

## DESIGN CRITERIA

LOCATION: BAYBORO, NORTH CAROLINA  
BUILDING CODE: 2018 NORTH CAROLINA STATE BUILDING CODE (2015 IBC WITH NORTH CAROLINA AMENDMENTS)

RISK CATEGORY III  
BASIC LATERAL FORCE RESISTING SYSTEM:  
ORDINARY REINFORCED MASONRY SHEAR WALLS

DESIGN LIVE LOADS	
ROOF	20 PSF
CLASSROOMS	60 PSF
FIRST FLOOR CORRIDORS	100 PSF
STORAGE	125 PSF
MECHANICAL	150 PSF
SECOND FLOOR CORRIDORS	80 PSF

DESIGN SUPERIMPOSED DEAD LOADS	
ROOF	10 PSF (INCLUDING 10 PSF SOLAR ALLOWANCE - ALTERNATE NO. 3)
CLASSROOMS	20 PSF
ROOF SNOW LOAD	
$P_g$	= 10 PSF
$C_e$	= 0.9
$I_e$	= 1.1
$C_s$	= 1.0
	15 PSF

**RAIN ON SNOW**  
**WIND LOAD**  
 $V = 143$  MPH (3 SECOND GUST)  
EXPOSURE C  
DESIGN (DESIGNULTIMATE) WIND BASE SHEAR:  
 $V_1 = 350k$   $V_2 = 700k$   
INTERNAL PRESSURE COEFFICIENT = 40.18  
COMPONENTS & CLADDING PER ASCE 7 FIGURES 6-3

WIND LOADS ON COMPONENTS & CLADDING FOR GIVEN TRIBUTARY AREAS (psf)					
	ZONE	10 SQ FT	20 SQ FT	50 SQ FT	500 SQ FT
ROOF	1	+19.5/-47.9	+18.3/-46.7	+16.7/-45.1	+16.0/-43.9
	2	+43.9/-80.5	+41.9/-71.9	+39.4/-60.6	+37.4/-52.0
	3	-43.9/-80.5	+41.9/-71.9	+39.4/-60.6	+37.4/-52.0
ROOF DRAINAGE	2	-81.5	-81.5	-81.5	-81.5
	3	-81.5	-81.5	-81.5	-81.5
WALL	4	+43.9/-47.5	+41.9/-45.6	+39.4/-43.0	+37.9/-41.1
	5	+43.9/-58.5	+41.9/-54.6	+39.4/-49.5	+37.4/-45.6

- DETERMINE WIND LOADS ON COMPONENTS IN ACCORDANCE WITH THE NCSCB AND ASCE 7 OR WITH THIS TABLE. REFERENCE ASCE 7-10 CHAPTER 30. TRIBUTARY AREA = GREATER OF  $LW$  OR  $LX/3$ .
- DESIGN FOR STRENGTH USING LOADS FROM ASCE 7 OR FROM THIS TABLE. FOR SERVICEABILITY AND DEFLECTION REQUIREMENTS, REFER TO ASCE 7-10 APPENDIX C.
- POSITIVE PRESSURES ARE DIRECTED TOWARD THE INTERIOR. NEGATIVE LOADS ARE DIRECTED AWAY FROM THE INTERIOR. NEGATIVE ROOF LOADS ARE UPLIFT LOADS.
- NET UPLIFT SHALL BE CALCULATED USING A RELIABLE ROOF DEAD LOAD OF 10 PSF AND THE APPLICABLE LOAD COMBINATIONS PER ASCE 7-10.

**SEISMIC CRITERIA**  
SEISMIC DESIGN VALUES DETERMINED UTILIZING 2008 USGS HAZARD DATA  
SPECTRAL RESPONSE ACCELERATIONS  
SITE CLASS D  $S_{a1} = 0.115$   $S_{a2} = 0.059$   
SPECTRAL RESPONSE COEFFICIENTS  $D_5 = 0.123$   $S_{d1} = 0.094$   
SEISMIC DESIGN CATEGORY B  
DESIGN ULTIMATE SEISMIC BASE SHEAR:  $V_1 = 1070k$   $V_2 = 1070k$   
IMPORTANCE FACTOR  $I_p = 1.25$   
DESIGN SEISMIC RESPONSE COEFFICIENT  $C_s = 0.077$   
RESPONSE MODIFICATION FACTOR  $R = 2.0$

### SPECIAL INSPECTION REQUIREMENTS

THE FOLLOWING SYSTEMS ARE SUBJECT TO THE SPECIAL INSPECTION REQUIREMENTS OF THE NCSCB, CHAPTER 17.

- RAMMED AGGREGATE PIERS
- CAST-IN-PLACE CONCRETE FOUNDATIONS
- ELEVATED CAST-IN-PLACE CONCRETE SLABS
- CONCRETE MASONRY (LOAD-BEARING OR LATERAL-LOAD RESISTING)
- STRUCTURAL STEEL
- STEEL DECK
- PRECAST CONCRETE HOLLOW CORE PLANKS
- STEEL JOISTS
- SPRAY FIRE RESISTANT MATERIALS

## GENERAL NOTES

### GENERAL

- DESIGN, FURNISH, AND INSTALL TEMPORARY SHORING, BRACING, AND OTHER TEMPORARY SUPPORTS REQUIRED FOR CONSTRUCTING THE STRUCTURE AND TO MAINTAIN THE STABILITY THROUGHOUT ALL PHASES OF CONSTRUCTION UNTIL THE STRUCTURE IS COMPLETED. ALL TEMPORARY SUPPORTS ARE TO BE REMOVED UNLESS NOTED OTHERWISE.
- USE STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND THE DRAWINGS OF OTHER TRADES.
- COORDINATE WITH OTHER TRADES THE ACTUAL LOCATIONS AND SIZES OF OPENINGS AND PENETRATIONS REQUIRED BY THEIR WORK.
- COORDINATE WITH OTHER TRADES THE ACTUAL LOCATIONS AND ELEVATIONS OF BURIED SERVICES PASSING NEAR FOUNDATIONS. UNDERGROUND SERVICES WHICH PASS BENEATH WALL FOOTINGS SHALL HAVE AT LEAST 12" OF CLEARANCE BELOW THE BOTTOM OF THE FOOTING. WHERE THIS IS NOT ACHIEVED, EITHER STEP THE FOOTING DOWN BENEATH THE SERVICE OR INSTALL A STEEL PIPE SLEEVE FOR THE SERVICE TO PASS THROUGH. SLEEVES ARE FURNISHED AND INSTALLED BY THE TRADE INSTALLING THE SERVICE. NO SERVICE IS TO BE INSTALLED BENEATH COLUMN FOOTINGS UNLESS APPROVED BY THE ARCHITECT.
- COORDINATE WITH OTHER TRADES THE ACTUAL LOCATIONS AND TYPES OF ATTACHMENTS AND ANCHORS THAT ARE REQUIRED BY THE TRADES TO FASTEN THEIR WORK TO THE STRUCTURE.
- MODIFICATIONS TO STRUCTURAL COMPONENTS AND INSTALLATION OF PENETRATIONS THROUGH STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT PRIOR APPROVAL OF THE ARCHITECT.
- VERIFY ACTUAL DIMENSIONS, ELEVATIONS, AND CONDITIONS OF EXISTING CONSTRUCTION PRIOR TO PROCEEDING WITH WORK OR ORDERING MATERIALS WHICH COULD BE AFFECTED BY EXISTING CONDITIONS.

### FOUNDATIONS

- THE IN-SITU SOIL ALLOWABLE BEARING PRESSURE IS 1,500 PSF PER THE GEOTECHNICAL ENGINEERING REPORT PREPARED BY ECS SOUTHEAST, LLP, AND DATED APRIL 18, 2023.
- THE BUILDING WALL FOOTINGS AND COLUMNS FOOTINGS HAVE BEEN DESIGNED BASED ON AN ASSUMED EQUIVALENT BEARING CAPACITY OF 3,000 PSF. THE BUILDING AREA SHALL BE SUPPORTED ON GROUND IMPROVEMENT BY RAMMED AGGREGATE PIERS/STONE COLUMNS.
- ALL COLUMN AND WALL FOOTINGS AND SLAB-ON-GRADE SHALL BE BACKFILLED WITH PROPERLY PLACED AND COMPACTED STRUCTURAL FILL IN ORDER TO ACHIEVE THE REQUIRED BEARING CAPACITY. USE OF GEOTEXTILES AND SELECT GRANULAR FILL MAY BE RECOMMENDED BY THE GEOTECHNICAL ENGINEER TO REDUCE REQUIRED UNDERCUT DEPTHS.

### FOUNDATIONS (CONT.)

- ALL STRUCTURAL EARTH FILL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 8 INCHES AND BE COMPACTED TO AT LEAST 98 PERCENT OF THE SOIL'S STANDARD PROCTOR OR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698. THE TOP 12 INCHES OF FILL IN LOAD BEARING AREAS SHOULD BE COMPACTED TO AT LEAST 98 PERCENT OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL STRUCTURAL FILL MATERIAL SHALL BE COMPACTED AT A MOISTURE CONTENT WITHIN 3 PERCENT OF THE SOIL'S OPTIMUM MOISTURE CONTENT (AS DETERMINED BY ASTM D-698). ALL STRUCTURAL FILL SHALL BE PLACED UNDER THE FULL CONTROL OF AN ENGINEERING TECHNICIAN WORKING UNDER THE DIRECTION OF A GEOTECHNICAL ENGINEER. THE PLACEMENT AND COMPACTION OF ALL FILL MATERIAL SHALL BE MONITORED AND TESTED IN ORDER TO CONFIRM THAT THE RECOMMENDED DEGREE OF COMPACTION IS BEING OBTAINED. IF AN IMPORTED STRUCTURAL FILL IS REQUIRED TO COMPLETE SITE GRADING, IT SHALL BE APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER PRIOR TO USE. IMPORTED STRUCTURAL FILL SHOULD TYPICALLY CONSIST OF LOW PLASTICITY SOIL (LL<50, PI<25). HAVE A STANDARD PROCTOR MAXIMUM DRY DENSITY OF AT LEAST 100 PCF, AND BE FREE OF ORGANIC AND OTHER DELETERIOUS MATERIALS. IF CLEAN SAND FILL IS NECESSARY TO REPLACE LOWER CONSISTENCY SOILS IN THE BUILDING AREA, THE SAND SHOULD CONTAIN LESS THAN 10 TO 12 PERCENT FINES.
- FINISHED SUBGRADES IN BUILDING AREAS RECEIVING MORE THAN 7 FEET OF FILL SHALL BE MONITORED FOR SETTLEMENT DUE TO THE FILL LOADING. SETTLEMENT MONUMENTS SHOULD BE INSTALLED AT THE TOP OF THE FILL IMMEDIATELY UPON FILL COMPLETION WITH SETTLEMENT MEASUREMENTS TAKEN AT LEAST TWO PER WEEK UNTIL SETTLEMENTS HAVE STABILIZED. SETTLEMENTS IN BUILDING FOUNDATIONS AND PAVEMENTS SHALL NOT OCCUR UNTIL IT IS CONFIRMED THAT SETTLEMENT DUE TO NEW FILL HAS STABILIZED.
- NO FOUNDATIONS SHALL BE PLACED IN WATER OR ON FROZEN GROUND.
- ALL FOOTING EXCAVATIONS ARE TO BE FINISHED BY HAND.
- ALL FINISHED FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE ARCHITECT OR HIS DESIGNATE BEFORE ANY CONCRETE IS PLACED.
- UNLESS OTHERWISE NOTED, ALL FOOTINGS AND PILASTERS SHALL BE CENTERED UNDER SUPPORTED MEMBERS.
- DOWELS FROM FOUNDATIONS INTO PIERS, COLUMNS, BUTTRESSES, OR WALLS ABOVE SHALL BE THE SAME SIZE AND NUMBER AS VERTICAL REINFORCEMENT IN PIERS, COLUMNS, BUTTRESSES, OR WALLS ABOVE, EXCEPT AS OTHERWISE SHOWN ON THE DRAWINGS. CAREFULLY FOLLOW THE REQUIREMENTS OF THE SPECIFICATIONS FOR BACKFILL UNDER OR ADJACENT TO ANY PORTION OF THE BUILDING.
- WHERE FOUNDATION ELEMENTS ARE TO HAVE FILL ON BOTH SIDES, EACH SIDE SHALL BE FILLED SIMULTANEOUSLY, MAINTAINING A COMMON ELEVATION.
- COORDINATE UNDERFLOOR DRAIN REQUIREMENTS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND THE REQUIREMENTS OF THE GEOTECHNICAL ENGINEER.
- CONTRACTOR SHALL PROVIDE CONTINUOUS CONTROL OF SURFACE AND UNDERGROUND WATER AS REQUIRED DURING CONSTRUCTION SUCH THAT THE WORK IS DONE IN THE DRY.

### STRUCTURAL PRECAST CONCRETE

- HOLLOW CORE PLANKS SHALL BE NORMAL WEIGHT CONCRETE AND SHALL TEST MINIMUM 5,000 PSI AT 28 DAYS. PRESTRESSING STRANDS SHALL BE #7 WIRE, 270K STRANDS TENSIONED TO 30,881 LB EA. UNO. PRESTRESSING AND FABRICATION SHALL CONFORM TO THE STANDARDS OF ACI 318, 2014 EDITION.
- PRECAST CONCRETE DESIGN SHALL CONFORM TO PRESTRESSED CONCRETE INSTITUTE DESIGN HANDBOOK, 7th EDITION.
- PRECAST SHALL BE DESIGNED FOR THE SUPERIMPOSED LOAD INDICATED IN THE DESIGN CRITERIA, INITIAL ERECTION AND HANDLING STRESSES AND 15 PSF FOR MEP ATTACHMENTS AND EQUIPMENT.
- PRECAST CONSTRUCTION SHALL PROVIDE A MINIMUM 2-HOUR FIRE RATING AS PER GOVERNING BUILDING CODE.
- THE DESIGN AND DETAILING OF ALL PRECAST CONCRETE AND THEIR CONNECTIONS SHALL BE THE RESPONSIBILITY OF THE PRECAST FABRICATOR. THIS SHALL INCLUDE ALL BOLTS, PLATES, BRACES, REINFORCING, WELD SIZES, AND ANY EMBEDDED ITEMS IN BOTH PRECAST AND THE STRUCTURAL FRAME FOR ALL CONNECTIONS BETWEEN THE PRECAST AND THE SUPPORT STRUCTURE. ALL CONNECTIONS SHOWN ON THE DRAWINGS ARE CONCEPTUAL. IN NATURE AND ARE FOR BIDDING PURPOSES ONLY.
- THE PRECAST DESIGN SHALL BE PERFORMED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. THE DESIGN SHALL CONFORM TO THE GOVERNING BUILDING CODE, THE PCI DESIGN MANUAL, AND ASCE 7-10. DESIGN CALCULATIONS, STAMPED BY THE REGISTERED ENGINEER, SHALL BE SUBMITTED FOR REVIEW BY THE ENGINEER OF RECORD. THE PRECAST FABRICATOR SHALL DESIGN THE ENTIRE STRUCTURAL SYSTEM FOR ALL GRAVITY AND LATERAL LOADS SHOWN ON THESE DRAWINGS AND REQUIRED BY THE GOVERNING BUILDING CODE.
- DESIGN MODIFICATIONS MAY BE MADE ONLY AS NECESSARY TO MEET FIELD CONDITIONS AND TO ENSURE PROPER FITTING OF THE WORK, AND ONLY AS ACCEPTABLE TO THE ENGINEER OF RECORD. MAINTAIN GENERAL DESIGN CONCEPT SHOWN WITHOUT INCREASING OR DECREASING SIZES OF MEMBERS OR ALTERING PROFILES AND ALIGNMENT SHOWN.
- REFER TO THE ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS FOR LOCATIONS OF ALL OPENINGS, SLEEVES, AND PENETRATIONS IN THE STRUCTURE. ALL OPENINGS, PLANT-CAST OR FIELD-OUT, MUST BE REVIEWED AND APPROVED BY THE PRECAST FABRICATOR'S ENGINEER. SUBMIT WALL SHOP DRAWINGS NOTING REQUIRED OPENINGS FOR REVIEW. MECHANICAL SUBCONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT.
- PRE-STRESSED CONCRETE MANUFACTURER MUST DESIGN OPENINGS WHICH REQUIRE HANGER TYPE SUPPLEMENTARY STEEL MEMBERS. CONCRETE STRENGTH TESTING OF HOLLOW CORE PLANKS:
  - TEST CONCRETE COMPRESSIVE STRENGTHS OBTAINED FROM THE CONCRETE BEING USED IN THE INSTALLED PANELS IN ACCORDANCE WITH PCI MNL-116.
  - PROVIDE NON-LEACHING MULTIMONOMER PLASTIC STRIP BEARING PADS CAPABLE OF SUPPORTING CONSTRUCTION LOADS WITH NO VISIBLE OVERALL EXPANSION.
  - INSTALL A CONCRETE TOPPING SLAB WITH A 28-DAY COMPRESSIVE STRENGTH OF 3500 PSI.
- ERECTION:
  - ERECT PRE-STRESSED CONCRETE UNITS INTO FINAL POSITION UNDER THE SUPERVISION OF THE MANUFACTURER OR AN ERECTOR EXPERIENCED IN INSTALLATION OF THE TYPES OF UNITS SPECIFIED ON THE CONTRACT DOCUMENTS.
  - ALIGN AND LEVEL PRE-STRESSED CONCRETE UNITS IN ACCORDANCE WITH THE REQUIREMENTS AND TOLERANCES OF PCI MNL-116 PRIOR TO GROUTING KEY WAY JOINTS. GROUT KEYWAYS PRIOR TO UNITS BEING LOADED OR ENDS RESTRAINED.
  - CLEAN AND FILL ALL KEYWAYS BETWEEN UNITS SOLIDLY WITH GROUT (MINIMUM 3000 PSI GROUT STRENGTH). REMOVE GROUT SEEPAGE PRIOR TO HARDENING.
  - PATCH DAMAGE AND HOLES IN HOLLOW-CORE SLAB UNITS TO MAINTAIN THE UNITS' STRUCTURAL ADEQUACY.
- TOPPING SLAB:
  - CLEAN AND WET PLANKS PRIOR TO PLACING CONCRETE TOPPING SLAB.
  - INSTALL A TOPPING SLAB WITH A MINIMUM DEPTH OF 2" UNO ON THE PLANS.
  - SEE ARCHITECTURAL DRAWING FOR ADDITIONAL TOPPING THICKNESS REQUIRED FOR SLAB DRAINAGE.

