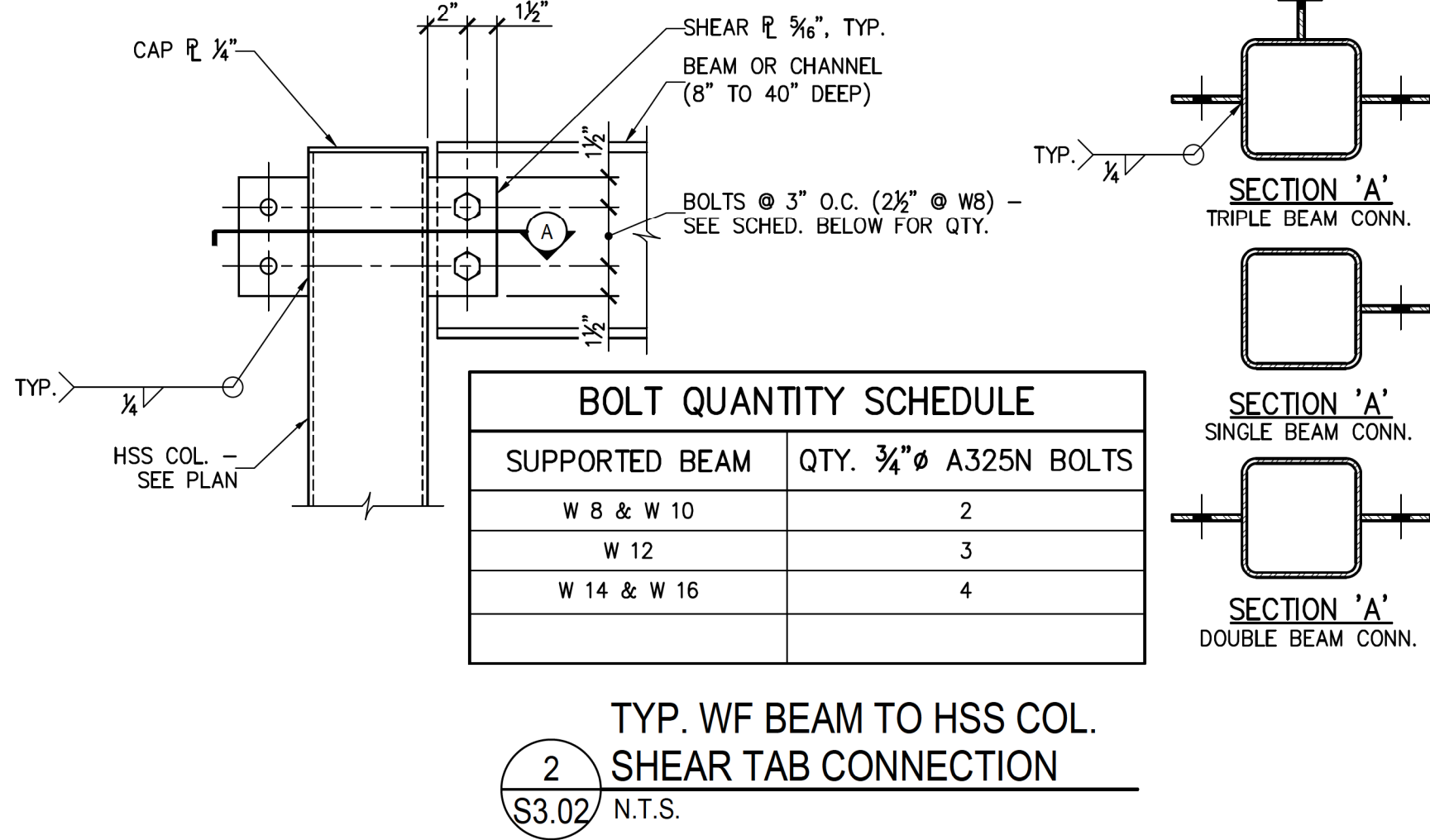
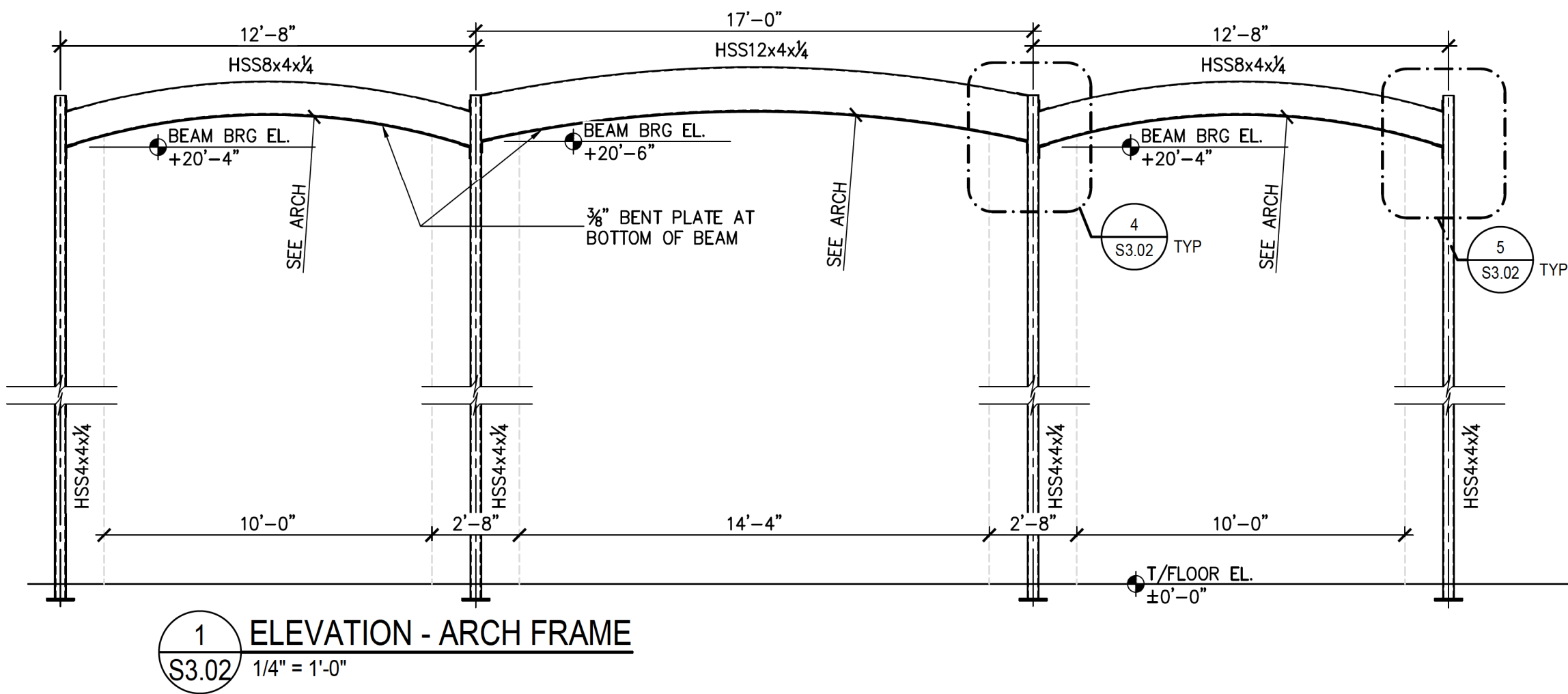
SHEET NAME & NUMBER