### CAST-IN-PLACE CONCRETE

- MATERIALS:
  - PORTLAND CEMENT: ASTM C150, TYPE I.
  - PLY ASH: ASTM A618, CLASS C OR F.
  - NORMAL-WEIGHT AGGREGATE: ASTM A588 C33, CLASS 3M.
  - LIGHTWEIGHT AGGREGATE EXPANDED SHALE OR SLATE: ASTM C330.
  - REINFORCING STEEL: ASTM A615 GRADE 60.
  - REINFORCING STEEL, WELDABLE: ASTM A706.
  - WELDED WIRE FABRIC: ASTM A185, FLAT SHEETS.
  - UNDER-SLAB DRAINAGE FILL: 6" GRANULAR FILL, MAXIMUM AGGREGATE SIZE OF 3/4".
  - VAPOR BARRIER: ASTM E1745, CLASS B, FIVE-PLY, NYLON OR POLYESTER CHORD, 10 MILS THICKNESS.
  - WATERSTOP: SELF EXPANDING.
- CONCRETE MIXES:
  - FOOTINGS 3000 PSI NW.
  - SLABS-ON-GRADE: 3000 PSI NW.
  - SLABS-ON-GRADE EXPOSED TO WEATHER: 4500 PSI NW, AIR-ENTRAINED.
  - SUPPORTED SLABS ON STEEL DECK: 3500 PSI LW (115 PCF).
  - TOPPING SLABS ON HOLLOW CORE: 3500 PSI LW (115 PCF).
- PERFORM CONCRETE WORK IN ACCORDANCE WITH ACI 318 AND ACI 301.

### CAST-IN-PLACE CONCRETE CONTINUED

- PROVIDE CONCRETE COVER AS FOLLOWS:
  - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
  - CONCRETE EXPOSED TO EARTH OR WEATHER:
    - #5 OR SMALLER: 1 1/2"
    - #6 OR LARGER: 2"
  - CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
    - SLABS, WALLS, JOIST: 3/4" BEAMS, COLUMNS: 1 1/2" TO PRIMARY
    - REINFORCEMENT, TIES, STIRRUPS, OR SPIRALS
- PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE. SPLICE ONLY AS SHOWN OR APPROVED. MINIMUM LAP LENGTHS, EXPRESSED IN NUMBER OF BAR DIAMETERS, SHALL BE AS FOLLOWS:

BAR SIZE	NORMAL WT. CONCRETE STRENGTH, $f_c$ (PSI)	4000	5000
#6 OR SMALLER	57 DIA.	49 DIA.	44 DIA.
#7 OR LARGER	71 DIA.	62 DIA.	55 DIA.
- MULTIPLY THE ABOVE LENGTHS BY 1.3 FOR TOP BARS AND BY 1.3 FOR LIGHTWEIGHT CONCRETE. WHERE BARS OF UNEQUAL DIAMETER ARE LAPPED, USE THE LAP LENGTH OF THE SMALLER BAR. THE ABOVE LENGTHS ARE CLASS "B" TENSION LAP SPLICES BASED ON GRADE 60 BARS WITH A COVER OF AT LEAST 1 BAR DIA. AND SPACING AT LEAST 3 BAR DIA. LAP LENGTHS SHALL BE INCREASED IN ACCORDANCE WITH ACI 318 IF COVER IS LESS THAN 1 BAR DIA. OR SPACING IS LESS THAN 3 BAR DIA.
- ACCURATELY INSTALL AND PROPERLY SECURE ANCHORS, BEARING PLATES, SLEEVES, AND OTHER EMBEDDED ITEMS.
- ACCURATELY LOCATE AND BLOCK OUT OPENINGS AND PENETRATIONS.
- COORDINATE WITH OTHER TRADES FOR ANCHORS, EMBEDDED ITEMS, SLEEVES, AND PENETRATIONS REQUIRED AND/OR FURNISHED BY THE OTHER TRADES.
- PROVIDE CONTRACTION JOINTS IN SLABS-ON-GRADE WHERE INDICATED ON THE PLANS. PROVIDE A JOINT DEPTH EQUAL TO AT LEAST 25% OF THE SLAB THICKNESS.
- INSTALL AND SEAL VAPOR BARRIER IN ACCORDANCE WITH ASTM E1643 AND MANUFACTURER'S INSTRUCTIONS. LAP JOINTS 6" AND SEAL WITH MANUFACTURER'S RECOMMENDED TAPE.
- FLOOR FINISHES:
  - FLOAT FINISH: SURFACES TO RECEIVE A TROWEL FINISH, TO BE COVERED WITH FLUID-APPLIED OR SHEET WATERPROOFING, OR TO BE COVERED WITH BUILT-UP OR MEMBRANE ROOFING.
  - TROWEL FINISH: SURFACES EXPOSED TO VIEW OR COVERED WITH RESILIENT FLOORING, CARPET, WOOD FLOORING, PAINT, SEALER, OR OTHER THIN FILM FINISH.
  - TROWEL AND FINE-BROOM FINISH: SURFACES TO BE COVERED WITH QUARRY OR CERAMIC TILE INSTALLED BY THE THIN-SET OR THICK-SET METHOD.
  - BROOM FINISH: EXTERIOR CONCRETE PLATFORMS, STEPS, AND RAMPS.
- FLOOR FINISH TOLERANCE:
  - SLABS TO RECEIVE WOOD FLOORING:
    - SPECIFIED OVERALL VALUES: FF-35 / FL-25.
    - MINIMUM LOCAL VALUES: FF-24 / FL-17.
  - ALL OTHERS RECEIVING TROWEL OR TROWEL AND FINE-BROOM FINISH:
    - SPECIFIED OVERALL VALUES: FF-25 / FL-20.
    - MINIMUM LOCAL VALUES: FF-17 / FL-15.
- FLOOR FRAMING AND FLOOR DECK ARE DESIGNED TO REMAIN UNSHORED DURING CONCRETE PLACEMENT. ACCOUNT FOR AN EXPECTED DEFLECTION IN BEAMS AND GIRDERS OF UP TO 1/360 OF THE SPAN LENGTH (IN INCHES) OR 1 INCH, WHICHEVER IS LESS, WHEN CALCULATING CONCRETE QUANTITIES.
- FINISH SLAB FLAT AND LEVEL.
- NO CONDUIT OR PIPE MAY BE RUN WITHIN STRUCTURAL CONCRETE MEMBERS EXCEPT WHERE INDICATED.

### COLD-FORMED STEEL STRUCTURAL FRAMING

- ENGINEER, FABRICATE, AND INSTALL COLD-FORMED STEEL STRUCTURAL FRAMING FOR THE FOLLOWING:
  - STRUCTURAL TRUSSES AS INDICATED ON ROOF PLANS.
  - ENGINEER, FABRICATE, AND INSTALL PERMANENT AND TEMPORARY BRACING, BRIDGING, CONNECTIONS, AND ANCHORAGES TO THE PRIMARY STRUCTURE FOR THE COMPONENTS LISTED ABOVE.
  - REFER TO ARCHITECTURAL DRAWINGS FOR NON-LOAD-BEARING COLD-FORMED STEEL CURTAIN WALL MEMBERS AND OTHER COLD-FORMED AND LIGHT GAUGE STEEL MEMBERS NOT LISTED ABOVE.
- COMPLY WITH THE FOLLOWING:
  - ANSI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
  - COFSS TECHNICAL BULLETIN: "ANSI SPECIFICATION FOR SCREW CONNECTIONS".
  - ANSI DESIGN GUIDE FOR COLD-FORMED STEEL TRUSSES".
  - AWS D1.3, "STRUCTURAL WELDING CODE, SHEET STEEL".
- STRUCTURAL PERFORMANCE:
  - DEAD, LIVE, AND WIND LOADS AND SEISMIC CRITERIA: SEE GENERAL NOTES ON THIS SHEET.
  - LATERAL LOAD ON INTERIOR LOAD-BEARING WALLS: 5 PSF.
  - HORIZONTAL DEFLECTION, EXTERIOR LOAD-BEARING WALL FRAMING: SPAN/800 FOR MASONRY VENEERS, SPAN/360 OTHERWISE.
  - ALLOW FOR CONSTRUCTION TOLERANCES AND ACCOMMODATE LIVE LOAD DEFLECTIONS OF THE PRIMARY STRUCTURE OF UP TO 3/4 INCH.
  - ASSUME NON-STRUCTURAL SHEATHING PROVIDES NO LATERAL BRACING TO FRAMING MEMBERS.
- SUBMIT COMPLETED DESIGN CALCULATIONS AND ERECTION DRAWINGS SEALED BY A PROFESSIONAL ENGINEER LICENSED IN NORTH CAROLINA, TO THE ARCHITECT FOR REVIEW.
- MATERIALS:
  - COLD-FORMED STEEL: ASTM A653, GRADE 33 UNLESS NOTED OTHERWISE, G60 COATING.
  - MINIMUM UNCOATED-STEEL THICKNESS: 0.0428" FOR ALL MEMBERS, EXCEPT TRUSS MEMBERS.
  - MINIMUM UNCOATED-STEEL THICKNESS, TRUSS MEMBERS: THICKNESS REQUIRED TO SATISFY DESIGN AND CONSTRUCTIBILITY REQUIREMENTS.
  - ANCHOR BOLTS: ASTM F1554, GRADE 36, ZINC-COATED IN ACCORDANCE WITH ASTM A653.
  - EXPANSION AND ADHESIVE ANCHORS: AS INDICATED ELSEWHERE IN THE GENERAL NOTES.
  - POWER-ACTUATED ANCHORS: CORROSION RESISTANT CARBON STEEL, 0.145" MINIMUM DIAMETER.
  - MECHANICAL FASTENERS: CORROSION-RESISTANT-COATED CARBON STEEL, SELF-DRILLING, SELF-THREADING DRILL SCREWS, #10 MIN.
  - WELD FILLER MATERIAL: IN ACCORDANCE WITH AWS D1.3.
- PERFORM WELDING WITH QUALIFIED WELDERS IN ACCORDANCE WITH AWS D1.3.
- INSTALL PERMANENT BRIDGING, BRACING, AND ANCHORAGES TO THE PRIMARY STRUCTURES AS INDICATED ON APPROVED SHOP DRAWINGS.
- INSTALL BENT PLATES AS INDICATED AT RIDGES, HIPS, VALLEYS, EAVES, AND OTHER TRANSITIONS TO PROVIDE ADEQUATE SUPPORT FOR DECKING AND SHEATHING.

### STRUCTURAL STEEL

- MATERIALS:
  - STRUCTURAL STEEL WIDE FLANGE SHAPES: ASTM A992
  - OTHER STRUCTURAL STEEL ROLLED SHAPES: ASTM A36
  - RECTANGULAR OR ROUND HSS: ASTM A500, GR C
  - STEEL PIPE: ASTM A53, GR B
  - STEEL PLATE: ASTM A36
  - HIGH STRENGTH BOLTS: ASTM A325
  - ANCHOR RODS: ASTM F1554, GRADE 36
  - WELD ELECTRODE: IN ACCORDANCE WITH AWS D1.1
- FABRICATE AND ERECT STEEL IN ACCORDANCE WITH THE AISI SPECIFICATION, PERFORM SHOP AND FIELD WELDING IN ACCORDANCE WITH AWS D1.1 WITH CURRENTLY CERTIFIED WELDERS.
- FLOOR FRAMING HAS BEEN DESIGNED TO REMAIN UNSHORED DURING CONCRETE PLACEMENT.
- UNLESS NOTED OTHERWISE, ALL BOLTED CONNECTIONS ARE MADE WITH 3/4" HIGH STRENGTH BOLTS INSTALLED SNUG TIGHT.
- WELD SHEAR CONNECTORS IN ACCORDANCE WITH AWS D1.1 WITH CURRENTLY CERTIFIED WELDERS. REMOVE AND DISCARD ARC SHIELDS AFTER WELDING.
- STEEL PREPARATION AND FINISH:
  - INTERIOR FRAMING: SSPC SP3 POWER TOOL CLEANING; PAINT 23 LATEX PRIMER FOR STEEL SURFACES.
  - EXTERIOR FRAMING: SSPC SP6 COMMERCIAL BLAST CLEANING; PAINT 20 ZINC RICH PRIMER.
  - BRICK RELIEF ANGLES: SSPC SP6 COMMERCIAL BLAST CLEANING; HOT DIPPED GALVANIZED.

### STRUCTURAL STEEL CONTINUED

- FOR BEAMS NOT MEETING THE MINIMUM SIZE REQUIREMENT OF THE AISI ASSEMBLY, THE CONTRACTOR SHALL PROVIDE FOR APPROVAL W/D CALCULATIONS AS REQUIRED IN SECTION 722.5.2.2 OF THE NC STATE BUILDING CODE.
- STRUCTURAL STEEL CONNECTIONS SHALL CONFORM TO THE FOLLOWING, UNO:
  - MINIMUM SIZE OF BOLTS SHALL BE 3/4" DIAMETER AND EACH CONNECTION SHALL HAVE A MINIMUM OF TWO BOLTS WITH ONE HARDENED WASHER PER BOLT. BEARING TYPE CONNECTIONS SHALL BE DESIGNED AS TYPE "N".
  - IN GENERAL, CONNECTIONS SHALL BE FIELD BOLTED AND TIGHTENED TO SNUG TIGHT CONDITION, UNO. ALL BOLTS DESIGNATED "SLIP CRITICAL" OR "FULLY TIGHTENED" SHALL BE TIGHTENED TO THE MINIMUM PRETENSION VALUE SHOWN IN TABLE J3.1 OF THE AISI SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS. IN ADDITION, CONNECTIONS DESIGNATED "SLIP CRITICAL" SHALL HAVE PROPERLY PREPARED FAYING SURFACES TO MEET CLASS A SURFACE CONDITION.
  - "FULLY TIGHTENED" CONNECTIONS SHALL INCLUDE ALL BOLTS IN MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, HANGERS, GIRT CONNECTIONS, BOLTS IN TENSION, CONNECTIONS SUBJECT TO VIBRATION AND ALL A490 BOLTS. DIRECT TENSION INDICATOR (DTI) WASHERS OR TENSION CONTROL BOLTS (TCBs) SHALL BE USED AT THESE CONDITIONS.
  - MINIMUM THICKNESS OF ALL CONNECTION MATERIAL TO BE 5/16", UNO. MINIMUM THICKNESS OF GUSSET PLATES AND SHEAR PLATES TO BE 3/8".
  - UNO IN THE DRAWINGS, MINIMUM NUMBER OF BOLTS REQUIRED IN A BEAM WEB CONNECTION SHALL BE AS FOLLOWS:

BEAM SIZE	MIN NO OF BOLTS
W8 / W10 / W12	2
W14 / W16 / W18	3
W21 / W24	4
W27 / W30	5
W33 / W36	6
W40 / W44	7
  - IN CONNECTIONS OF BEAMS, THE MINIMUM NUMBER OF BOLTS SHALL BE REQUIRED TO DEVELOP THE END REACTION NOTED ON THE CONTRACT DRAWINGS. MINIMUM BEAM END REACTION TO BE USED IS 10 KIPS ASD.
  - CONNECTIONS OF BEAMS SHALL DEVELOP THE BEAM SHEAR END REACTION IN ADDITION TO ANY AXIAL FORCES LISTED ON THE STRUCTURAL DRAWINGS, WHERE APPLICABLE. FORCES SHALL BE CONSIDERED TO ACT SIMULTANEOUSLY.
  - ALL MOMENT CONNECTIONS SHALL DEVELOP THE FULL MOMENT CAPACITY OF THE BEAM, UNO.
  - ALL BRACING CONNECTIONS SHALL DEVELOP THE TENSION/COMPRESSION FORCES NOTED ON THE DRAWING. SPLICE CONNECTIONS ARE NOT NOTED ON THE DRAWINGS. THE BRACING CONNECTION SHALL DEVELOP THE ALLOWABLE TENSION FORCE IN THE MEMBER. BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED SO THAT THE BRACING CONNECTION IS NOT TRANSMITTED DIRECTLY TO THE CENTER OF GRAVITY OF INTERSECTING MEMBERS, WHERE THIS IS NOT POSSIBLE, CONNECTIONS SHALL BE DESIGNED FOR ALL RESULTING FORCES.
- WELD SHEAR CONNECTORS IN ACCORDANCE WITH AWS D1.1 WITH CURRENTLY CERTIFIED WELDERS. REMOVE AND DISCARD ARC SHIELDS AFTER WELDING.
- ALL STEEL EXPOSED TO VIEW SHALL COMPLY WITH THE REQUIREMENTS OF SECTION 10 OF THE "AISC CODE OF STANDARD PRACTICE".

### STEEL DECK

- MATERIALS:
  - COMPOSITE STEEL FLOOR DECK: 3" DEEP, 16 GAGE, ASTM A653, SS, GALVALUME GALVALANIZED COATING.
  - COMPOSITE STEEL FLOOR DECK: 2" DEEP, 16 GAGE, ASTM A653, SS, GRADE 50, G60 GALVANIZED COATING.
  - STEEL ROOF DECK: 1 1/2" DEEP, TYPE B (WIDE RIB), 20 GAGE, ASTM A653, SS, GRADE 50, G60 GALVANIZED COATING.
  - STEEL ROOF DECK: 3" DEEP, TYPE NLA, NON-PASIVATED, 20 GAGE, ASTM A653, SS, GRADE 50, G60 GALVANIZED COATING.
  - POUR STOPS, GIRDERS, FILLERS, COLUMN, END, AND 2 CLOSURES, COVER PLATES, OTHER STEEL SHEET DECKING ACCESSORIES, THICKNESS AS REQUIRED FOR STRENGTH BUT NOT LESS THAN THE DECKING THICKNESS OF THE SAME MATERIAL AND FINISH AS THE DECKING MATERIAL.
  - MECHANICAL FASTENERS: CORROSION RESISTANT SELF-DRILLING CARBON STEEL SCREWS, #10 MINIMUM DIAMETER.
  - WELD ELECTRODE: IN ACCORDANCE WITH AWS D1.3.
- FABRICATE AND ERECT DECK IN ACCORDANCE WITH SDI PUBLICATION NO. 29, PERFORM WELDING IN ACCORDANCE WITH AWS D1.3 WITH CURRENTLY CERTIFIED WELDERS.
- CUT AND NEATLY FIT DECK AROUND OPENINGS AND OTHER WORK PROJECTING THROUGH THE DECK.
- PROVIDE ADDITIONAL SUPPORT AND CLOSURE PIECES AS REQUIRED FOR STRENGTH, CONTINUITY OF DECK, AND SUPPORT OF OTHER WORK.
- DURING DECK PLACEMENT, DECK INSTALLER SHALL MARK LOCATIONS OF STRUCTURE BELOW DECK TO FACILITATE ATTACHMENT TO STRUCTURE.
- COMPOSITE FLOOR DECK ATTACHMENT:
  - AT SUPPORTS, WELD EDGE AND INTERIOR RIBS OF DECK UNITS AT EACH SUPPORT AT AN AVERAGE SPACING OF 12" (18" MAXIMUM) WITH 5/8" NOMINAL DIAMETER PUDDLE WELDS. WELDING WASHERS SHALL BE USED ON ALL DECK UNITS WITH A METAL THICKNESS LESS THAN 0.028 INCHES.
  - AT PERIMETER EDGES BETWEEN SUPPORTS, FASTEN WITH 1 1/2" LONG WELDS SPACED A MAXIMUM OF 36" APART.
  - AT SIDE LAPS, INSTALL #10 SELF DRILLING SCREWS A MAXIMUM OF 36 INCHES APART.
  - END BEARING: 1 1/2" MINIMUM.
  - END JOINTS: BUTTED OR LAPPED.
  - 1 1/2" DEEP ROOF DECK ATTACHMENT TO STRUCTURAL STEEL:
    - FASTEN ROOF DECK PANELS TO STRUCTURAL STEEL SUPPORTING MEMBERS WITH 5/8" NOMINAL DIAMETER PUDDLE WELDS, OR WELDS WITH AN EQUAL PERIMETER, OR SEAM WELDS NOT LESS THAN 1 1/2" LONG.
  - FASTEREN SPACING: SPACE FASTENERS IN 3/6T ATTACHMENT PATTERN.
  - FASTEN SIDE LAPS WITH #10 SELF-DRILLING SCREWS AT 6" OC. END BEARINGS: 1 1/2" MINIMUM.
  - END JOINTS: LAPPED.
  - DO NOT HANG ANYTHING FROM THE ROOF DECK.
  - MECHANICAL FASTENERS OR POWER-DRIVEN FASTENERS MAY BE USED AS APPROVED BY ENGINEER OF RECORD.
- 3" DEEP ROOF DECK ATTACHMENT TO STRUCTURAL STEEL:
  - WELD ROOF DECK PANELS TO COLD-FORMED STEEL SUPPORTING MEMBERS WITH MINIMUM OF 5/8" DIAMETER PUDDLE WELDS OR EQUIVALENT.
  - FASTEREN SPACING: SPACE FASTENERS IN A 3/2S ATTACHMENT PATTERN.
  - FASTEN SIDE LAPS WITH #10 SELF-DRILLING SCREWS AT 4" OC. END BEARINGS: 1 1/2" MINIMUM.
  - END JOINTS: LAPPED.
  - DO NOT HANG ANYTHING FROM THE ROOF DECK.
  - MECHANICAL FASTENERS OR POWER-DRIVEN FASTENERS MAY BE USED AS APPROVED BY ENGINEER OF RECORD.

### POST-INSTALLED ANCHORS

UNLESS OTHERWISE INDICATED ON PLANS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES, OR APPROVED EQUAL:

	ADHESIVE ANCHOR	MECHANICAL ANCHOR
SOLID CONCRETE	HILTI HY 202 HILTI RE 500 SD POWERS AC200+ POWERS PURE110+ SIMPSON SET-XP	HILTI KWIK BOLT T2 POWERS POWER-STUD+SD2 POWERS SCREW-BOLT+ SIMPSON STRONG-BOLT 2
	HILTI HY 270 POWERS AC100+GOLD SIMPSON SET-XP	HILTI KWIK BOLT 3 POWERS POWER-STUD+SD1 SIMPSON STRONG-BOLT 2
HOLLOW MASONRY OR BRICK	HILTI HY 270 W/ SCREEN TUBE POWERS AC100+GOLD SIMPSON SET-XP	HILTI HL-C SLEEVE ANCHOR POWERS LOK-BOLT AS SIMPSON HOLLOW DROP-IN

- SUBSTITUTION REQUESTS FOR ALTERNATIVE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE.
- INSTALL ANCHORS PER THE MANUFACTURED INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGE.
- ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH AIA/CES (ACI 318-11 D.9.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
- ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS (ACI 318-11 D.2.2).
- ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.4 AND ICC-ES AC308 FOR USE IN CONCRETE APPLICATION, OR ICC-ES AC508 FOR USE IN MASONRY APPLICATIONS.

### STEEL JOISTS

- MATERIALS:
  - STEEL JOISTS: IN ACCORDANCE WITH SJI SPECIFICATIONS.
  - BRIDGING AND ACCESSORIES: IN ACCORDANCE WITH SJI SPECIFICATIONS.
  - HIGH-STRENGTH BOLTS: ASTM A325
  - CARBON STEEL BOLTS: ASTM A307, GRADE A
  - WELD ELECTRODE: IN ACCORDANCE WITH AWS D1.1
- FABRICATE AND ERECT JOISTS IN ACCORDANCE WITH THE SJI SPECIFICATIONS.
- ALL MOMENT CONNECTIONS AND FIELD WELDING WITH CERTIFIED WELDERS IN ACCORDANCE WITH AWS D1.1.
- INSTALL 3/4 INCH DIAMETER HIGH STRENGTH BOLTS, SNUG TIGHT, IN BOLTED JOIST-TO-STRUCTURAL STEEL, JOIST-TO-JOIST GIRDER, AND JOIST SPLICE CONNECTIONS.
- INSTALL CARBON STEEL BOLTS IN BOLTED CONNECTIONS FOR BRIDGING AND JOIST ACCESSORIES.
- INSTALL BRIDGING AND UPLIFT BRIDGING AS REQUIRED BY THE SJI SPECIFICATIONS.

### STRUCTURAL MASONRY

- SCOPE: THESE NOTES APPLY TO LOAD BEARING MASONRY OR MASONRY THAT IS PART OF THE LATERAL LOAD RESISTING SYSTEM. SEE ARCHITECTURAL FOR OTHER MASONRY.
- ALL MASONRY WORK SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI530.1-13) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI530.1-13).
- MATERIALS:
  - CONCRETE MASONRY UNITS: ASTM C90, 2000 PSI MIN. UNIT STRENGTH.
  - MORTAR: ASTM C270, PROPORTION SPECIFICATION, TYPE S.
  - GROUT: ASTM C476, SLUMP = 8" TO 11", COMPRESSIVE STRENGTH  $f_c = 3000$  PSI.
  - REINFORCING IRON = 2000 PSI.
  - REINFORCING STEEL: ASTM A615, GRADE 60.  $F_s = 24,000$  PSI.
- LAP REINFORCING AS FOLLOWS, UNLESS NOTED OTHERWISE:

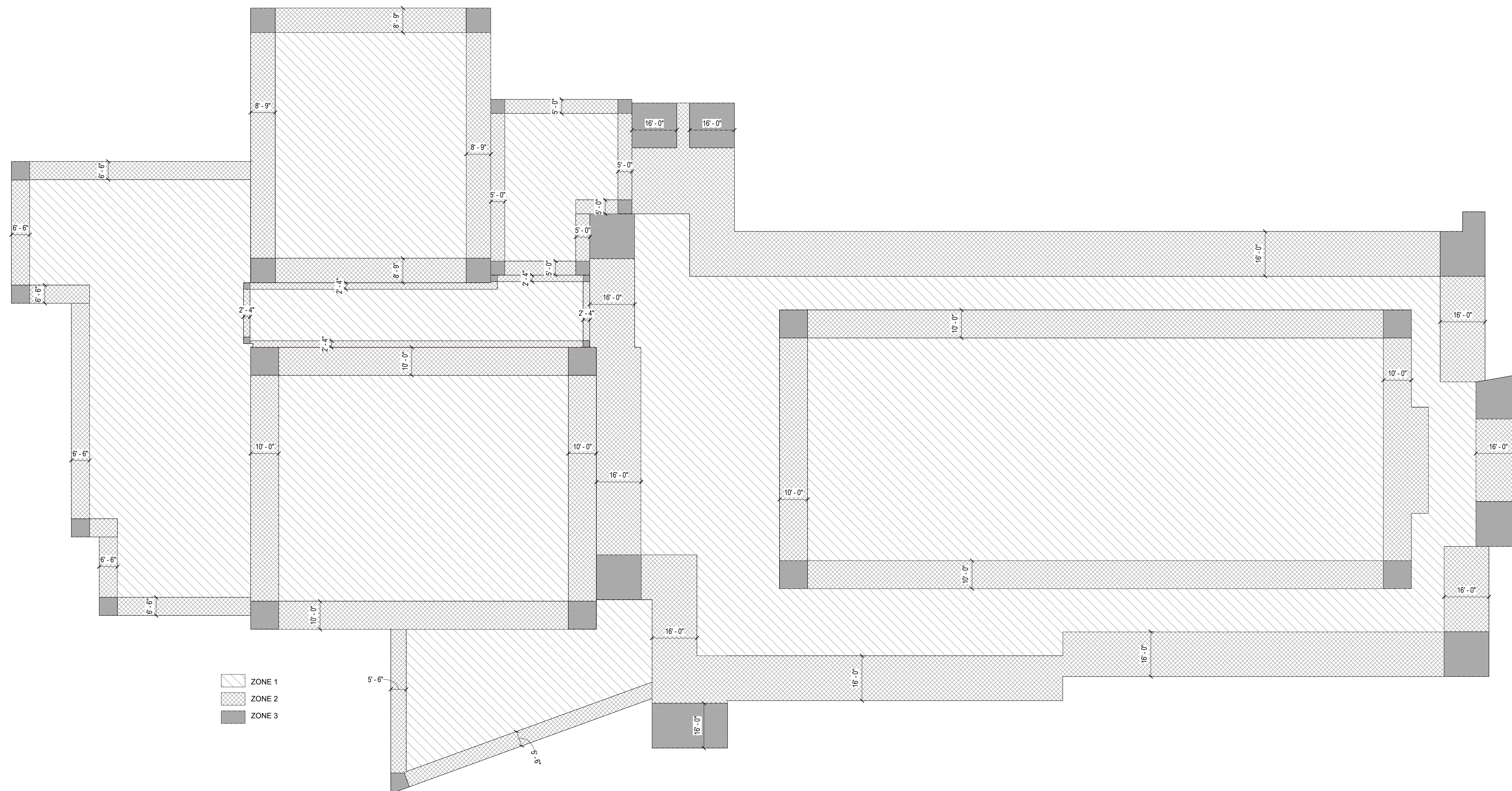
	#3	1'-6"	#7	5'-0"
	#4	2'-0"	#8	8'-0"
	#5	2'-6"	#9	10'-0"
	#6	4'-0"	#10	12'-6"
- INSTALL REINFORCING IN THE CENTER OF CELLS UNLESS IND





	06/12/2024	BID DOCUMENTS
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S-020

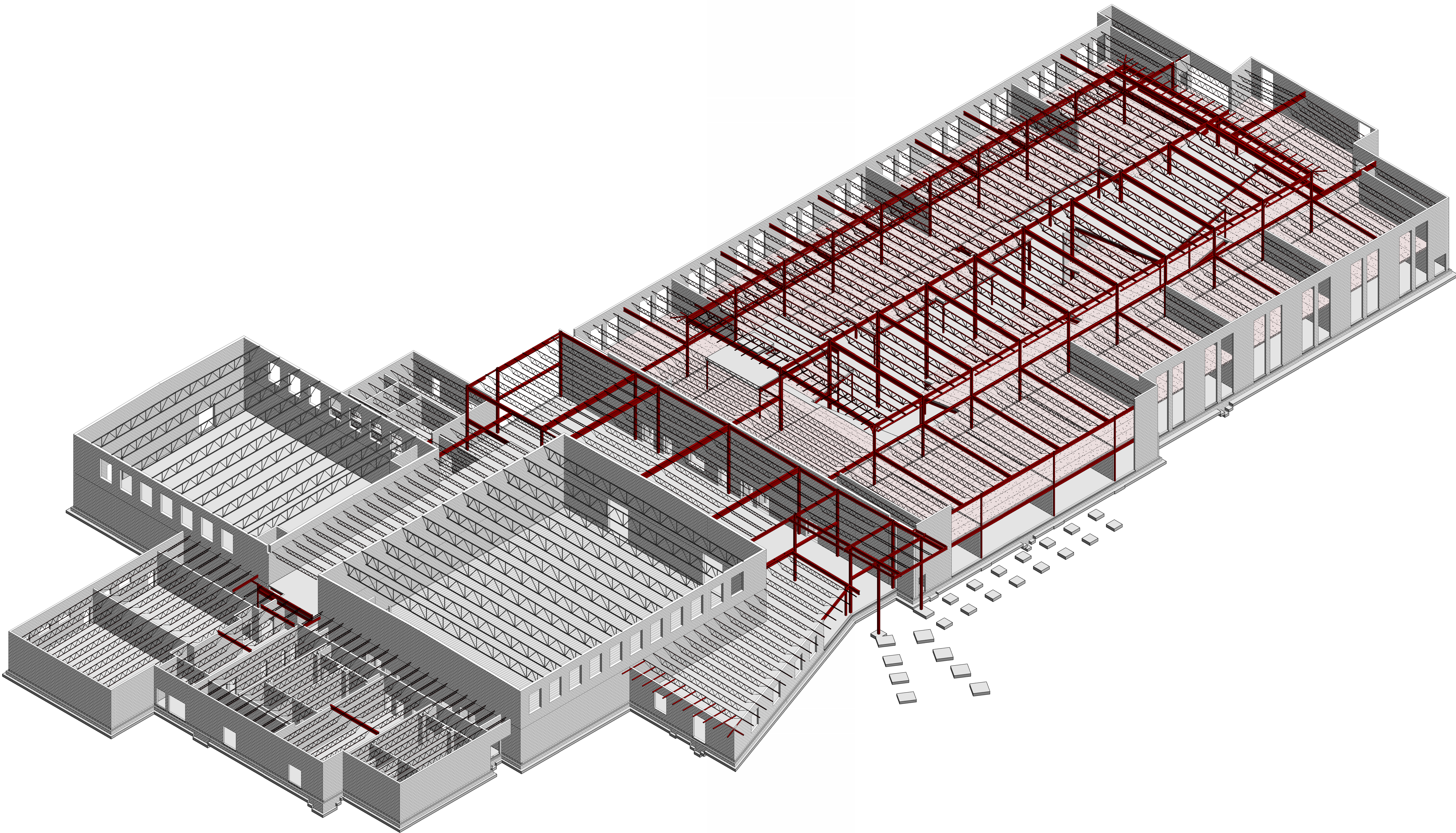


1 ROOF UPLIFT DESIGN PRESSURES  
S-020 SCALE: 1" = 20'-0"

**ROOF UPLIFT DESIGN PRESSURE NOTES:**

1. ROOF WIND UPLIFT DESIGN PRESSURES INDICATED ON THIS SHEET ARE FOR ROOFING MATERIAL COMPONENTS AND CLADDING ONLY. SEE SHEET S-010 FOR DESIGN PRESSURES.
2. DESIGN NET WIND UPLIFT FOR JOISTS SHALL BE DETERMINED BASED ON WIND ZONES SHOWN IN THE DIAGRAM ABOVE. JOIST TRIBUTARY AREA, AND WIND PRESSURES INDICATED ON SHEET S-010. NET UPLIFT SHALL BE CALCULATED USING A RELIABLE ROOF DEAD LOAF OF 10 PSF AND THE APPLICABLE LOAD COMBINATIONS PER ASCE 7-10.

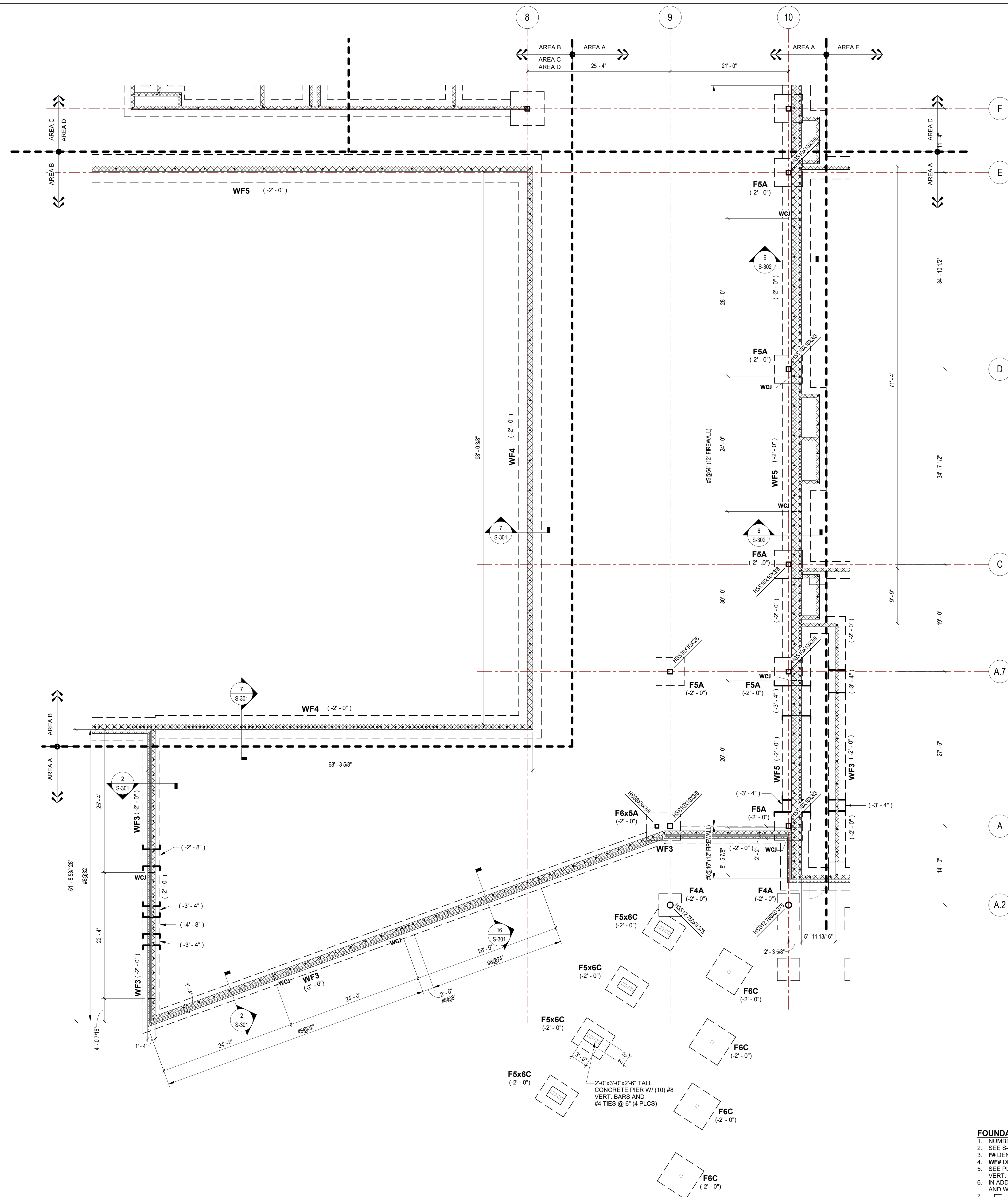




1  
S-030  
STRUCTURAL ISOMETRIC VIEW  
SCALE:

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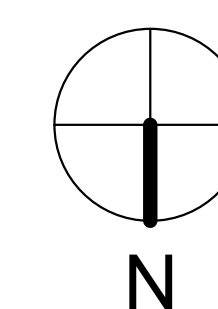
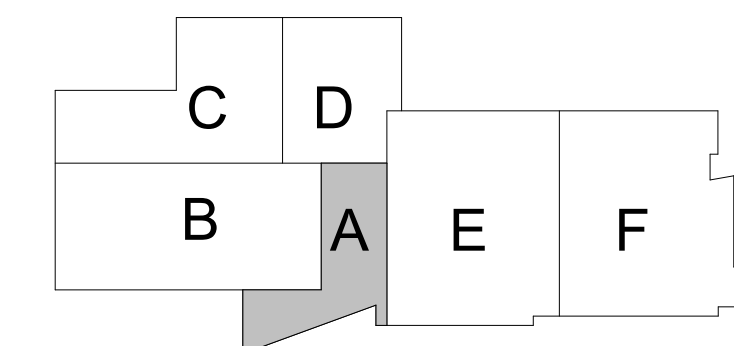
WALL FOUNDATION SCHEDULE	
MARK	FOUNDATION SIZE (WIDTH x THICKNESS)
WF3	3' - 0"x1' - 0"
WF4	4' - 0"x1' - 0"
WF5	5' - 0"x1' - 0"
WF5.5	5' - 6"x1' - 0"