FRAMING SECTIONS &  
DETAILS

S3.01

RPA ENGINEERING, P.A.  
Structural Engineering Solutions  
Engineering License Certificate No. C-2734  
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Washington, NC 27889  
RPA Project No.: 2024223







GENERAL STRUCTURAL NOTES:

1. **GENERAL NOTES**
- 1.1. METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.
- 1.2. THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS OR OPENINGS NOT HEREIN INDICATED.
- 1.3. COORDINATE THESE DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS.
- 1.4. VERIFY ALL FLOOR AND ROOF OPENING SIZES AND LOCATIONS, EQUIPMENT PAD SIZES AND LOCATIONS, ANCHOR BOLT LAYOUTS, ETCETERA, WITH EQUIPMENT SELECTED.
- 1.5. VERIFY BUILDING LOCATION AND ORIENTATION WITH OWNER AND LOT SETBACK REQUIREMENTS BEFORE ANY CONSTRUCTION IS STARTED ON THE PROJECT.
- 1.6. CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION DIMENSIONS WHICH IMPACT NEW CONSTRUCTION PRIOR TO FABRICATING ANY REBAR, STEEL, TRUSSES, ETCETERA.
- 1.7. DO NOT CUT, NOTCH, OR OTHERWISE MODIFY ANY STRUCTURAL MEMBERS UNLESS SPECIFICALLY INDICATED ON THE DRAWINGS WITHOUT APPROVAL OF THE ENGINEER OF RECORD.
- 1.8. CUTTING OF STEEL MEMBERS AND INSTALLATION OF HOLES IN STEEL MEMBERS SHALL BE DONE BY CUTTING OR DRILLING. DO NOT USE TORCHES FOR CUTTING UNLESS APPROVED BY THE ENGINEER OF RECORD.
- 1.9. CONTRACTOR IS RESPONSIBLE FOR DESIGN AND INSTALLATION OF ALL SHORING REQUIRED TO SUPPORT NEW AND EXISTING STRUCTURAL ELEMENTS.
2. **FOUNDATION**
- 2.1. ALL FOOTINGS SHALL BE ON UNDISTURBED SOIL OR 98% COMPACTED FILL PER ASTM D698.
- 2.2. NO FOOTINGS OR SLABS SHALL BE POURED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, ICE OR LOOSE MATERIAL.
- 2.3. EXCAVATIONS FOR FOOTINGS SHALL HAVE THE SIDES AND BOTTOMS TEMPORARILY LINED WITH 6 MIL. POLYETHYLENE IF PLACEMENT OF CONCRETE DOES NOT OCCUR WITHIN 24 HRS OF THE EXCAVATION OF THE FOOTING.
- 2.4. ADVERSE FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION SUCH AS SOFT SOILS, ORGANIC MATTER, ETCETERA, SHALL BE REPORTED TO THE ENGINEER BEFORE FURTHER CONSTRUCTION IS ATTEMPTED.
- 2.5. IF UNDERMINING OF FOOTINGS OCCURS, FILL VOIDS WITH LEAN CONCRETE MIX. DO NOT ATTEMPT TO REPLACE AND RECOMPACT SOIL.
3. **REINFORCED CONCRETE MASONRY**
- 3.1. LOAD-BEARING MASONRY PIERS OR WALLS, FOUNDATION WALLS, AND ANY OTHER MASONRY SO DESIGNATED ON THE DRAWINGS, ARE CONSIDERED TO BE STRUCTURAL MASONRY.
- 3.2. COMPRESSIVE STRENGTH OF MASONRY UNITS
- SOLID CLAY UNITS 8250 PSI
- CONCRETE UNITS 1900 PSI ON NET AREA
- MINIMUM NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY (F<sub>m</sub>) IS 1,900 PSI.
- 3.3. MORTAR SHALL BE TYPE 'S' ASTM C270.
- 3.4. GROUT FOR REINFORCED MASONRY SHALL BE FINE GROUT ASTM C476. MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 3000 PSI. MAXIMUM HEIGHT TO WHICH MASONRY SHALL BE LAID BEFORE FILLING IS 6'-0". PROVIDE CLEANOUT OPENINGS AT THE BOTTOM OF EACH GROUT LIFT. CLEANOUT OPENINGS SHALL BE PROVIDED AT EACH CELL TO BE FILLED WITH GROUT.