COLUMN FOOTING SCHEDULE				
MARK	SIZE	DEPTH	REINFORCING (EACH WAY)	REMARKS
F4	4'-0" x4'-0"	1'-2"	(4) #5 EW BOTTOM	FOOTING SIZE FOR BID PURPOSES ONLY. TO BE CONFIRMED AFTER REVIEW OF CANOPY SHOP DRAWINGS. CANOPY & FOUNDATIONS ARE PART OF ALTERNATE NO. 1B
F4A	1'-0" x3'-0"	1'-2"	(4) #5 EW TOP & BOTTOM	
F4C	4'-0" x4'-0"	1'-2"	(4) #5 EW TOP & BOTTOM	
F6	5'-0" x5'-0"	1'-2"	(6) #5 EW TOP; (6) #5 EW BOTTOM	
F6.5	5'-0" x5'-0"	1'-2"	(6) #5 EW BOTTOM	FOOTING SIZE FOR BID PURPOSES ONLY. TO BE CONFIRMED AFTER REVIEW OF CANOPY SHOP DRAWINGS. CANOPY & FOUNDATIONS ARE PART OF ALTERNATE NO. 1A
F5A	5'-0" x5'-0"	1'-2"	(6) #5 EW TOP & BOTTOM	
F5x6C	6'-0" x6'-0"	1'-0"	(4) #5 EW TOP & BOTTOM	
F6	6'-0" x6'-0"	1'-2"	(7) #5 EW BOTTOM	
F6A	6'-0" x6'-0"	1'-2"	(7) #5 EW TOP & BOTTOM	FOOTING SIZE FOR BID PURPOSES ONLY. TO BE CONFIRMED AFTER REVIEW OF CANOPY SHOP DRAWINGS. CANOPY & FOUNDATIONS ARE PART OF ALTERNATE NO. 1A
F6C	6'-0" x6'-0"	1'-2"	(6) #5 EW TOP & BOTTOM	
F6x5A	5'-0" x5'-0"	1'-2"	(7) #5 SW, (5) #5 LW TOP & BOTTOM	
F7	7'-0" x7'-0"	1'-6"	(7) #6 EW BOTTOM	
F7.5	7'-6" x7'-6"	1'-6"	(7) #6 EW BOTTOM	FOOTING SIZE FOR BID PURPOSES ONLY. TO BE CONFIRMED AFTER REVIEW OF CANOPY SHOP DRAWINGS. CANOPY & FOUNDATIONS ARE PART OF ALTERNATE NO. 1A
F8	8'-0" x8'-0"	1'-8"	(6) #7 EW BOTTOM	
F8.5	8'-6" x8'-6"	1'-8"	(7) #7 EW BOTTOM	
F10	10'-0" x10'-0"	2'-0"	(9) #7 EW BOTTOM	

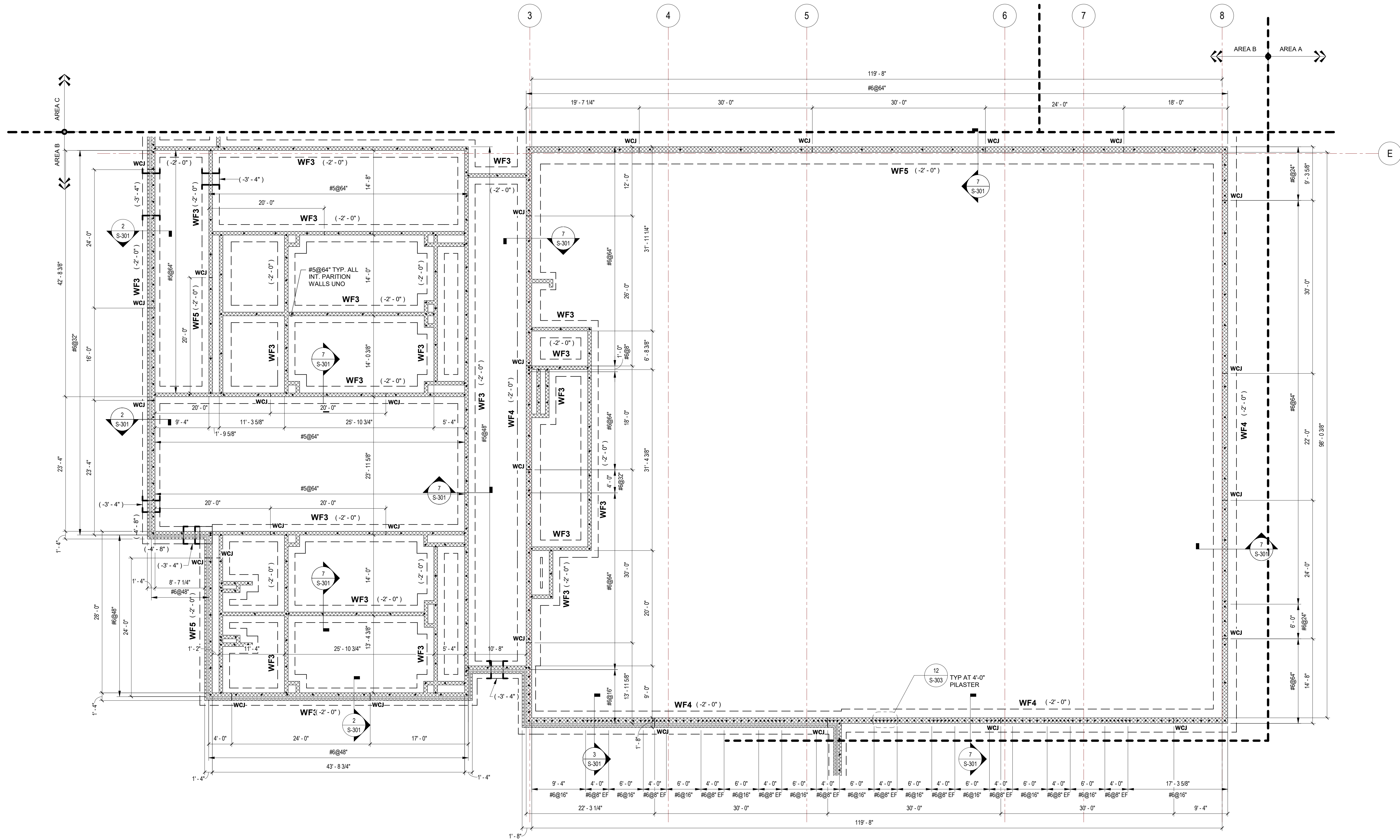
NOTE WALL AND COLUMN FOOTINGS HAVE BEEN DESIGNED TO BE SUPPORTED ON IMPROVED SOIL SUPPORTED BY RAMMED  
AGGREGATE PIERS WITH A DESIGN BEARING CAPACITY OF 3,000 PSF.

### FOUNDATION PLAN NOTES

- ### FOUNDATION PLAN NOTES:
1. NUMBER IN PARENTHESIS DENOTES TOP OF FOOTING BELOW FIN. FLOOR ELEVATION = 0'-0".
  2. SEE S-2000 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
  3. **#4** DENOTES COLUMN FOOTING. SEE S-100A FOR SCHEDULE.
  4. **W#F** DENOTES WALL FOOTING. SEE S-100A FOR SCHEDULE. INTERIOR LOAD BEARING 8" CMU IS W#F3 U-10.
  5. SEE PLAN FOR CMU WALL REINFORCING. FOR WALLS WHERE SPACING IS NOT IDENTIFIED, PROVIDE #8@64" MAX SPACING.
  6. **W#F** DENOTES WALL FOOTING. SEE S-100A FOR SCHEDULE. INTERIOR LOAD BEARING 8" CMU IS W#F3 U-10.
  7. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE ADDITIONAL REINFORCING IN JAMBS OF ALL DOORS AND WINDOWS AND EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. REFER TO DETAILS S-530/30 AND S-750/30.
  8. **#4** DENOTES TOP OF COLUMN FOOTING. SEE S-100A FOR DETAIL. G.C. COORDINATE STEP LOCATION AND DEPTH W/ UTILITY CONTRACTOR PRIOR TO FOOTING EXCAVATION.
  9. REFER TO ARCH. DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
  10. **#4** DENOTES TOP OF COLUMN FOOTING. SEE S-100A FOR DETAIL. G.C. COORDINATE STEP LOCATION AND DEPTH W/ UTILITY CONTRACTOR PRIOR TO FOOTING EXCAVATION.
  11. PROVIDE BOND BEAMS AT 9"4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
  12. **P1** DENOTES 11"11" O.C. ELEVATOR SHAFT. PROVIDE 11"11" O.C. ELEVATOR SHAFT REINFORCING AT ALL ELEVATIONS.
  13. **P1** DENOTES 24"x24" CONCRETE PIER REIN' W/ 8#BVERT AND 4#4 TIES AT "C". REFER TO 8 AND 9 ON S-301.
  14. LIGHT GREY HATCHING OF COLUMN FOOTINGS DENOTES LOCATIONS OF GROUT-FILLED AGGREGATE PIERS UNDER COLUMN FOOTINGS.
  15. **GB1** DENOTES 36"x14" CONCRETE GRADE BEAM. SEE DETAIL 17-S301.



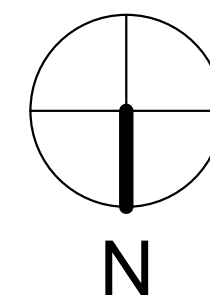
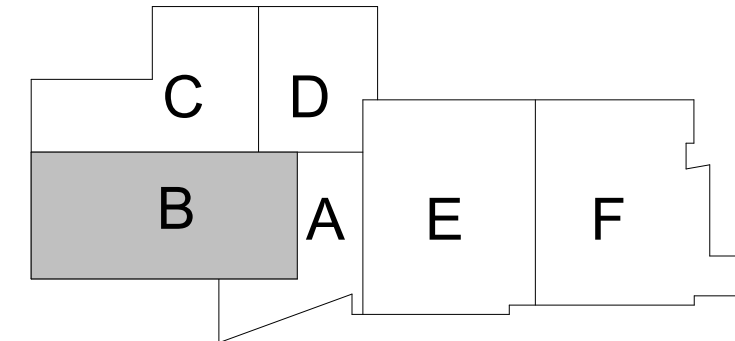




1 FOUNDATION PLAN - AREA B  
SCALE: 1/8" = 1'-0"

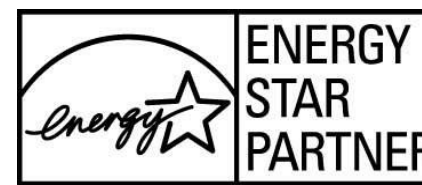
FOUNDATION PLAN NOTES

1. NUMBER IN PARENTHESIS DENOTES TOP OF FOOTING BELOW FIN. FLOOR ELEVATION = 0'-0".
2. SEE S-200 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
3. WF DENOTES COLUMN FOOTING. SEE S-100A FOR SCHEDULE.
4. WFF DENOTES WALL FOOTING. SEE S-100A FOR SCHEDULE. INTERIOR NON-LOAD BEARING 8" CMU IS WF3 U.O.N.
5. SEE PLAN FOR CMU WALL REINFORCING. FOR WALLS WHERE SPACING IS NOT IDENTIFIED, PROVIDE #5@64" MAX SPACING.
6. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE ADDITIONAL REINFORCING IN JAMBS OF ALL DOORS AND WINDOWS AND EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. REFER TO DETAILS 9/S-303 AND 7/S-303.
7. TYP DENOTES STEPPED FOOTING. SEE 12/S-301 FOR DETAIL. G.C. COORDINATE STEP LOCATION AND DEPTH W/ UTILITY CONTRACTOR PRIOR TO FOOTING EXCAVATION.
8. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
9. SEE DETAIL 9/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
10. PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
11. VERIFY ELEVATOR PIT DIMENSIONS W/ ELEVATOR SUPPLIER PRIOR TO EXCAVATION.
12. PT DENOTES 24"x24" CONCRETE PIER REIN W/ 8-#8 VERT AND #4 TIES AT 6". REFER TO 8 AND 9 ON S-302.
13. LIGHT GREY HATCHING OF COLUMN FOOTINGS DENOTES LOCATIONS OF GROUT-FILLED AGGREGATE PIERS UNDER COLUMN FOOTINGS.
14. GB1 DENOTES 30"x14" CONCRETE GRADE BEAM. SEE DETAIL 17/S-301.



CONSTRUCTION  
DOCUMENTS

PAMLICO COUNTY  
PAMLICO 6-12 SCHOOL  
601 Main Street, Bayboro, NC, 28515

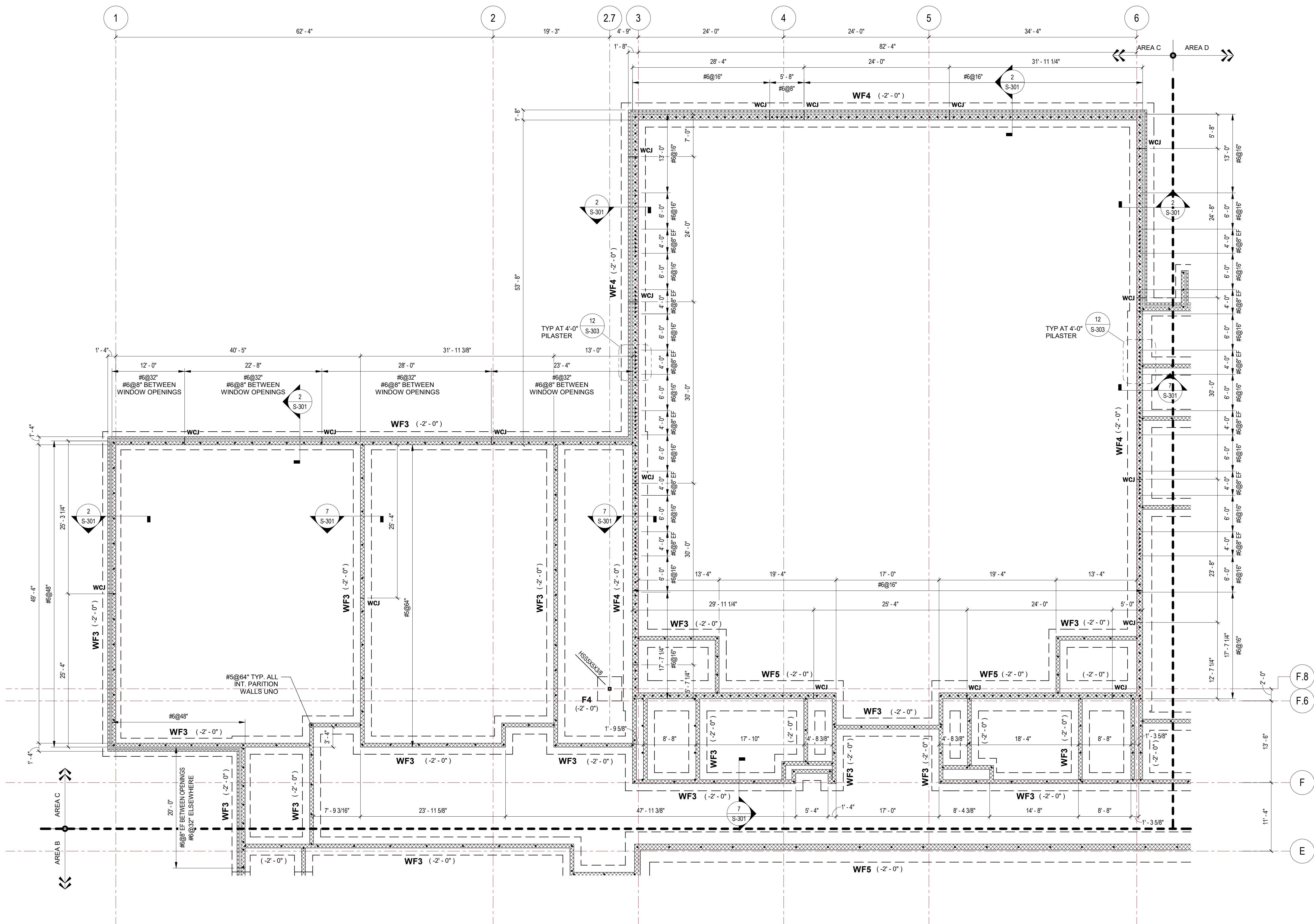


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PROJECT #:	23.08.034	
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FOUNDATION PLAN  
AREA B

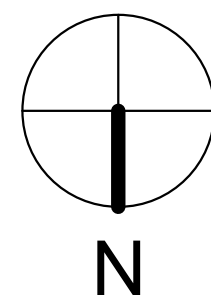
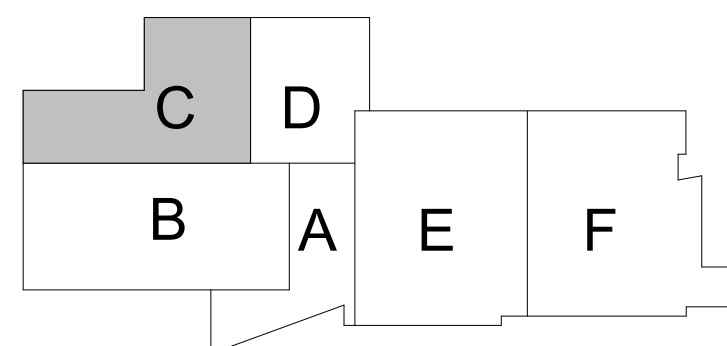
S-100B