- 3.5. REINFORCING GRADE AND DETAILS FOR MASONRY, SHALL BE AS THAT FOR CONCRETE. TIE IN REBAR IN POSITION, AND PLACE CONCRETE AROUND REINFORCING DURING CONSTRUCTION OF MASONRY. DO NOT PUSH REINFORCING DOWN INTO PREVIOUSLY PLACED GROUT FILL. SET BOLTS SIMILARLY. TIE WYTHES WITH HORIZONTAL REINFORCING AS SPECIFIED.
- 3.6. ALL CELLS BELOW GRADE SHALL BE FULLY GROUTED WITH MASONRY GROUT.
- 3.7. PROVIDE HORIZONTAL JOINT REINFORCING AT 16" O.C. UNLESS OTHERWISE NOTED.
- 3.8. PLACE ALL VERTICAL REINFORCING BARS IN CENTERS OF BLOCK CELLS UNLESS OTHERWISE NOTED.
- 3.9. FILL ALL CELLS, AT VERTICAL REINFORCING, FULL HEIGHT WITH MASONRY GROUT.
- 3.10. CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS IN MASONRY. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SIZE AND LOCATION OF OPENINGS.
- 3.11. ALL MASONRY WORK PERFORMED, SHALL BE IN ACCORDANCE WITH ACI/ASCE 530, MASONRY CONSTRUCTION AND MATERIALS USED, SHALL CONFORM TO ALL REQUIREMENTS OF THESE CONTRACT DOCUMENTS.
- 3.12. UNLESS OTHERWISE SHOWN, MASONRY WALLS SHALL HAVE VERTICAL CONTROL JOINTS AT A MAXIMUM SPACING OF 40'-0" ON CENTER FOR BRICK AND OF 25'-0" FOR CMU. THE JOINT SHALL BE FORMED USING PVC MATERIAL CONFORMING TO ASTM D2287, TYPE PVC 654-4. COORDINATE LOCATION OF JOINTS WITH THE ARCHITECTURAL ELEVATIONS.
- 3.13. PLACE A CONTINUOUS HORIZONTAL CMU BOND BEAM AT EACH FLOOR, AND AT THE TOP OF THE WALL, AND AT INTERMEDIATE LOCATIONS AS REQUIRED TO PROVIDE A MAXIMUM VERTICAL SPACING OF 12'-0", UNLESS OTHERWISE NOTED ON THE PLAN.
4. **CONCRETE**
- 4.1. ALL PLACED CONCRETE, SHALL HAVE NORMAL WEIGHT COARSE AGGREGATES UNLESS OTHERWISE NOTED, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f<sub>c</sub>) AT 28 DAYS AS SHOWN ON THE CONCRETE MATERIALS SCHEDULE.
- 4.2. GROUT FOR BASE PLATES SHALL BE NON-METALLIC, NON-SHRINKABLE GROUT, AND SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH, AT 28 DAYS, OF 5000 PSI.
- 4.3. CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
- 4.4. CHAMFER ALL EXPOSED EXTERNAL CORNERS OF CONCRETE WITH ¾" x 45 DEGREE CHAMFER, UNLESS OTHERWISE NOTED.
- 4.5. HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCING SHALL BE CONTINUOUS, AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED, WITH A CLASS B TENSION SPLICE, AT CORNERS AND INTERSECTIONS. TOP BAR CRITERIA SHALL APPLY IF 12" OR MORE OF FRESH CONCRETE IS PLACED BELOW BAR.
- 4.6. SEE ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING / DAMPPROOFING DETAILS.
- 4.7. ALL DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- 4.8. SEE ARCHITECTURAL DRAWINGS FOR TYPE AND LOCATION OF FLOOR FINISHES.
- 4.9. SEE MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS FOR ADDITIONAL WALL / SLAB OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 4.10. ALL REINFORCING SHALL CONFORM TO ASTM A515, GRADE 60.
- 4.11. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- 4.12. DETAIL AND FABRICATE REINFORCING STEEL IN ACCORDANCE WITH THE ACI DETAILING MANUAL.
- 4.13. IN-PLACE REINFORCING STEEL, SHALL BE REVIEWED BY THE ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
- 4.14. AT CORNERS AND INTERSECTIONS, PROVIDE BARS OF THE SAME NUMBER AND SIZE AS THE LONGITUDINAL BARS IN THE FOOTING.
- 4.15. CONCRETE MATERIALS SHALL BE AS FOLLOWS:
- 4.15.1. USE TYPE 1/II PORTLAND CEMENT CONFORMING TO ASTM C150
- 4.15.2. AGGREGATE SHALL CONFORM TO ASTM C33 (FINE AND COARSE AGGREGATES)
- 4.15.3. AIR ENTRAINING ADMIXTURE SHALL CONFORM TO ASTM C260
- 4.15.4. PLASTICIZER CAN BE USED TO IMPROVE WORKABILITY IF REQUIRED
- 4.16. CONCRETE MIX DESIGN:
- 4.16.1. MAXIMUM WATER/CEMENT RATIO – 0.50 FOR SLAB, 0.55 FOR FOOTINGS AND OTHER CONCRETE UNLESS OTHERWISE NOTED.
- 4.16.2. SLUMP SHALL BE 4 INCHES TO 6 INCHES (WITHOUT PLASTICIZER)
- 4.16.3. AIR ENTRAINMENT SHALL BE 4% TO 6% (EXTERIOR CONCRETE)
- 4.17. CONCRETE SLAB SHALL BE CURED USING A WATER-BASED CURING COMPOUND. CURING COMPOUND SHALL BE APPLIED TO ALL HORIZONTAL SURFACES. ONCE THE SURFACE WATER DISSIPATES AND THE SURFACE IS NOT MARRED BY WALKING, APPLY PER MANUFACTURER'S SPECIFICATIONS.
- 4.18. CONDUCT SLUMP, AIR, AND STRENGTH TESTS OF CONCRETE IN ACCORDANCE WITH THE FOLLOWING PROCEDURES:
- 4.18.1. SECURE SAMPLES IN ACCORDANCE WITH "METHOD OF SAMPLING FRESH CONCRETE" (ASTM C 172). MOLD AND CURE FIVE SPECIMENS FROM EACH SAMPLE IN ACCORDANCE WITH "METHOD OF MAKING ANDS CURING CONCRETE, COMPRESSION AND FLEXURE SPECIMENS IN THE FIELD" (ASTM C 31). FIVE SPECIMENS COMPRISE ONE TEST, TEST TWO SPECIMENS AT 7 DAYS (ASTM C 39). TEST TWO SPECIMENS AT 28 DAYS IN ACCORDANCE WITH "METHOD OF TEST FOR COMPRESSIVE STRENGTH OF MOLDED CONCRETE CYLINDERS" (ASTM C 39). TEST EVALUATION SHALL BE CONDUCTED IN ACCORDANCE WITH PROVISIONS OF ACI 318-05. KEEP ONE SPECIMEN IN RESERVE.
- 4.18.2. MAKE ONE STRENGTH TEST FOR EACH 100 CUBIC YARDS OR FRACTION THEREOF FOR EACH MIX DESIGN OF CONCRETE PLACED IN ONE DAY, EXCEPT THAT IN NO CASE SHALL A GIVEN MIX DESIGN BE REPRESENTED BY LESS THAN THREE TESTS.
5. **PRE-ENGINEERED METAL BUILDINGS**
- 5.1. CONFIGURATION, COLUMN LOCATIONS, EAVE HEIGHTS, ROOF SLOPE, ETCETERA, SHALL BE AS SHOWN ON THE DRAWINGS. SHOULD BUILDING MANUFACTURER WISH TO FURNISH A SYSTEM THAT WILL DIFFER FROM THAT SHOWN, WRITTEN APPROVAL SHALL BE OBTAINED FROM THE ARCHITECT/ENGINEER OF RECORD PRIOR TO BIDDING.
- 5.2. BUILDING DESIGN AND LOAD APPLICATION SHALL CONFORM TO THE CURRENT NORTH CAROLINA STATE BUILDING CODE. THE COLLATERAL LOAD SHALL NOT BE USED TO REDUCE THE EFFECTS OF WIND LOADS ON THE BUILDING. REFER TO THE 'GENERAL' SECTION OF THE STRUCTURAL NOTES FOR ADDITIONAL LOADING INFORMATION.
- 5.3. THE METAL BUILDING FRAMES SHALL BE DESIGNED SUCH THAT THE MAXIMUM HORIZONTAL DRIFT DUE TO WIND AND SEISMIC LOADING SHALL SATISFY AN H /180 CRITERIA. THE MAXIMUM VERTICAL DEFLECTION OF PRIMARY AND SECONDARY FRAMING MEMBERS SHALL BE WITHIN THE TOLERANCES PROSCRIBED BY THE NC STATE BUILDING CODE. MANUFACTURER SHALL VERIFY THAT THE DEFLECTION CRITERIA ARE COMPATIBLE WITH EXTERIOR AND INTERIOR FINISHES SUPPORTED BY THE METAL BUILDING STRUCTURE.
- 5.4. THE FOOTING DESIGN IS BASED UPON AN ASSUMED LOADING OF THE METAL BUILDING SUPER-STRUCTURE. THE FOUNDATIONS SHALL NOT BE CONSTRUCTED UNTIL THE STRUCTURAL ENGINEER HAS REVIEWED THE ACTUAL DESIGN REACTIONS SUPPLIED BY THE MANUFACTURER.