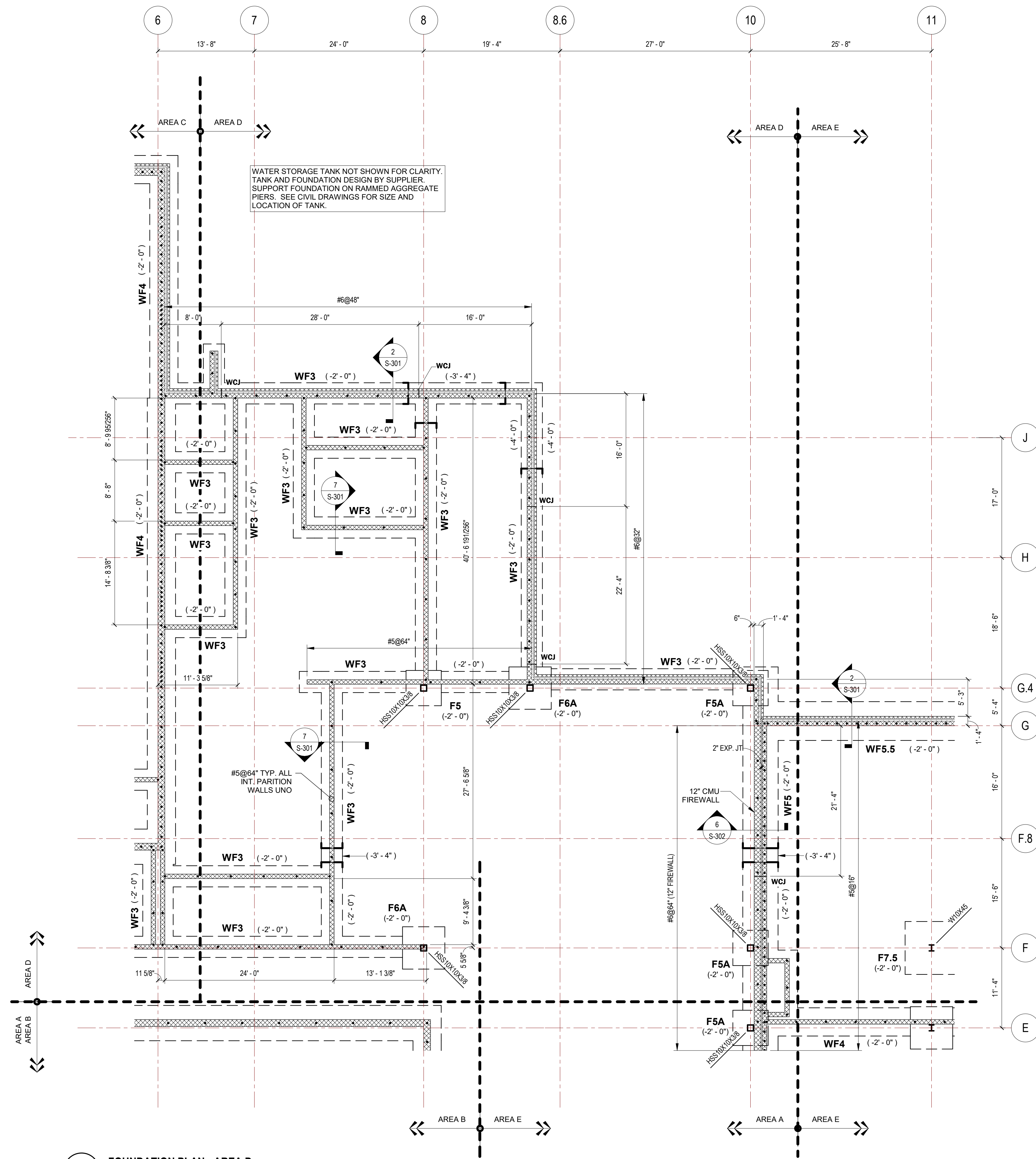
1 FOUNDATION PLAN - AREA C  
S-100C SCALE: 1/8" = 1'-0"

- FOUNDATION PLAN NOTES:**
- NUMBER IN PARENTHESIS DENOTES TOP OF FOOTING BELOW FIN. FLOOR ELEVATION = 0'-0".
  - SEE S-200 AND S-301 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
  - F# DENOTES COLUMN FOOTING. SEE S-100A FOR SCHEDULE.
  - WF# DENOTES WALL FOOTING. SEE S-100A FOR SCHEDULE. INTERIOR NON-LOAD BEARING 8" CMU IS WF3 U.O.N.
  - SEE PLAN FOR CMU WALL REINFORCING. FOR WALLS WHERE SPACING IS NOT IDENTIFIED, PROVIDE #5@64" MAX SPACING VERT. IN ALL 8" CMU PARTITION WALLS. SEE S-303 FOR ADDITIONAL CMU REINFORCING REQUIREMENTS.
  - IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE ADDITIONAL REINFORCING IN JAMBS OF ALL DOORS AND WINDOWS AND EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. REFER TO DETAILS 5/S-303 AND 7/S-303.
  - 12" DENOTES STEPPED FOOTING. SEE 12/S-301 FOR DETAIL. G.C. COORDINATE STEP LOCATION AND DEPTH W/ UTILITY CONTRACTOR PRIOR TO FOOTING EXCAVATION.
  - REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
  - SEE DETAIL 9/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
  - PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
  - VERIFY ELEVATOR PIT DIMENSIONS W/ ELEVATOR SUPPLIER PRIOR TO EXCAVATION.
  - P# DENOTES 24"x24" CONCRETE PIER REINF W/ 8-#6 VERT AND #4 TIES AT 6". REFER TO 8 AND 9 ON S-302.
  - LIGHT GREY HATCHING OF COLUMN FOOTINGS DENOTES LOCATIONS OF GROUT-FILLED AGGREGATE PIERS UNDER COLUMN FOOTINGS.
  - GB1 DENOTES 36"x14" CONCRETE GRADE BEAM. SEE DETAIL 17/S-301.



No.	Date	Description
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ISSUE DATE: 06/12/2024		
PROJECT #: 23.08.034		
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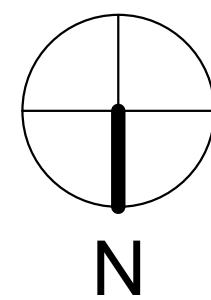
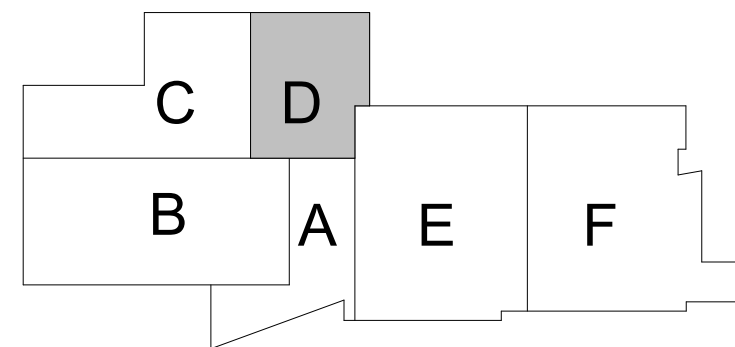




**1**  
**S-100D** FOUNDATION PLAN - AREA D  
SCALE: 1/8" = 1'-0"

**FOUNDATION PLAN NOTES:**

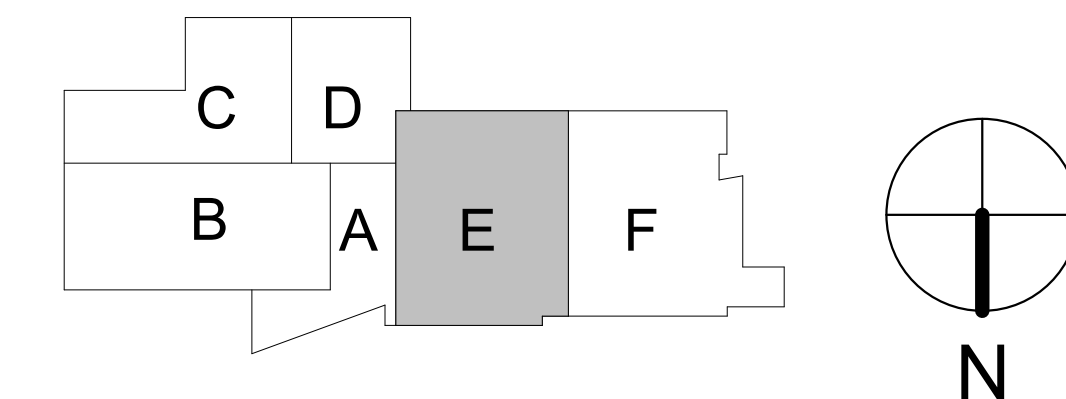
1. NUMBER IN PARENTHESIS DENOTES TOP OF FOOTING BELOW FIN. FLOOR ELEVATION = 0'-0".
2. SEE S-200 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
3. **F#** DENOTES COLUMN FOOTING. SEE S-100A FOR SCHEDULE.
4. **WF#** DENOTES WALL FOOTING. SEE S-100A FOR SCHEDULE. INTERIOR NON-LOAD BEARING 8" CMU IS WF3 U.O.N.
5. SEE PLAN FOR CMU WALL REINFORCING. FOR WALLS WHERE SPACING IS NOT IDENTIFIED, PROVIDE #5@64" MAX SPACING VERT. IN ALL 8" CMU PARTITION WALLS. SEE S-303 FOR ADDITIONAL CMU REINFORCING REQUIREMENTS.
6. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE ADDITIONAL REINFORCING IN JAMBS OF ALL DOORS AND WINDOWS AND EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. REFER TO DETAILS 5/S-303 AND 7/S-303.
7. DENOTES STEPPED FOOTING. SEE 12/S-301 FOR DETAIL. G.C. COORDINATE STEP LOCATION AND DEPTH W/ UTILITY CONTRACTOR PRIOR TO FOOTING EXCAVATION.
8. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
9. SEE DETAIL 9/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
10. PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
11. VERIFY ELEVATOR PIT DIMENSIONS W/ ELEVATOR SUPPLIER PRIOR TO EXCAVATION.
12. **P1** DENOTES 24"x24" CONCRETE PIER REINF W/ 6#8 VERT AND #4 TIES AT 8". REFER TO 8 AND 9 ON S-302.
13. LIGHT GREY HATCHING OF COLUMN FOOTINGS DENOTES LOCATIONS OF GROUT-FILLED AGGREGATE PIERS UNDER COLUMN FOOTINGS.
14. **GB1** DENOTES 36"x14" CONCRETE GRADE BEAM. SEE DETAIL 17/S-301.



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S-100E

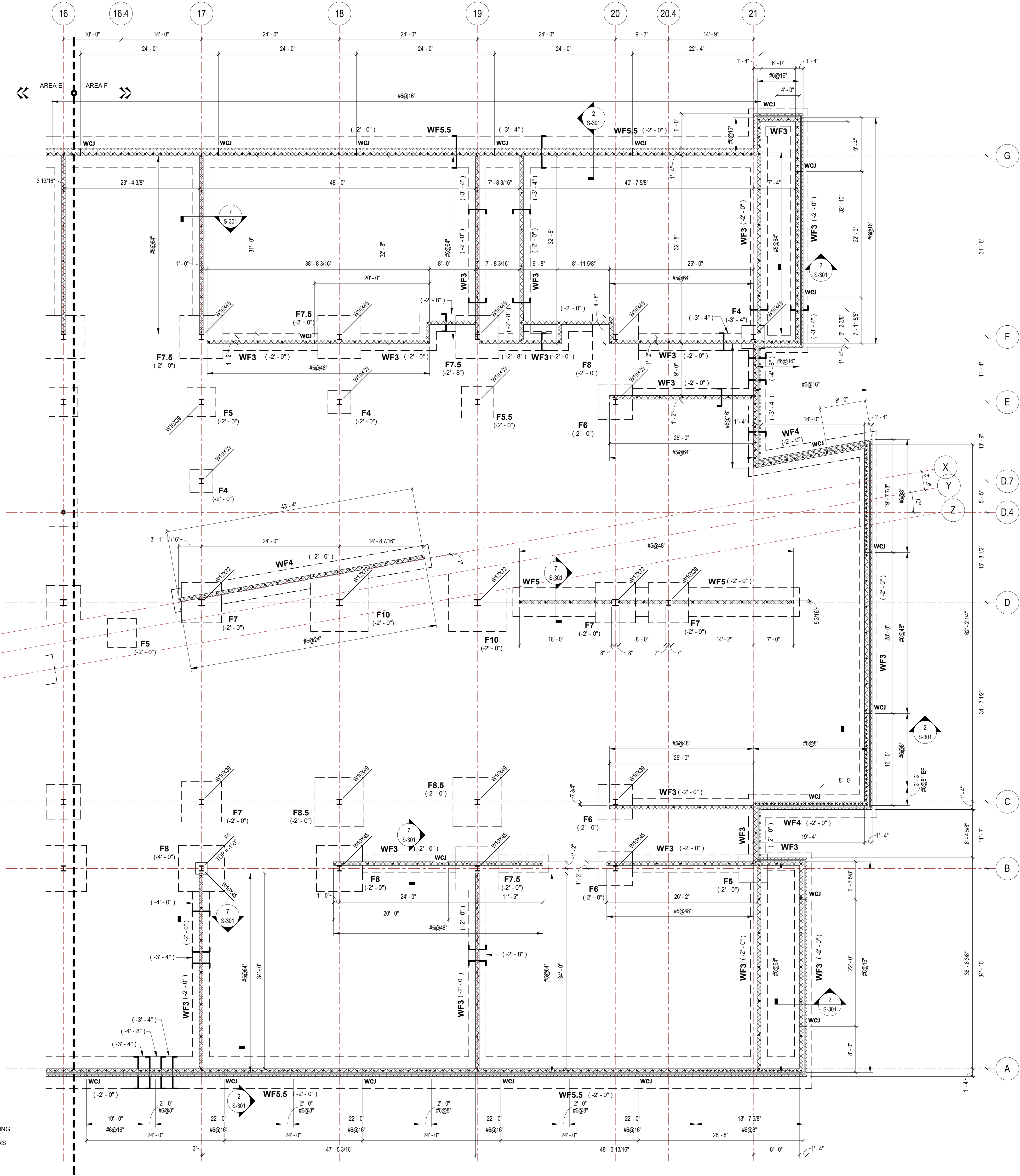




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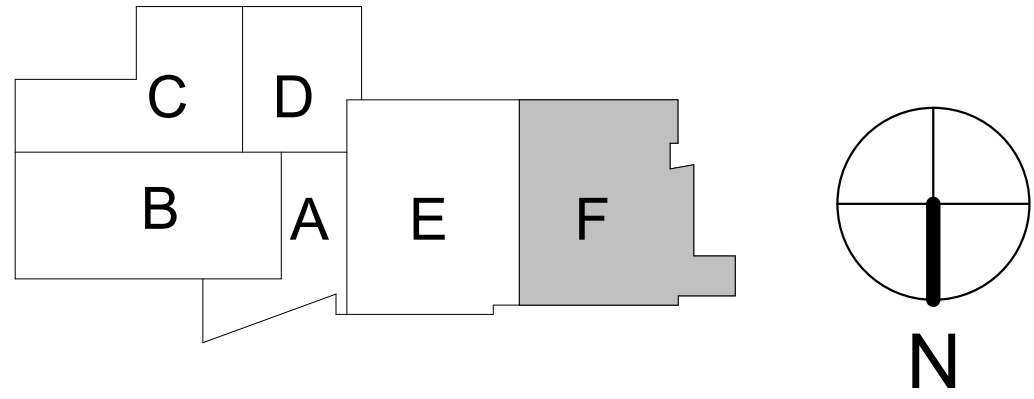
FOUNDATION PLAN NOTES:

1. NUMBER IN PARENTHESIS DENOTES TOP OF FOOTING BELOW FIN. FLOOR ELEVATION = 0'-0".
2. SEE S-200 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
3. #F# DENOTES COLUMN FOOTING. SEE S-100A FOR SCHEDULE.
4. WF# DENOTES WALL FOOTING. SEE S-100A FOR SCHEDULE. INTERIOR NON-LOAD BEARING 8" CMU IS WF3 U.O.N.
5. SEE PLAN FOR CMU WALL REINFORCING. FOR WALLS WHERE SPACING IS NOT IDENTIFIED, PROVIDE #5@64" MAX SPACING.
6. VERT. IN ALL 8" CMU PARTITION WALLS. SEE S-303 FOR ADDITIONAL CMU REINFORCING REQUIREMENTS.
7. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE ADDITIONAL REINFORCING IN JAMBS OF ALL DOORS AND WINDOWS AND EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. REFER TO DETAILS 5/ S-303 AND 7/ S-303.
8. DENOTES STEPPED FOOTING. SEE 12/ S-301 FOR DETAIL. G.C. COORDINATE STEP LOCATION AND DEPTH W/ UTILITY CONTRACTOR PRIOR TO FOOTING EXCAVATION.
9. REFER TO ARCH. DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
10. SEE DETAIL 9/ S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
11. PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
12. VERIFY ELEVATOR PIT DIMENSIONS W/ ELEVATOR SUPPLIER PRIOR TO EXCAVATION.
13. P1 DENOTES 24"x24" CONCRETE PIER REINF W/ 8-#8 VERT AND #4 TIES AT 6". REFER TO 8 AND 9 ON S-302.
14. LIGHT GREY HATCHING OF COLUMN FOOTINGS DENOTES LOCATIONS OF GROUT-FILLED AGGREGATE PIERS UNDER COLUMN FOOTINGS.
15. GB1 DENOTES 36"x14" CONCRETE GRADE BEAM. SEE DETAIL 17/ S-301.