6. **COLD FORMED METAL FRAMING**

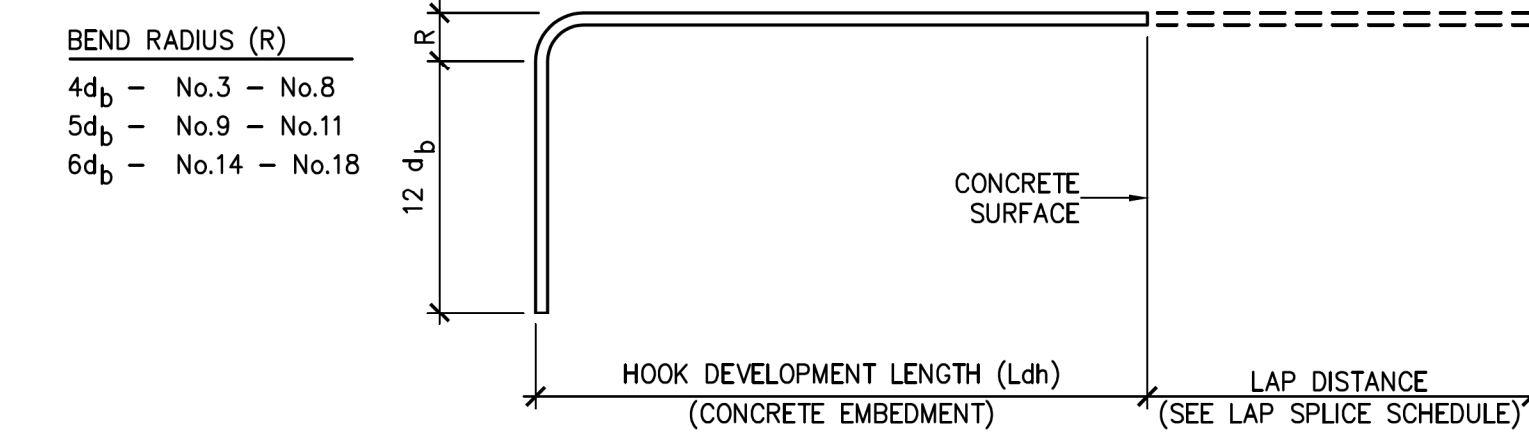
- 6.1. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE AS SHOWN ON THE DRAWINGS AND AS REQUIRED BY THE MANUFACTURER'S SPECIFICATIONS.
- 6.2. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) 'SPECIFICATION FOR THE DESIGN OF COLD FORMED STRUCTURAL MEMBERS', LATEST EDITION.
- 6.3. ALL STRUCTURAL MEMBERS SHALL BE FORMED OF CORROSION RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENT OF ASTM-A446, WITH A MINIMUM YIELD STRENGTH OF 40 KSI.
- 6.4. ALL STRUCTURAL MEMBERS SHALL BE ZINC COATED AND CONFORM TO ASTM-A525.
- 6.5. INSTALL JACK AND KING STUDS AT ALL WINDOW AND DOOR OPENINGS IN EXTERIOR WALLS AND INTERIOR LOAD-BEARING WALLS PER THE BOX BEAM HEADER SCHEDULE.
- 6.6. ALL EXTERIOR STUD WALLS ARE LOAD BEARING UNLESS OTHERWISE NOTED. SEE ARCHITECTURAL DETAILS FOR ADDITIONAL INFORMATION ON CONNECTIONS. ALL EXTERIOR WALL STUDS SHALL BE 6", 16 GA., SPACED AT 16" O.C., UNLESS OTHERWISE NOTED. STUD TRACK GAUGE SHALL MATCH THE STUD GAUGE SPECIFIED UNLESS OTHERWISE NOTED.
- 6.7. BOTTOM TRACK SHALL BE ATTACHED WITH 'HILTI X-U' POWDER ACTUATED FASTENERS (0.157" SHANK DIAMETER) WITH 1¼" EMBEDMENT AT 12" O.C.
- 6.8. ALL STUDS INSTALLED BELOW STEEL BEAMS OR OTHER LOAD BEARING STRUCTURAL MEMBERS SHALL BE ATTACHED WITH A CONTINUOUS DEFLECTION TRACK OR DEFLECTION CLIPS EQUIVALENT TO 'VERTITRACK' OR 'VERTICUP' FROM THE STEEL NETWORK.

7. **WOOD FRAMING**

- 7.1. ALL STRUCTURAL WOOD MEMBERS SHALL BE No. 2 SOUTHERN YELLOW PINE, 19% MAXIMUM MOISTURE CONTENT, UNLESS OTHERWISE NOTED. INTERIOR NON BEARING PARTITIONS MAY BE No. 2 SPRUCE (SPF).
- 7.2. ALL WOOD FRAMING, DIRECTLY EXPOSED TO WEATHER, OR IN DIRECT CONTACT WITH MASONRY, SOIL OR CONCRETE, SHALL BE PRESSURE TREATED, UNLESS OTHERWISE NOTED.
- 7.3. ALL LVLS, DIRECTLY EXPOSED TO WEATHER, OR IN DIRECT CONTACT WITH MASONRY, SOIL OR CONCRETE, SHALL BE EXTERIOR GRADE, UNLESS NOTED OTHERWISE.
- 7.4. ALL METAL CONNECTORS SHALL BE HOT DIP GALVANIZED. INSTALL ALL CONNECTORS PER THE MANUFACTURER'S RECOMMENDATIONS. METAL CONNECTOR DESIGNATIONS INDICATED ON PLANS, ARE FOR 'SIMPSON STRONG-TIE' ANCHORS. ANCHORS FROM OTHER MANUFACTURERS MAY BE USED, PROVIDED THEY HAVE EQUIVALENT STRENGTH.
- 7.5. ALL NAILED CONNECTIONS SHALL BE IN ACCORDANCE WITH NORTH CAROLINA STATE BUILDING CODE TABLE 2304.9.1. — FASTENING SCHEDULE, UNLESS OTHERWISE NOTED.
- 7.6. FRAMING CONNECTIONS THAT ARE BOLTED OR SCREWED, SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD.
- 7.7. PROVIDE STUDS AND HEADERS AT ALL EXTERIOR WALLS AND INTERIOR BEARING WALLS AS FOLLOWS, UNLESS OTHERWISE NOTED:
- | OPENING WIDTH   | STUDS                      | HEADER   |
|-----------------|----------------------------|--|
| 0'-0" TO 6'-0"  | 2 KING STUDS, 1 JACK STUD  | (2) 2 x 10 @ 2 x 4 WALL<br>(3) 2 x 10 @ 2 x 6 WALL |
| 6'-1" TO 8'-0"  | 2 KING STUDS, 2 JACK STUDS | (2) 2 x 10 @ 2 x 4 WALL<br>(2) 2 x 10 @ 2 x 6 WALL |
| 8'-1" TO 12'-0" | 3 KING STUDS, 2 JACK STUDS | (2) 2 x 12 @ 2 x 4 WALL<br>(3) 2 x 12 @ 2 x 6 WALL |

8. **WOOD DECKING/SHEATHING**

- 8.1. WALL SHEATHING SHALL BE ½" PLYWOOD OR ORIENTED STRAND BOARD (OSB), UNLESS OTHERWISE NOTED. ATTACH WALL SHEATHING TO FRAMING WITH 10d NAILS AT 4" O.C. AT PANEL EDGES AND 12" O.C. AT INTERIOR MEMBERS. PROVIDE SOLID BLOCKING AT PANEL EDGES (48" O.C.).
- 8.2. ROOF SHEATHING SHALL BE ½" PLYWOOD OR ORIENTED STRAND BOARD (OSB), UNLESS OTHERWISE NOTED. ATTACH ROOF SHEATHING TO FRAMING WITH 8d NAILS AT 4" O.C. AT PANEL EDGES AND 12" O.C. AT INTERIOR MEMBERS.
- 8.3. SUB-FLOOR SHALL CONSIST OF ¾" TONGUE AND GROOVE PLYWOOD UNLESS OTHERWISE NOTED. FASTEN WITH 8d NAILS AT 6" O.C. AT PANEL EDGES, AND AT 12" O.C. AT INTERIOR SUPPORTS. THE FLOOR SHEATHING TO COORDINATE WITH UL FLOOR CEILING ASSEMBLY AS OUTLINED IN 'G' SERIES.



STANDARD HOOKS IN TENSION (PER ACI 318-02)				
HOOK DEVELOPMENT LENGTH Ldh (INCHES)				
BAR SIZE	f'c 3000 psi	f'c 4000 psi	f'c 5000 psi	
#3	9	7	7	
#4	11	10	9	
#5	14	12	11	
#6	17	15	13	
#7	19	17	15	
#8	22	19	17	
#9	25	22	19	
#10	28	24	22	
#11	31	26	24	

- NOTES:
- CONCRETE IS NORMAL WEIGHT CONCRETE. IF LIGHTWEIGHT CONCRETE IS USED, MULTIPLY LENGTHS IN TABLE BY 1.3.
  - BAR YIELD STRENGTH (fy) IS 60 KSI.
  - SIDE COVER REQUIREMENTS OF ACI SECTION 25.4.3.2 ARE ASSUMED TO NOT BE MET.
  - THE OR STIRRUP REQUIREMENTS OF ACI SECTION 25.4.3.2 ARE ASSUMED TO NOT BE MET.
  - REDUCTION OF EXCESS REINFORCEMENT IS NOT TAKEN.
  - HOOK DEVELOPMENT LENGTH IS VALID FOR 180° HOOKS ALSO.
- db = BAR DIAMETER

CONCRETE MATERIALS SCHEDULE		
LOCATION	MINIMUM COMPRESSIVE STRENGTH (AT 28 DAYS)	REMARKS
FOUNDATIONS	3000 PSI	—
SLAB ON GRADE	4000 PSI	—
EQUIPMENT PADS	3000 PSI	—
CONCRETE FOR MASONRY CORES, BOND BEAMS	3000 PSI	ASTM C476 GROUT
MISCELLANEOUS	3000 PSI	—

CONCRETE REBAR SPLICE SCHEDULE			
BAR SIZE	LAP LENGTH (in.)		
	f'c = 3000 psi	f'c = 4000 psi	f'c = 5000 psi
#4	22	19	17
#5	28	24	21
#6	32	29	26
#7	48	42	37
#8	55	48	43
#9	62	54	48
#10	68	60	53
#11	76	66	59

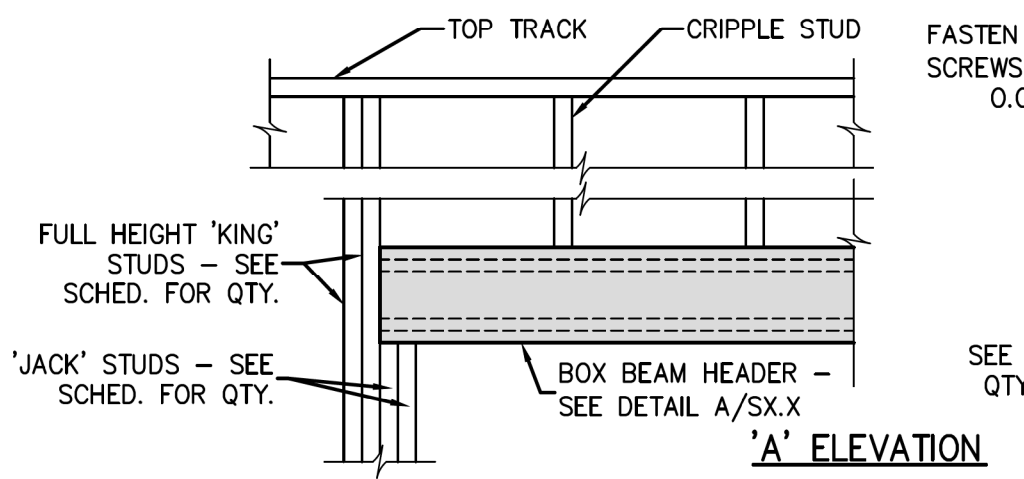
- NOTES:
- CONCRETE IS NORMAL WEIGHT CONCRETE. IF LIGHTWEIGHT CONCRETE IS USED, MULTIPLY LENGTHS IN TABLE BY 1.3.
  - BAR YIELD STRENGTH (fy) IS 60 KSI.
  - BAR SPACING AND COVER REQUIREMENTS OF ACI SECTION 25.4.2.2 ARE ASSUMED TO BE MET. IF NOT, MULTIPLY LENGTHS IN TABLE BY 1.5.
  - REDUCTION OF EXCESS REINFORCEMENT NOT TAKEN.
  - IF MORE THAN 12" OF FRESH CONCRETE IS CAST IN MEMBER BELOW HORIZONTAL SPLICE, MULTIPLY LENGTHS IN TABLE BY 1.3.