1 FOUNDATION PLAN - AREA F

SCALE: 1/8" = 1'-0"



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CONSTRUCTION  
DOCUMENTS

PAMLICO COUNTY  
PAMLICO 6-12 SCHOOL  
601 Main Street, Bayboro, NC, 28515

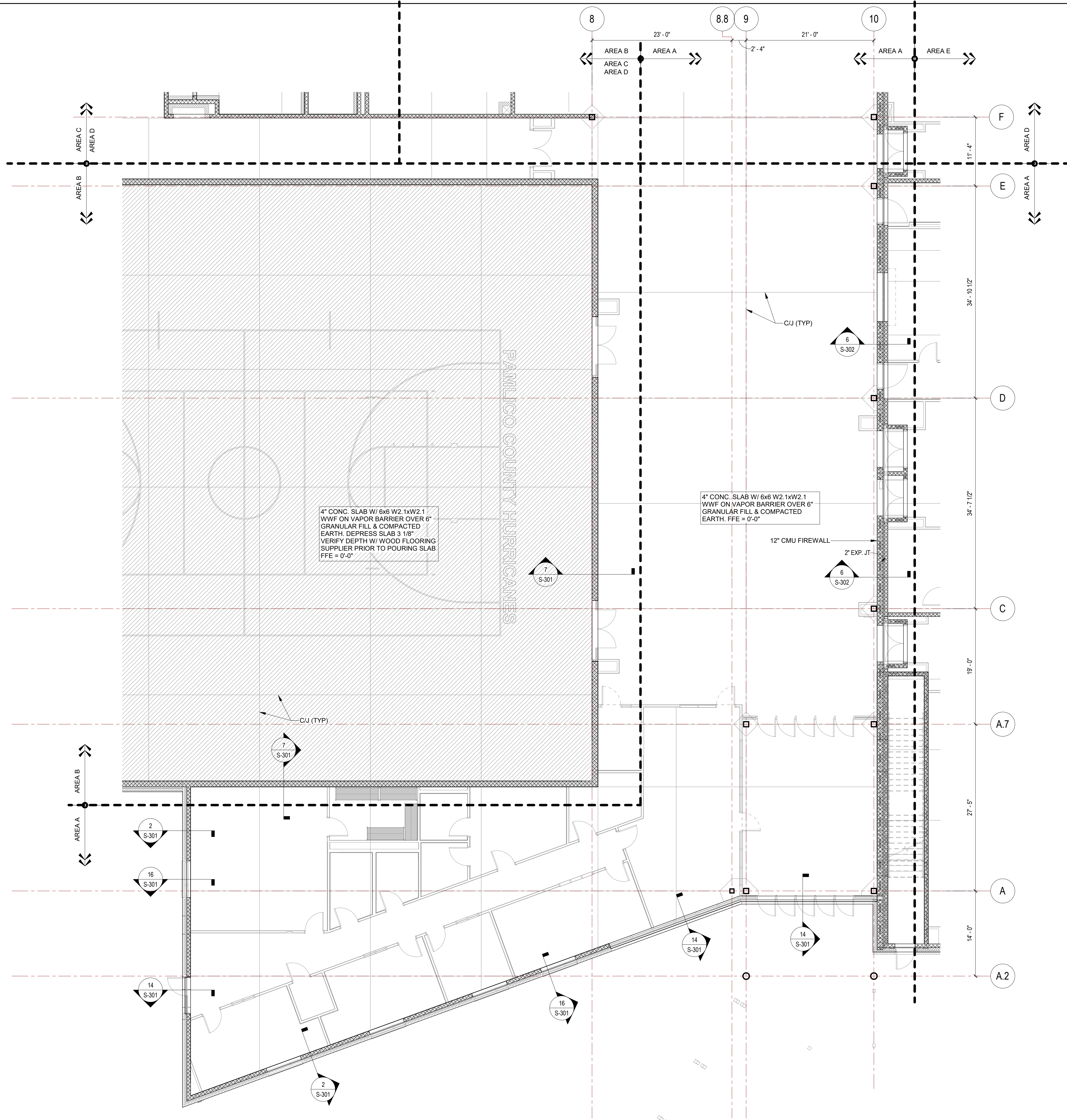


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FOUNDATION PLAN  
AREA F

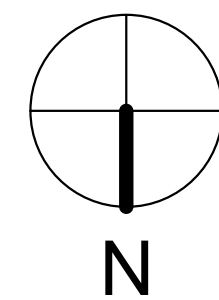
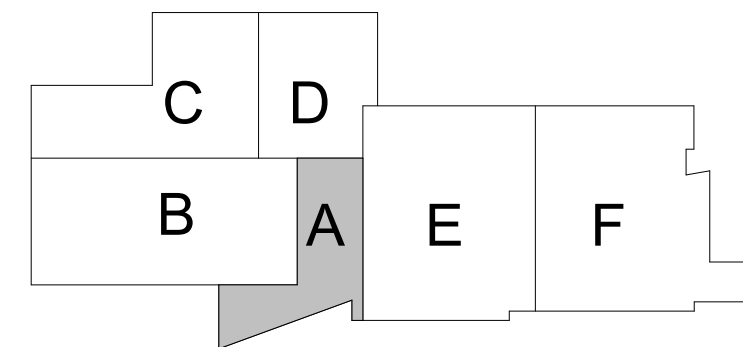
S-100F





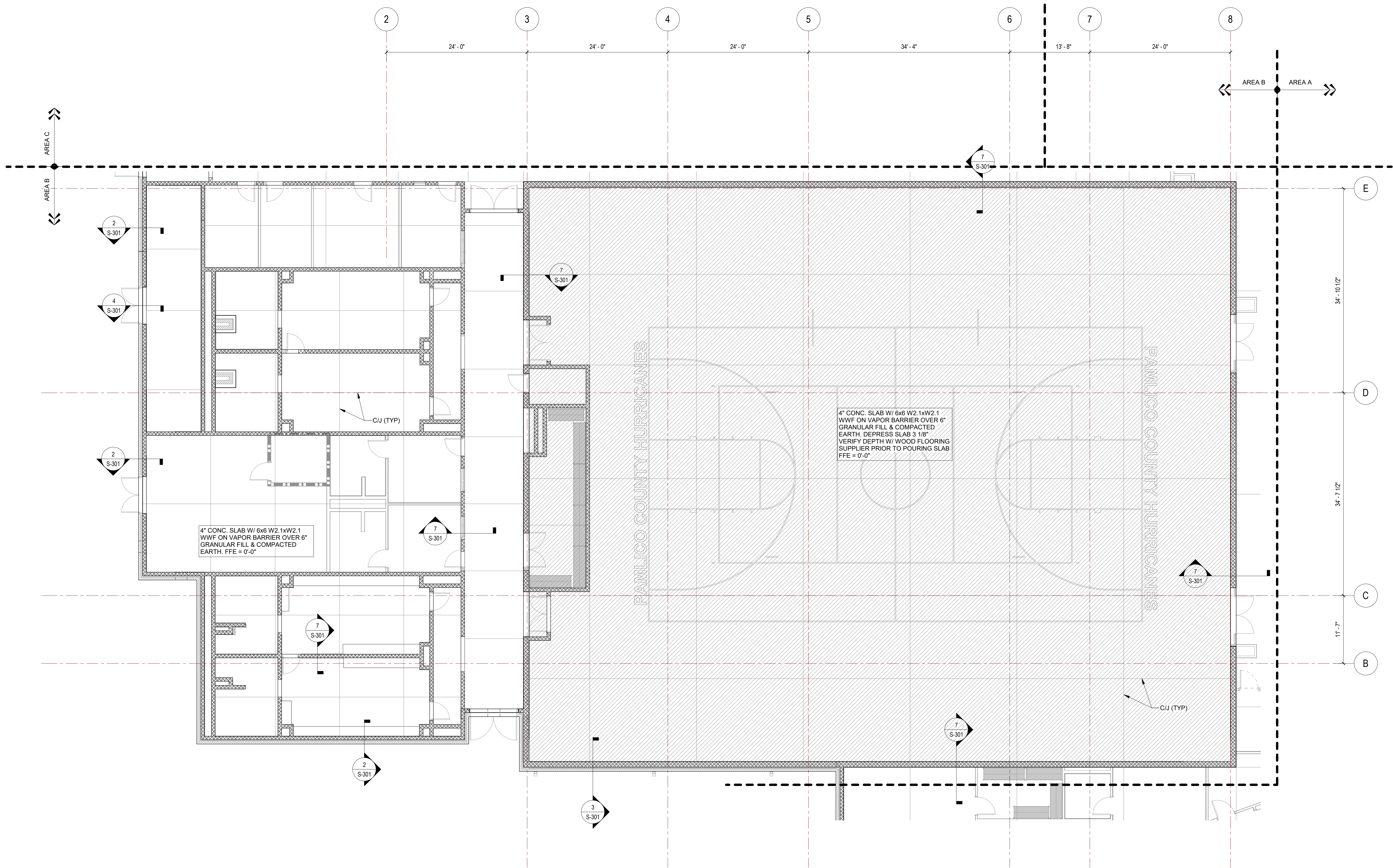
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S-101A FIRST FLOOR - SLAB PLAN AREA A  
SCALE: 1/8" = 1'-0"

- SLAB PLAN NOTES:**
1. FIN. FLOOR ELEVATION = 0'-0" CORRESPONDS TO ELEVATION 13.80'.
  2. C/J DENOTES SLAB CONTROL/CONSTRUCTION JOINT. SEE 11/S-301.
  3. SEE S-200 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
  4. SEE S-303 FOR CMU WALL REINFORCING REQUIREMENTS.
  5. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE #6 VERT. BAR IN JAMBS OF ALL DOORS AND WINDOWS. PROVIDE #6 VERT. BAR EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. SEE ARCH'L FOR JOINT LOCATIONS. SEE DETAIL 9/S-303 & 6/S-303.
  6. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
  7. SEE DETAIL 9/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
  8. PROVIDE BOND BEAMS AT 8'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
  9. PROVIDE #6@48" VERTICAL REINFORCING IN ALL NON-LOAD BEARING WALLS U.O.N.
  10. SEE DETAIL 6/S-303 FOR TYPICAL CMU WALL PENETRATION DETAILS.
  11. PROVIDE INTERLOCKING CELLS AT CMU WALL CORNERS AND 1-#6 VERT.





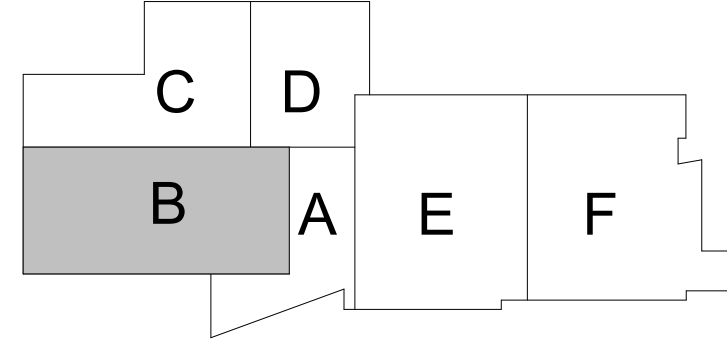
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1  
S-101B  
FIRST FLOOR - SLAB PLAN AREA B  
SCALE: 1/8" = 1'-0"

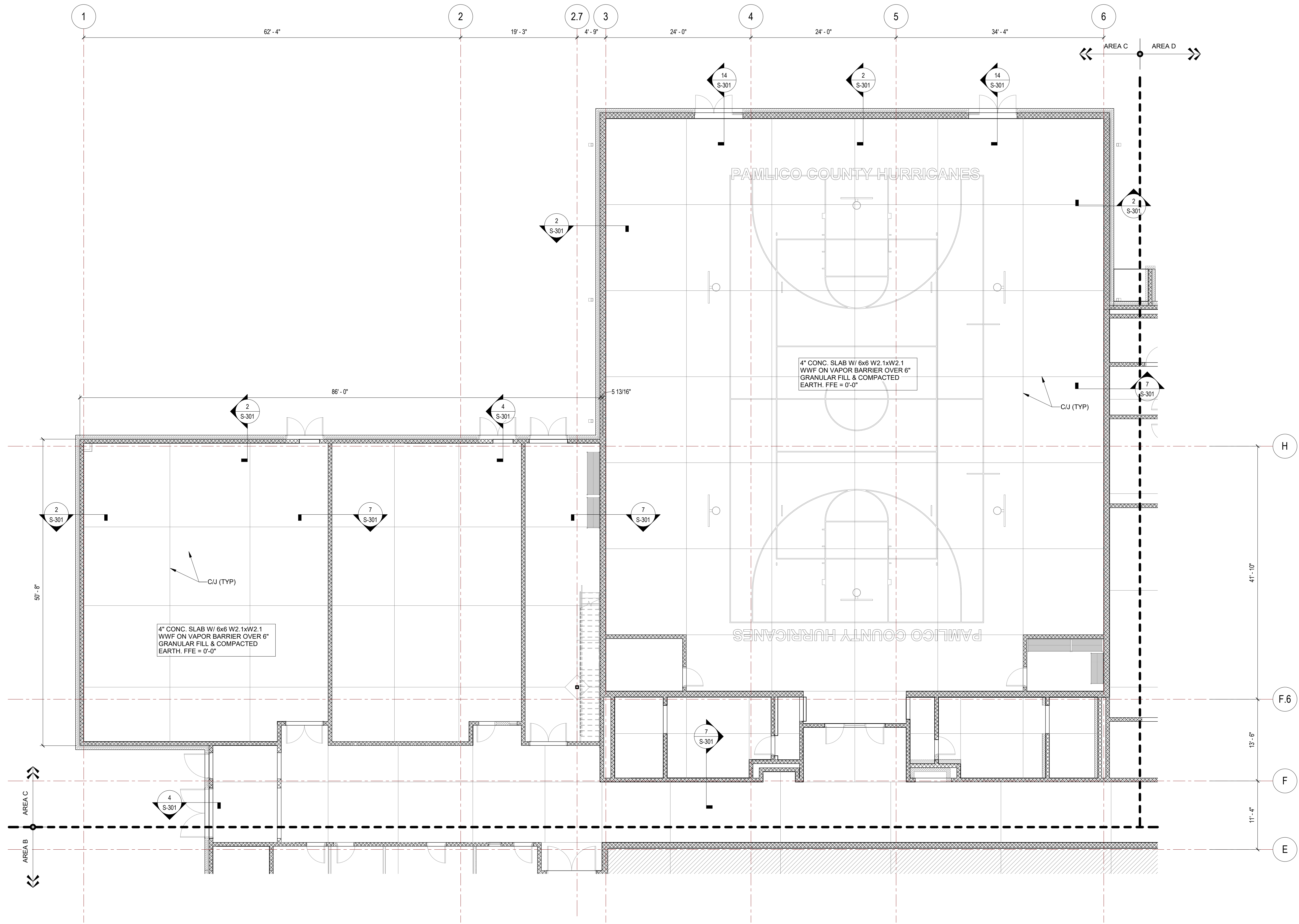
SLAB PLAN NOTES:

1. FIN. FLOOR ELEVATION = 0'-0" CORRESPONDS TO ELEVATION 13.80'.
2. CUJ DENOTES SLAB CONTROL/CONSTRUCTION JOINT. SEE 11/S-301.
3. SEE S-200 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
4. SEE S-303 FOR CMU WALL REINFORCING REQUIREMENTS.
5. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE #6 VERT. BAR IN JAMBS OF ALL DOORS AND WINDOWS. PROVIDE #6 VERT. BAR EA. SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. SEE ARCH'L FOR JOINT LOCATIONS. SEE DETAIL 5/S-303 & 6/S-303.
6. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
7. SEE DETAIL 5/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
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9. PROVIDE #5@48" VERTICAL REINFORCING IN ALL NON-LOAD BEARING WALLS U.O.N.
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11. PROVIDE INTERLOCKING CELLS AT CMU WALL CORNERS AND 1-#6 VERT.





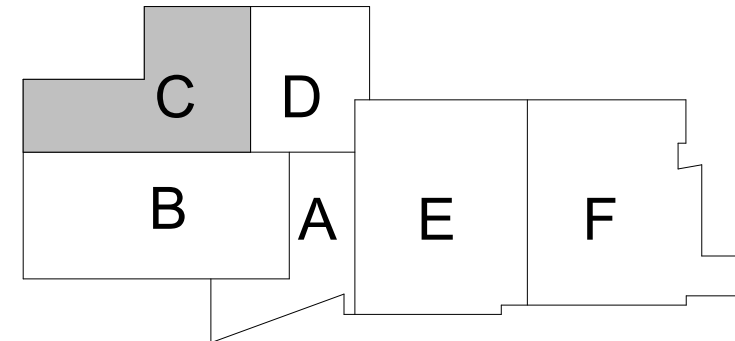
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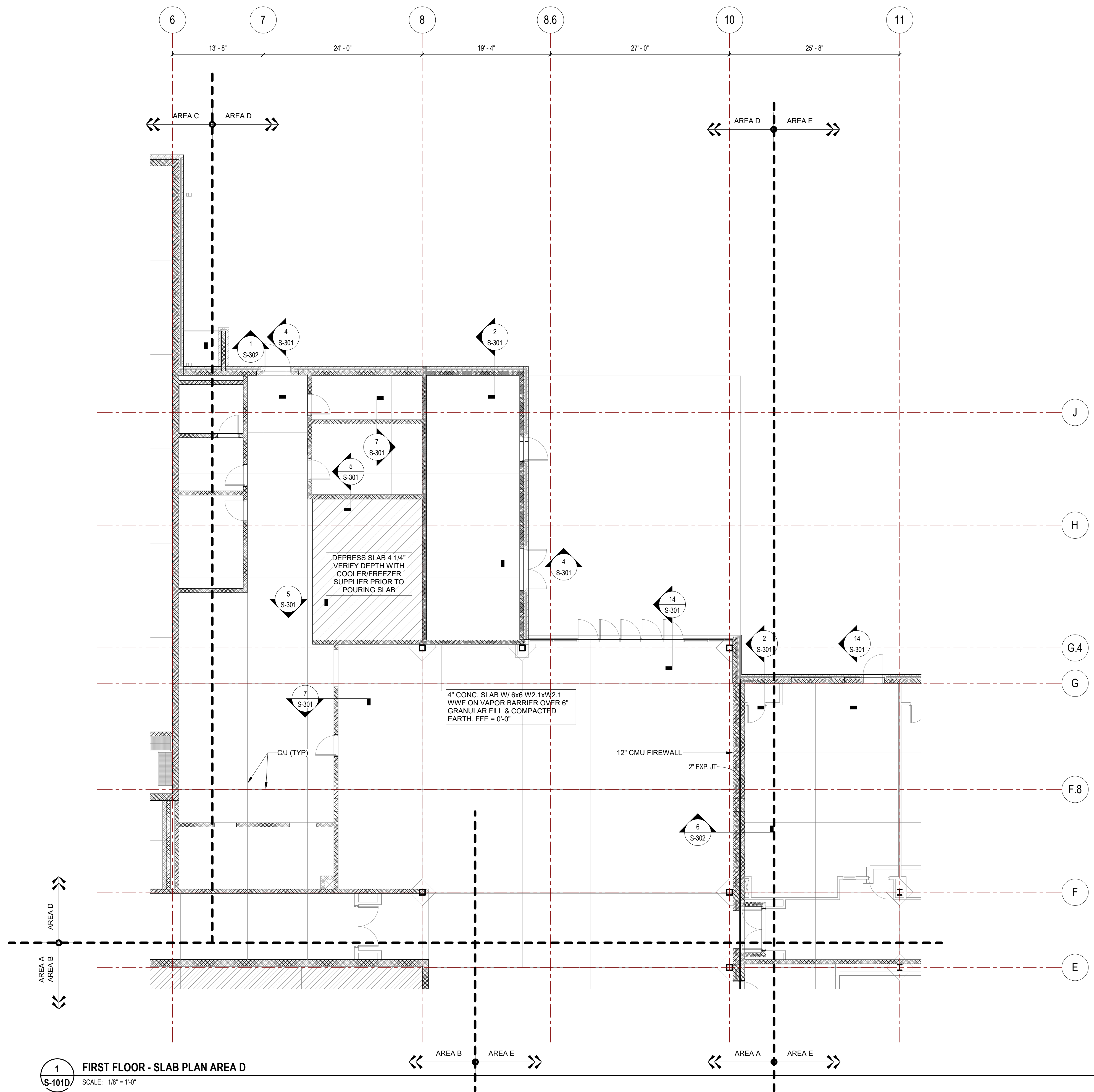
1 FIRST FLOOR - SLAB PLAN AREA C  
S-101C SCALE: 1/8" = 1'-0"

**SLAB PLAN NOTES:**

1. FIN. FLOOR ELEVATION = 0'-0" CORRESPONDS TO ELEVATION 13.80'.
2. C/J DENOTES SLAB CONTROL/CONSTRUCTION JOINT. SEE 11/S-301.
3. SEE S-200 AND S-201 FOR COLUMN SCHEDULE AND TYPICAL DETAILS.
4. SEE S-303 FOR CMU WALL REINFORCING REQUIREMENTS.
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6. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
7. SEE DETAIL 9/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
8. PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
9. PROVIDE #5@48" VERTICAL REINFORCING IN ALL NON-LOAD BEARING WALLS U.O.N.
10. SEE DETAIL 6/S-303 FOR TYPICAL CMU WALL PENETRATION DETAILS.
11. PROVIDE INTERLOCKING CELLS AT CMU WALL CORNERS AND 1-#6 VERT.