EXPOSED CONCRETE FINISH SCHEDULE		
AREA	FINISH	COMMENTS
EXTERIOR CONCRETE PAVEMENT, SIDEWALKS	COARSE BROOM	SEE NOTE 1
SLAB ON GRADE	TROWEL	SEE NOTE 1
EXT. EQUIP. PADS	COARSE BROOM	SEE NOTE 1
—	—	—

- NOTES:
- SEE SPECIFICATIONS SECTION, '033000 CAST-IN-PLACE CONCRETE', FOR ADDITIONAL INFORMATION.

MASONRY VENEER LINTEL SCHEDULE		
OPENING DIMENSION	ANGLE SIZE	ORIENTATION
0'-0" THRU 4'-0"	L 4 x 4 x ¾	N/A
4'-1" THRU 6'-0"	L 4 x 4 x ¾	N/A
6'-1" THRU 8'-0"	L 6 x 4 x ¾	LLV
8'-1" THRU 10'-0"	L 6 x 4 x ¾	LLV
10'-1" THRU 12'-0"	L 7 x 4 x ¾	LLV
—	—	—

- NOTES:
- PROVIDE LOOSE STEEL ANGLE LINTELS FOR ALL MASONRY VENEER OPENINGS, PER ABOVE DATA UNLESS NOTED OTHERWISE.
  - PROVIDE MINIMUM OF 8" BEARING FOR ALL LINTELS UNLESS NOTED OTHERWISE.
  - COORDINATE HORIZONTAL LEG SIZE WITH ARCHITECTURAL DRAWINGS. SIZES MAY NEED TO BE CHANGED TO ACCOMMODATE AIR SPACE, INSULATION AND OTHER WALL COMPONENTS.



BOX HEADER BEAM SCHEDULE			
OPENING WIDTH	JACK STUD QUANTITY	KING STUD QUANTITY	HORIZ. MEMBER QTY. & CONFIG.
0'-0" TO 4'-0"	1	2	(3) 6" x 16 GA. OR (2) 8" x 16 GA.
4'-1" TO 8'-0"	2	3	(3) 8" x 16 GA. OR (2) 10" x 16 GA.
8'-1" TO 12'-0"	3	4	(3) 10" x 16 GA.

- NOTES:
- USE INFORMATION IN THIS SCHEDULE UNLESS OTHERWISE NOTED ON PLAN.
  - SEE GENERAL STRUCTURAL NOTES, SECTION FOR 'COLD FORMED METAL FRAMING' FOR ADDITIONAL INFORMATION.

1 TYP. METAL STUD BOX BEAM DETAIL  
S4.01 3/4" = 1'-0"

STRUCTURAL DESIGN CRITERIA:

1. **DESIGN LOADS:**
- 1.1. ROOF DEAD LOAD MAX MIN (FOR UPLIFT)
- ROOF MEMBRANE & INSULATION 4 PSF 2 PSF
- SHEATHING 3 PSF 2 PSF
- ROOF FRAMING 5 PSF 3 PSF
- PIPING, DUCT, ETC. 3 PSF 0 PSF
- 15 PSF 7 PSF
- 1.2. **LIVE LOADS**
- ROOF LIVE LOAD – ALL AREAS GREATER OF 20 PSF MINIMUM OR SNOW LOAD. LIVE LOAD REDUCTION CAN BE USED IN ACCORDANCE WITH 2018 NBCB, SECTION 1607.10
- 1ST FLOOR LIVE LOAD 100 PSF
- 2ND FLOOR LIVE LOAD 40 PSF (OFFICE)
- 1.3. **SNOW LOAD**
- GROUND SNOW LOAD = 10 PSF (MAYSVILLE, NC)
- SNOW LOAD IMPORTANCE FACTOR: I = 1.25
- SNOW EXPOSURE FACTOR = 1.0
- SNOW THERMAL FACTOR = 1.0
- ROOF SNOW LOAD = 10 PSF
- BASIC DESIGN ROOF SNOW LOAD = 12.5 PSF
- 1.4. **WIND LOAD**
- BASIC WIND SPEED: Vult = 150 MPH (MAYSVILLE, NC)
- RISK CATEGORY: I II III IV
- WIND EXPOSURE CATEGORY: 'B' (ASCE 7-10)
- WIND BASE SHEAR (FOR MWFRS): Vx = K Vy = K
- INTERNAL PRESSURE COEFFICIENT: ±0.55
- 1.5. **SEISMIC LOADS (N.C. STATE BLDG. CODE):**
- SEISMIC IMPORTANCE FACTOR: I = 1.0
- RISK CATEGORY: I II III IV
- SEISMIC DESIGN CATEGORY: A B C D
- MAPPED SPECTRAL RESPONSE ACCELERATION: Ss 13.7 % g S1 6.7 % g
- SPECTRAL RESPONSE COEFFICIENTS: Sps 14.6 % Sps 10.6 %
- SEISMIC RESPONSE COEFFICIENT: Cs
- RESPONSE MODIFICATION FACTOR, R (-----)
- SITE CLASSIFICATION: A B C X D E F
- BASIC STRUCTURAL SYSTEM:
- BEARING WALL DUAL w/ SPECIAL MOMENT FRAME
- BUILDING FRAME DUAL w/ INTERMEDIATE R/C OR SPECIAL STEEL
- X MOMENT FRAME INVERTED PENDULUM
- SEISMIC BASE SHEAR Vx = K Vy = K
- ANALYSIS PROCEDURE: SIMPLIFIED X EQUIVALENT LATERAL FORCE MODAL
- LATERAL DESIGN CONTROL: EARTHQUAKE WIND
- ALL DESIGN LOADS ARE PER NORTH CAROLINA STATE BUILDING CODE 2018 EDITION.
- WIND LOADS CONTROL THE LATERAL LOAD DESIGN. THE BUILDING UTILIZES SHEAR WALLS FOR LATERAL LOAD RESISTANCE.
2. **FOUNDATION DESIGN CRITERIA:**
- 2.1. MINIMUM FOOTING BEARING DEPTH BELOW GRADE IS 12 INCHES.
- 2.2. FOUNDATION DESIGN IS BASED ON A PRESUMPTIVE MAXIMUM ALLOWABLE SOIL BEARING CAPACITY OF 1,500 PSF.
- 2.3. CONTRACTOR SHALL FIELD VERIFY THE SOIL BEARING CAPACITY PRIOR TO START OF CONSTRUCTION.

NOTE:  
SEE PRE-ENGINEERED METAL BUILDING DRAWINGS FOR INFORMATION NOT SHOWN.



MAYSVILLE FIRE STATION  
603 4TH STREET  
MAYSVILLE, NC 28555

THESE DRAWINGS AND THE ACCOMPANYING SPECIFICATIONS ARE INSTRUMENTS OF SERVICE AND SHALL BE USED IN CONJUNCTION WITH THE ARCHITECT'S PREPARED FOR A SPECIFIC PROJECT AND SHALL NOT BE USED IN CONJUNCTION WITH ANY OTHER PROJECT WITHOUT THE WRITTEN PERMISSION OF THE ARCHITECT.  
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REVISIONS:

#	DESC:	DATE
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DRAWN BY: RPA/GBP RPA/MSR  
PROJECT #: 24008  
ISSUE DATE: 04.30.2025  
PHASE:  
CONSTRUCTION DOCUMENTS

SHEET NAME & NUMBER

FRAMING SECTIONS & DETAILS

S4.01



Digitally signed by Mark S. Roy  
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cn=Mark S. Roy  
Date: 2025.05.12 13:07:22-0400

RPA ENGINEERING, P.A.	
Structural Engineering Solutions	
Engineering License Certificate No. C-2734	
1 Commerce Square, Suite 202 Washington, NC 27889	Phone: 252-321-6027 Fax: 252-355-2179
RPA Project No.: 2024223	



Project: MAYSVILLE FIRE STATION  
Location: 603 4TH STREET, MAYSVILLE, NC

Design Professional In Responsible Charge: *Mark S. Roy, PE*

The Special Inspection Coordinator shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.

A Final Report of Special Inspections documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and Occupancy.

Interim Report Frequency:    ☐ Weekly        ☐ Quarterly        ☒ Monthly        ☐ Per Attached Schedule

Mark S. Roy, PE

05-12-2025  
Date

Building Official's Acceptance: \_\_\_\_\_

---

Signature
Date

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems

<input checked="" type="checkbox"/> Soils and Foundations	<input type="checkbox"/> Spray Fire Resistant Material
<input checked="" type="checkbox"/> Cast-in-Place Concrete	<input checked="" type="checkbox"/> Wood Construction
<input type="checkbox"/> Precast Concrete	<input checked="" type="checkbox"/> Exterior Insulation and Finish Systems
<input checked="" type="checkbox"/> Masonry	<input type="checkbox"/> Mechanical & Electrical Systems
<input type="checkbox"/> Structural Steel	<input type="checkbox"/> Architectural Systems
<input checked="" type="checkbox"/> Cold-Formed Steel Framing	<input type="checkbox"/> Special Cases

### Qualifications of Inspectors and Testing Technicians

**Key for Minimum Qualifications of Inspection Agents:**

SE/SE	Structural Engineer - a licensed SE or PE specializing in the design of building structures
PE/GE	Geotechnical Engineer - a licensed PE specializing in soil mechanics and foundations
EIT	Engineer-In-Training - a graduate engineer who has passed the Fundamentals of Engineering examination

ACI-CFTT	Concrete Field Testing Technician - Grade 1
ACI-CCI	Concrete Construction Inspector
ACI-LTT	Laboratory Testing Technician - Grade 1&2
ACI-STT	Strength Testing Technician

AWS-CWI	Certified Welding Inspector
AWS/AISC-SSI	Certified Structural Steel Inspector

ASNT Non-Destructive Testing Technician - Level II or III.

CC-SMSI	Structural Masonry Special Inspector
CC-SWSI	Structural Steel and Welding Special Inspector
CC-SFSI	Spray-Applied Fireproofing Special Inspector
CC-PCSI	Prestressed Concrete Special Inspector
CC-RCSI	Reinforced Concrete Special Inspector

NICET-CT	Concrete Technician - Levels I, II, III & IV
NICET-ST	Soils Technician - Levels I, II, III & IV
NICET-GET	Geotechnical Engineering Technician - Levels I, II, III & IV

## EDI-EIFS EIFS Third Party Inspector

### Other

## Quality Assurance for Seismic Resistance

Seismic Design Category	C
Quality Assurance Plan Required (Y/N)	N

## Quality Assurance for Wind Requirements

Basic Wind Speed (3 second gust)	150 mph
Wind Exposure Category	B
Quality Assurance Plan Required (Y/N)	N

## Statement of Responsibility

Each contractor responsible for the construction or fabrication of a system or component designated above must submit a Statement of Responsibility.

1. The inspection and testing agencies shall be engaged by the owner's agent and not by the contractor or sub-contractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.
2. Each Special Inspection Agency shall submit a Final Report of Special Inspections to the Special Inspection Coordinator at the conclusion of the project.

## SOILS AND FOUNDATIONS

### CAST-IN-PLACE CONCRETE

## COLD-FORMED STEEL FRAMING

Digitally signed by Mark S. Roy  
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# MAYSVILLE FIRE STATION

603 4TH STREET  
MAYSVILLE, NC 28555

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# SPECIAL INSPECTION REPORT

**\$4.02**