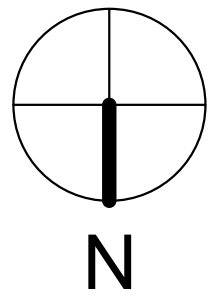
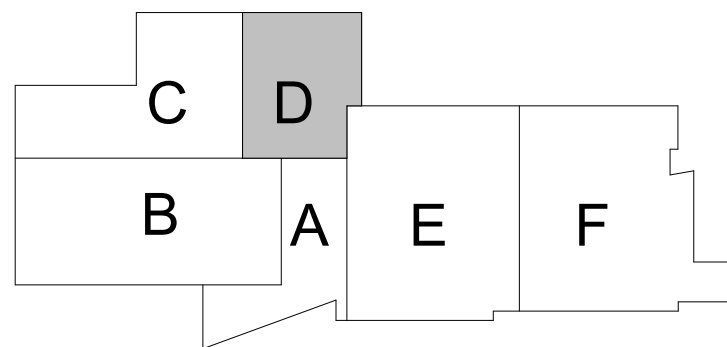






**SLAB PLAN NOTES:**

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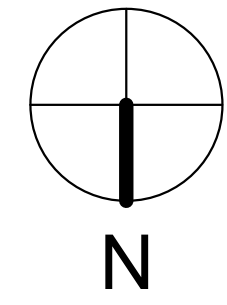
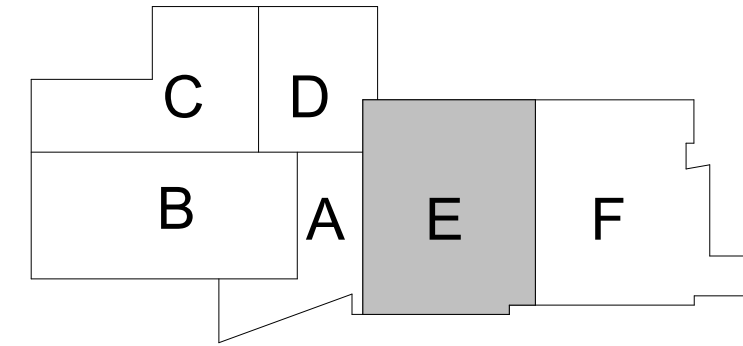
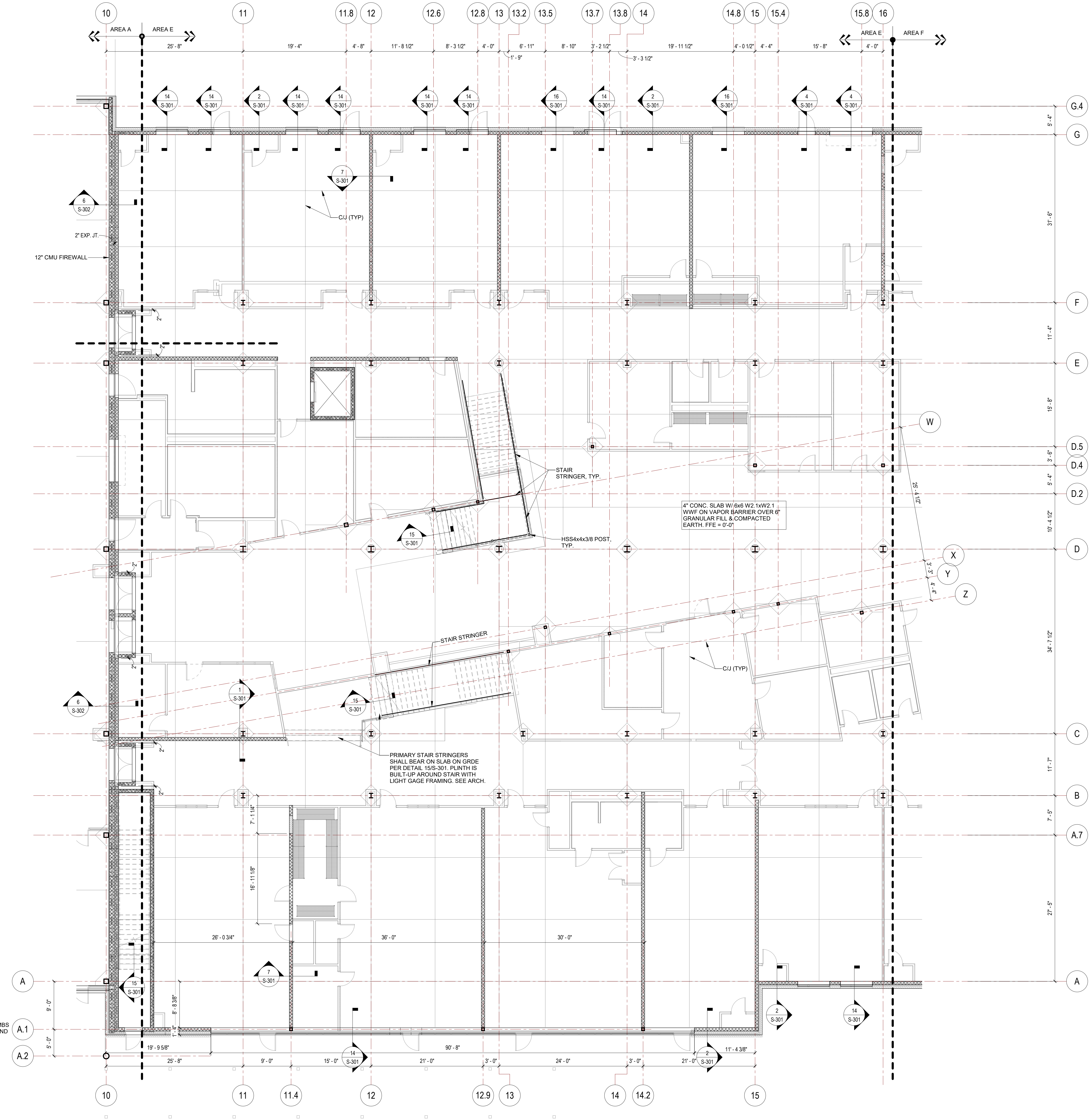




SLAB PLAN NOTES:

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6. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
7. SEE DETAIL 5/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
8. PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
9. PROVIDE #4@48" VERTICAL REINFORCING IN ALL NON-LOAD BEARING WALLS U.O.N.
10. SEE DETAIL 6/S-303 FOR TYPICAL CMU WALL PENETRATION DETAILS.
11. PROVIDE INTERLOCKING CELLS AT CMU WALL CORNERS AND 1-#6 VERT.

1 FIRST FLOOR - SLAB PLAN AREA E  
SCALE: 1/8" = 1'-0"





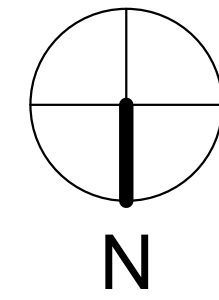
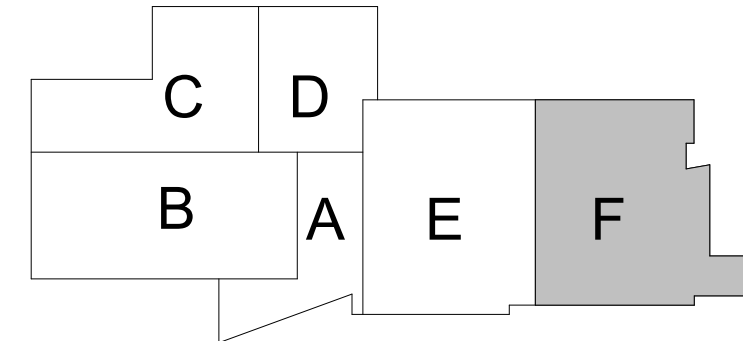
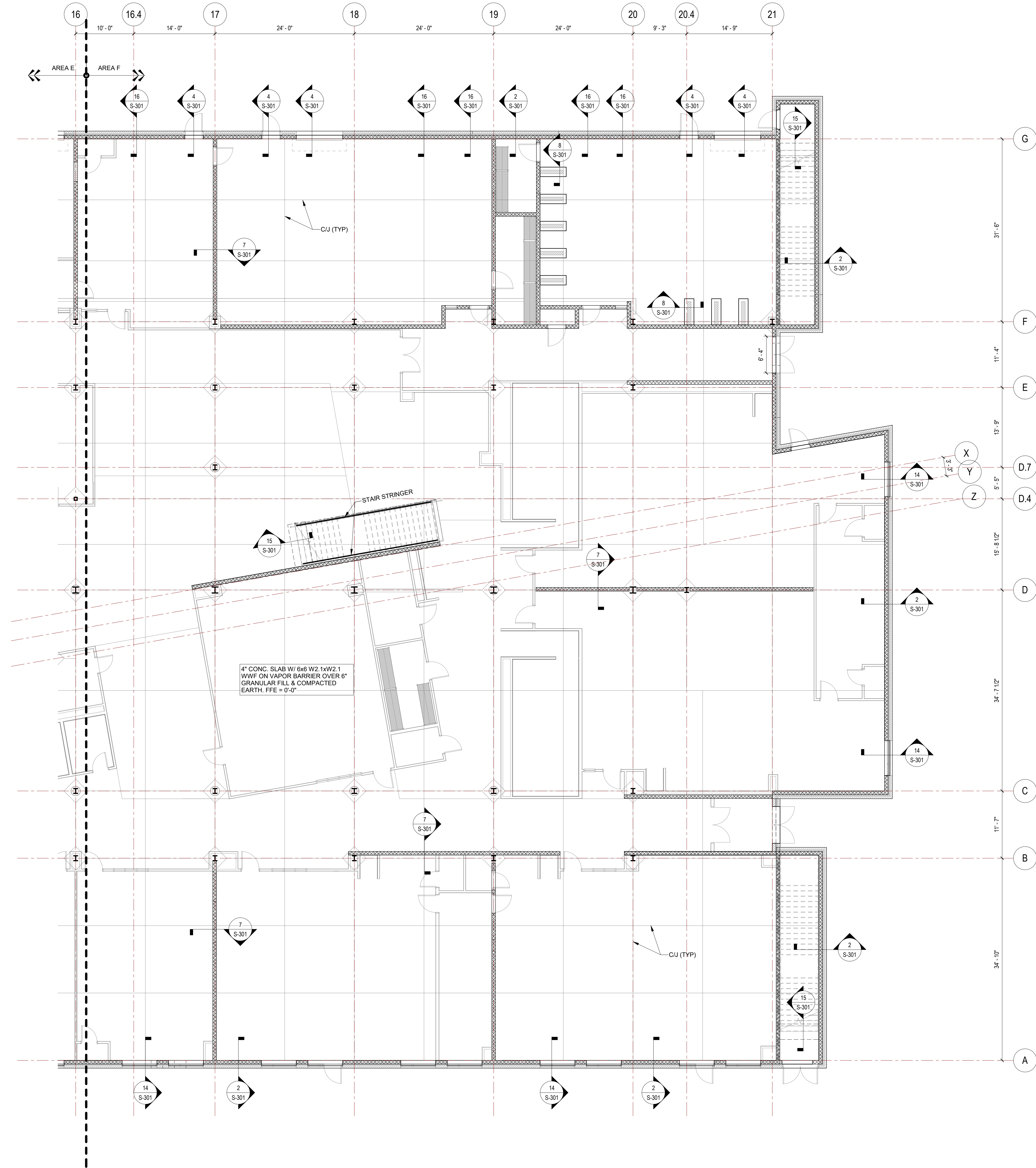
**SLAB PLAN NOTES:**

1. FIN. FLOOR ELEVATION = 0'-0" CORRESPONDS TO ELEVATION 13.80'.
2. C/J DENOTES SLAB CONTROL/CONSTRUCTION JOINT. SEE 11/S-301.
3. SEE S-301 AND S-301 FOR C/J UMN SCHEDULE AND TYPICAL DETAILS.
4. SEE S-303 FOR CMU WALL REINFORCING REQUIREMENTS.
5. IN ADDITION TO REINFORCING SHOWN ON THE DRAWINGS, PROVIDE #6 VERT. BAR IN JAMBS OF ALL DOORS AND WINDOWS. PROVIDE #6 VERT. BAR @ SIDE OF EXPANSION JOINTS AND CONTROL JOINTS. SEE ARCH'L FOR JOINT LOCATIONS. SEE DETAIL 5/S-303 & 6/S-303.
6. REFER TO ARCH'L DRAWINGS FOR INTERIOR WALL DIMENSIONS NOT SHOWN ON STRUCTURAL.
7. SEE DETAIL 9/S-301 FOR SLAB REINFORCING AT RE-ENTRANT CORNERS.
8. PROVIDE BOND BEAMS AT 9'-4" O.C. AND AT ALL BEARING ELEVATIONS IN LOAD BEARING WALLS AND AT TOP COURSE OF ALL WALLS. PROVIDE CORNER BARS IN BOND BEAMS AT WALL CORNERS AND INTERSECTIONS. LAP 2'-0".
9. PROVIDE #5@48" VERTICAL REINFORCING IN ALL NON-LOAD BEARING WALLS U.O.N.
10. SEE DETAIL 6/S-301 FOR TYPICAL CMU WALL PENETRATION DETAILS.
11. PROVIDE INTERLOCKING CELLS AT CMU WALL CORNERS AND 1-#6 VERT.

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S-101F

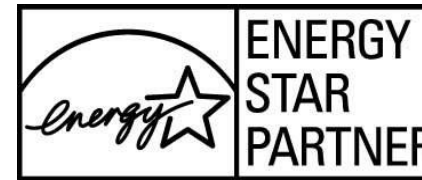
FIRST FLOOR - SLAB PLAN AREA F

SCALE: 1/8" = 1'-0"



CONSTRUCTION  
DOCUMENTS

**PAMLICO COUNTY**  
**PAMLICO 6-12 SCHOOL**  
601 Main Street, Bayboro, NC, 28515

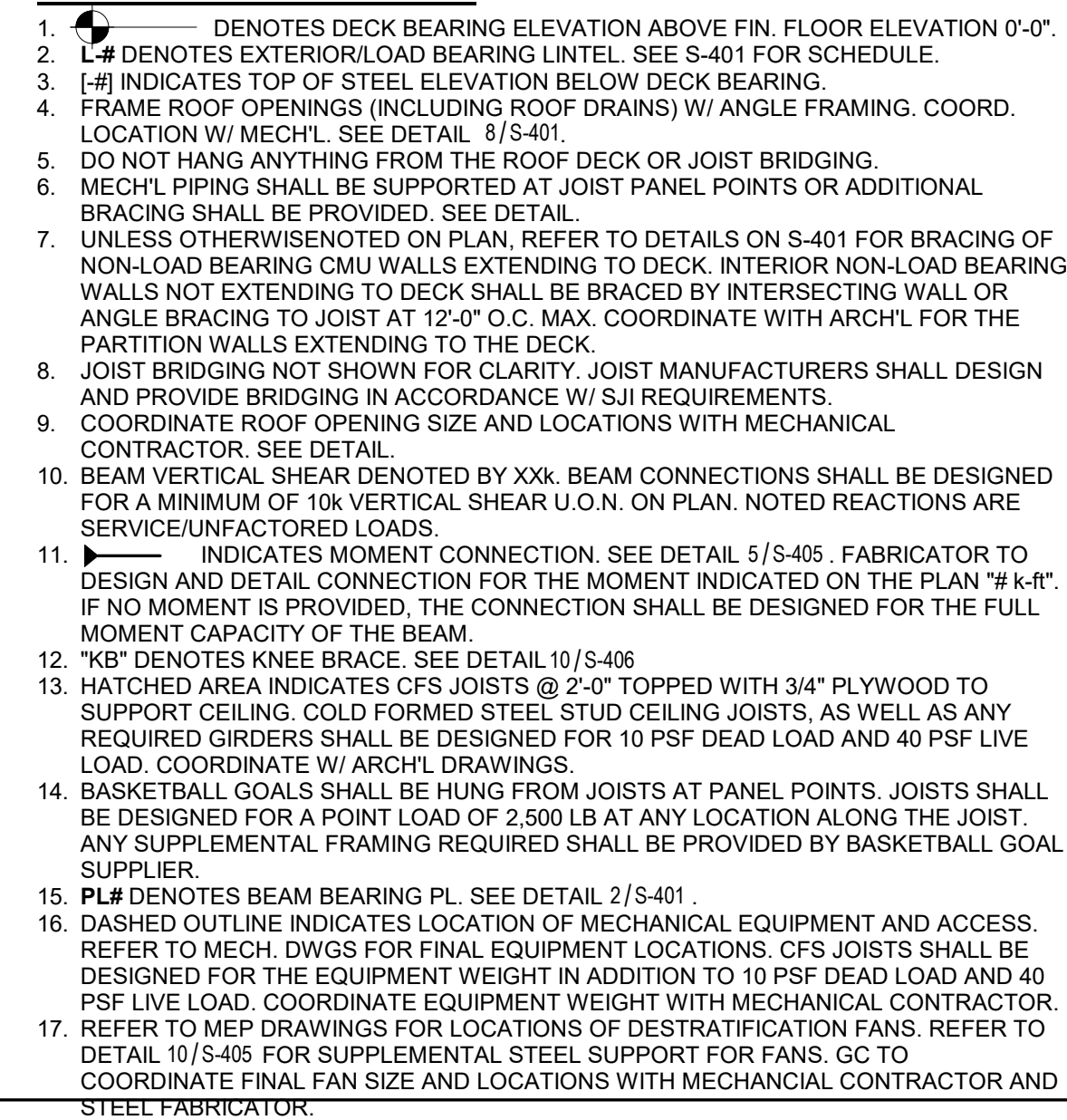


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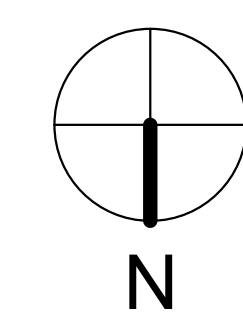
SLAB-ON-GRADE  
PLAN AREA F

S-101F





**S-102A** SCALE: 1/8" = 1'-0"



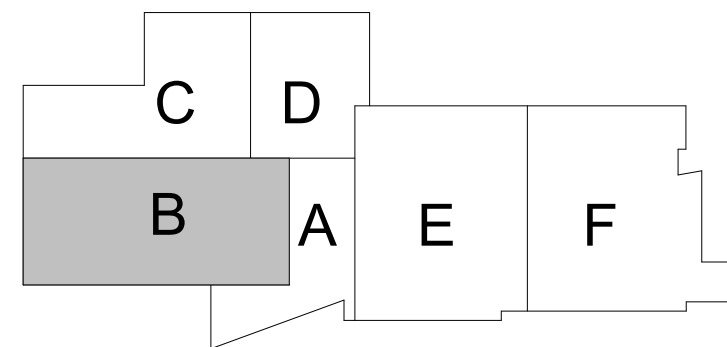


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1 LOW ROOF FRAMING PLAN - AREA B  
S-102B SCALE: 1/8" = 1'-0"

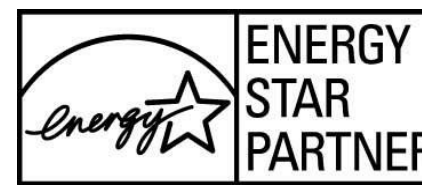
ROOF FRAMING PLAN NOTES:

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. L# DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. (#) INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD.
5. LOCATION W/ MECH. SEE DETAIL 8/S-401.
6. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
7. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
8. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCH. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
9. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
10. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
11. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
12. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "k.k". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
13. "KB" DENOTES KNEE BRACE. SEE DETAIL 10/S-406.
14. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCH. DRAWINGS.
15. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
16. PL# DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
17. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
18. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-406 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.



CONSTRUCTION  
DOCUMENTS

PAMLICO COUNTY  
PAMLICO 6-12 SCHOOL  
601 Main Street, Bayboro, NC, 28515



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LOW ROOF  
FRAMING PLAN  
AREA B

S-102B



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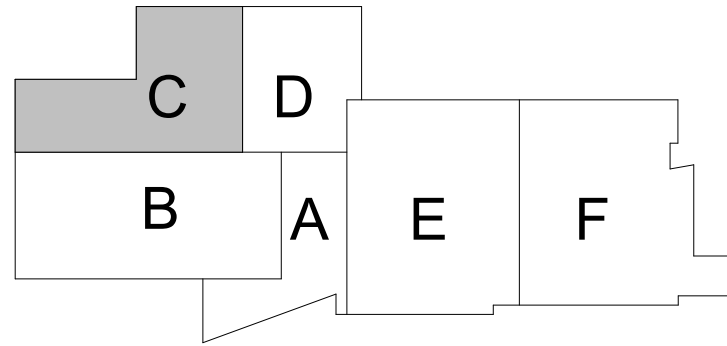
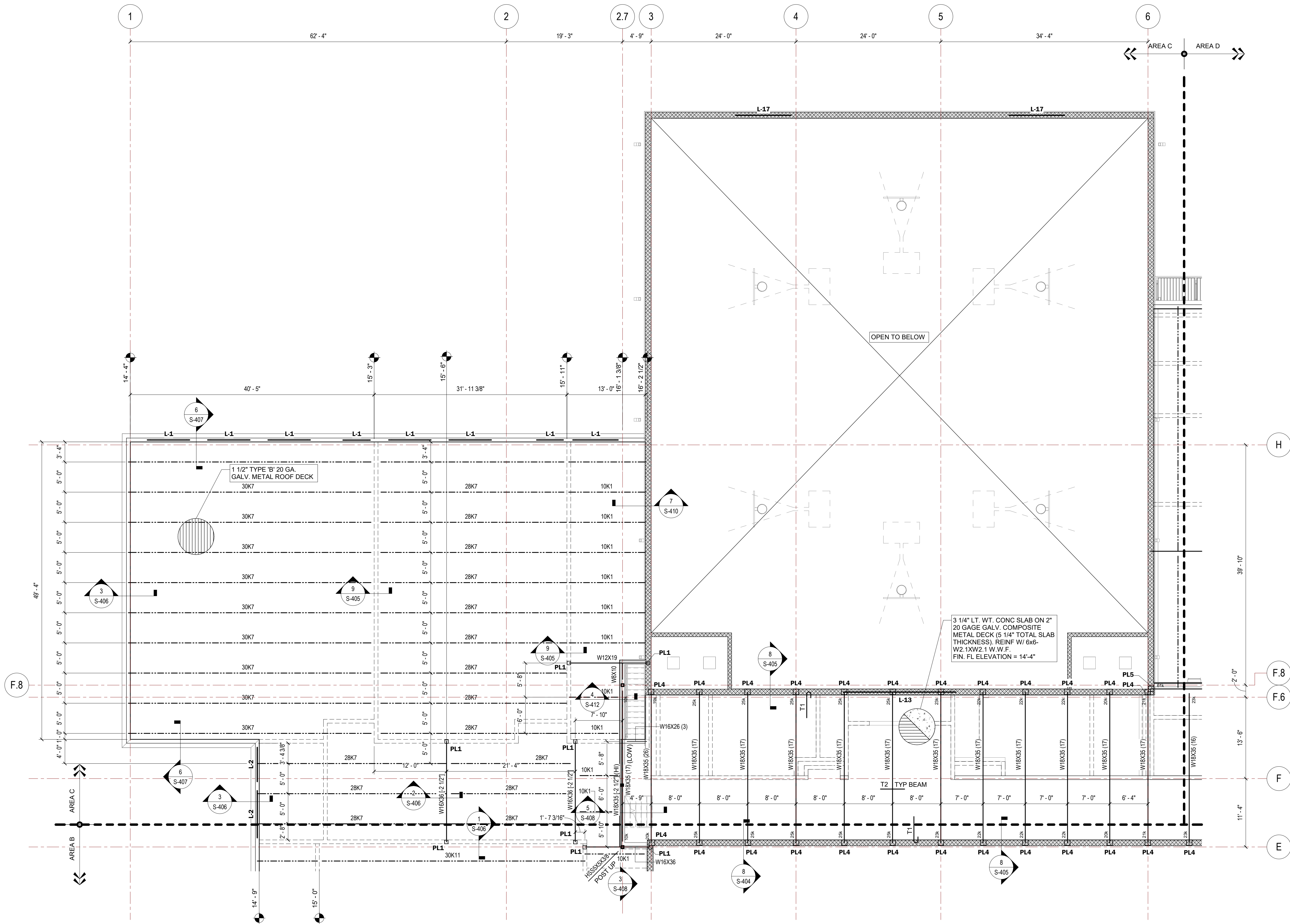
#### ROOF FRAMING PLAN NOTES:

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. L# DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. I-# INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECHL. SEE DETAIL. 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCHL. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL. 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "X K". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "KB" DENOTES KNEE BRACE. SEE DETAIL. 10/S-406.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED ORDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCHL. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. PL# DENOTES BEAM BEARING PL. SEE DETAIL. 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL. 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

#### FLOOR FRAMING NOTES:

1. TOP OF STEEL ELEVATION = 5 1/4" BELOW FIN. FLOOR ELEV. U.O.N. SEE PLAN FOR FINISHED FLOOR ELEVATION.
2. ALL ELEVATIONS REFERENCED FROM 0'-0" U.O.N.
3. NUMBERS IN PARENTHESIS DENOTES QUANTITY OF 3/4"Ø x 4" STUDS EQUALLY SPACED ON BEAM. SEE 3/S-406 FOR COMPOSITE BEAM LEGEND.
4. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
5. INDICATES MOMENT CONNECTION. SEE DETAIL. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "X K". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
6. L# DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
7. T# DENOTES SLAB TOP BARS. SEE DETAIL. 10/S-402 & 11/S-402.
8. PL# DENOTES BEAM BEARING PL. SEE SCHEDULE AND DETAILS.
9. FLOOR FRAMING AND COMPOSITE FLOOR DECK ARE DESIGNED TO REMAIN UNSHORED DURING CONCRETE PLACEMENT. ACCOUNT FOR AN EXPECTED DEFLECTION IN BEAMS AND GIRDERS OF UP TO 1/360 OF THE SPAN LENGTH (IN INCHES) OR 1", WHICHEVER IS LESS WHEN CALCULATING CONCRETE QUANTITIES. FINISH SUPPORTED SLABS FLAT AND LEVEL.
10. CONSTRUCTION JOINTS IN ELEVATED CONCRETE ON METAL DECK POURS SHALL BE SUBMITTED FOR REVIEW PRIOR TO CONSTRUCTION. SEE DETAIL. 8/S-402.
11. FRAMED FLOOR OPENINGS COORD. LOCATION W/ MECHL. CONTRACTOR. SEE DETAIL. 7/S-405.

1 SECOND FLOOR & LOW ROOF FRAMING PLAN - AREA C  
S-102C SCALE: 1/8" = 1'-0"



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CONSTRUCTION  
DOCUMENTS

PAMLICO COUNTY  
PAMLICO 6-12 SCHOOL  
601 Main Street, Bayboro, NC, 28515



No.	Date	Description
06/12/2024	06/12/2024	BID DOCUMENTS
ISSUE DATE:	06/12/2024	
PROJECT #:	23.08.034	
DRAWN BY:	AS	
CHECKED BY:	TH	

SECOND FLOOR &  
LOW ROOF  
FRAMING PLAN  
AREA C

S-102C



ROOF FRAMING PLAN NOTES:

- 1.
- 2.
- 3.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING, COORD. LOCATION W/ MECH. SEE DETAIL 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCH. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "k-k". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "KB" DENOTES KNEE BRACE. SEE DETAIL 10/S-405.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCH. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

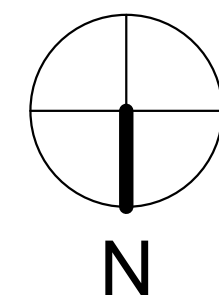
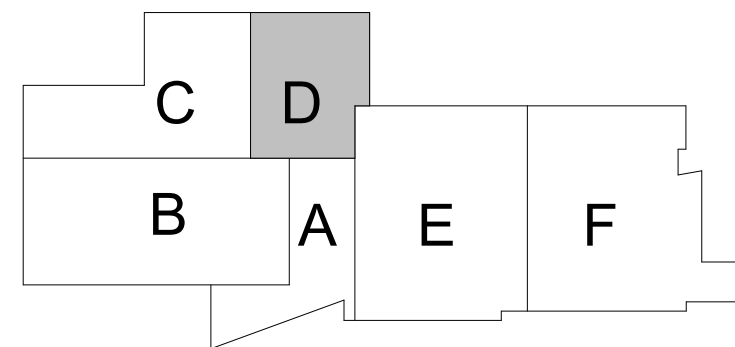
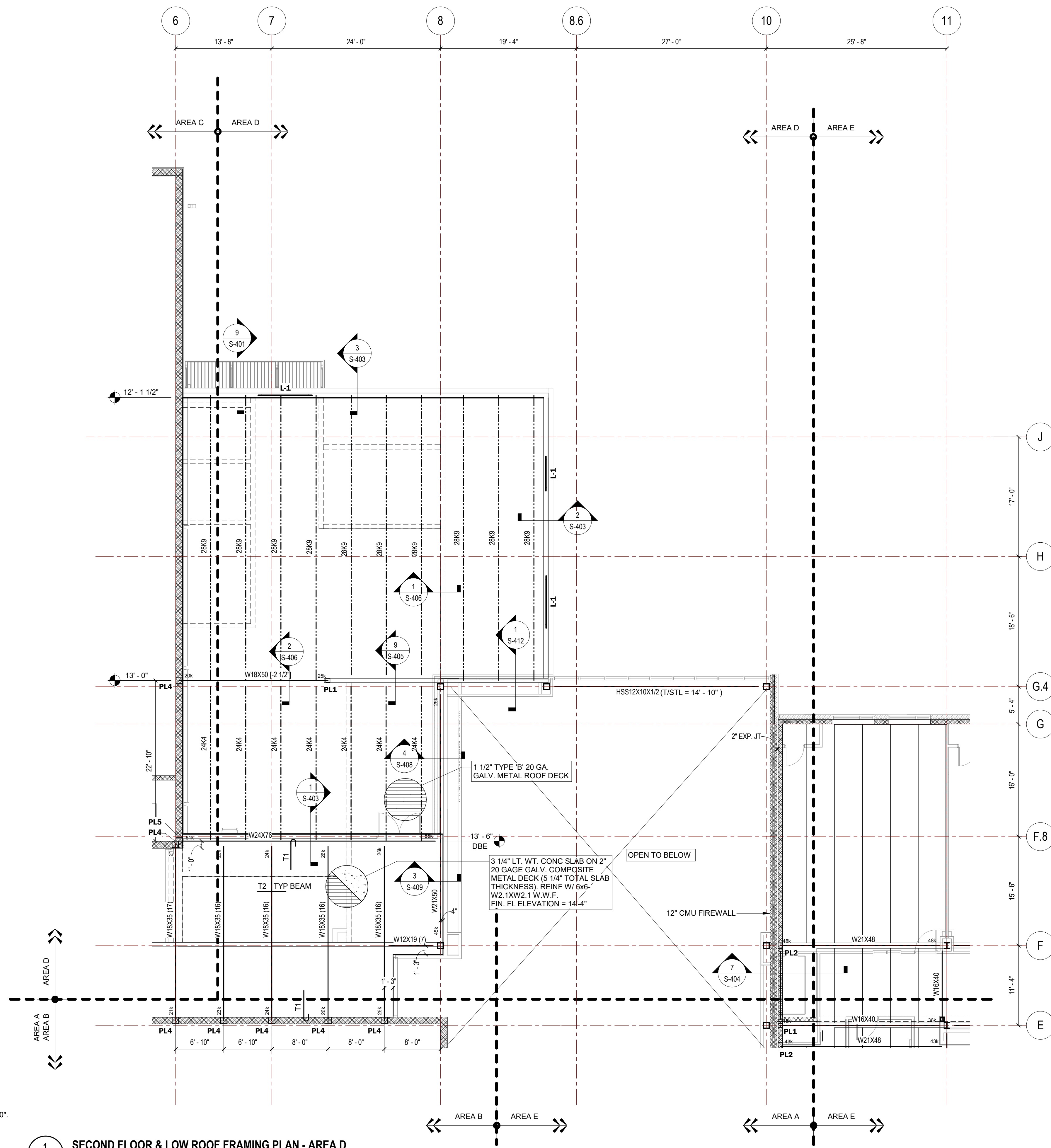
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S-102D

SECOND FLOOR & LOW ROOF FRAMING PLAN - AREA D

SCALE: 1/8" = 1'-0"

FLOOR FRAMING NOTES:

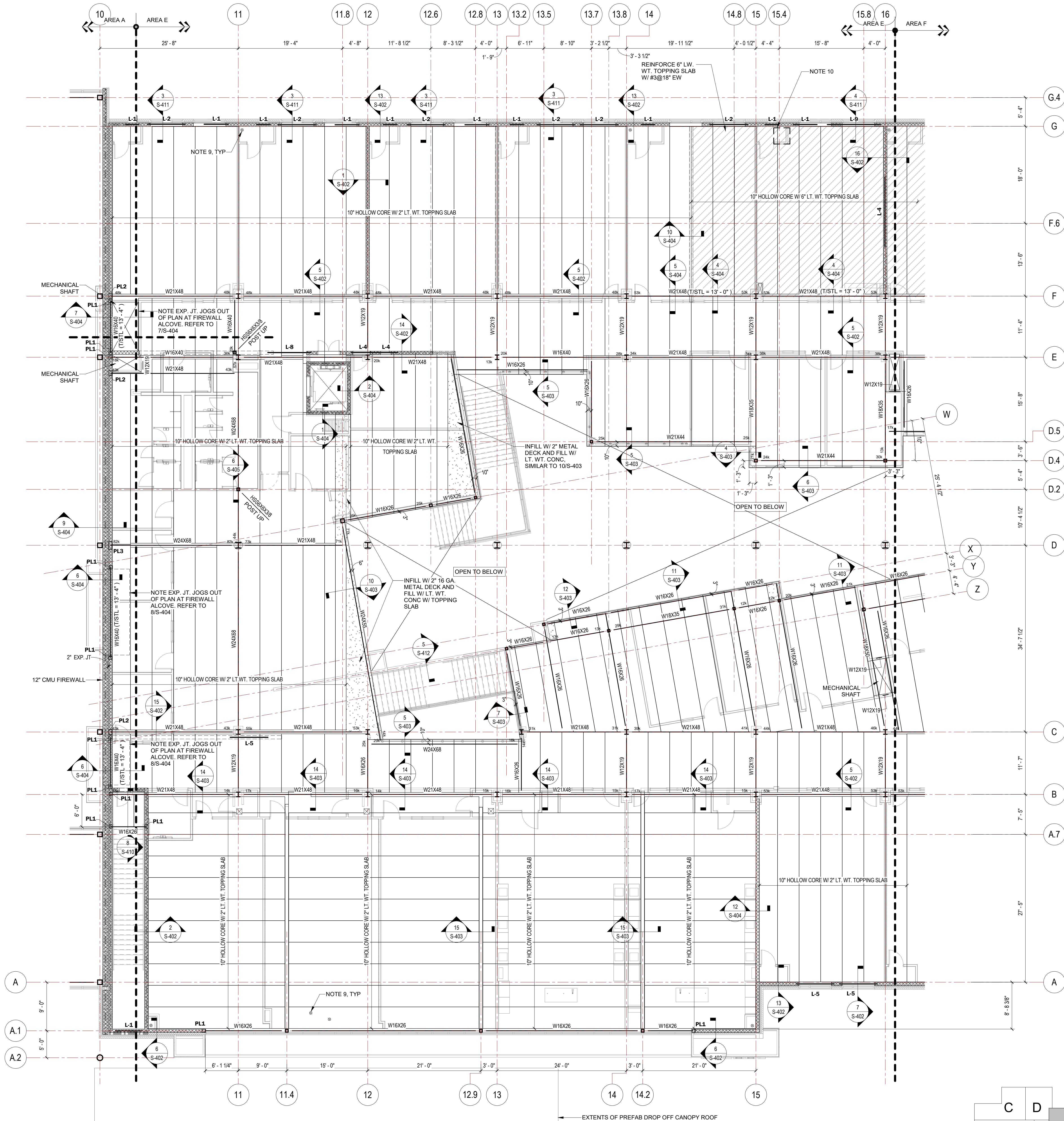
1. TOP OF 2" TOPPING SLAB ELEVATION = 14'-4". TOPPING SLAB SHALL BE REINFORCED WITH 6x6 W14xw14 W.W.F.
2. BOTTOM OF HOLLOW CORE PLANKS ELEVATION = 13'-4". TOP OF STEEL SHALL BE AT BOTTOM OF HOLLOW CORE PLANKS, U.O.N.
3. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "k-k". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
- 4.
- 5.
6. BEAM VERTICAL SHEAR DENOTED BY BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
7. DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
8. SEE DETAIL 6/S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO BOTTOM OF HOLLOW CORE. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. SEE DETAIL 5/S-401. COORDINATE WITH ARCH. THE WALLS EXTENDING TO THE DECK.
9. HOLLOW CORE PENETRATION. REFER TO MECHANICAL DRAWINGS FOR OPENING SIZE AND FINAL LOCATIONS. HOLLOW CORE SUPPLIER SHALL DESIGN HOLLOW CORE PLANKS ACCOUNTING FOR OPENINGS DENOTED ON PLANS.
10. ELEC. TRANSFORMER HUNG FROM UNDERSIDE OF HOLLOW CORE PLANKS ON UNISTRUT FRAME. REFER TO ELEC. DWGS FOR FINAL EQUIPMENT LOCATION. HOLLOW CORE PLANKS SHALL BE DESIGNED TO SUPPORT THE WEIGHT OF THE EQUIPMENT AND FRAME. COORD. W/ EQUIPMENT SUPPLIER FOR FINAL EQUIPMENT DIMENSIONS AND WEIGHT.



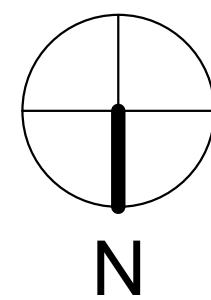
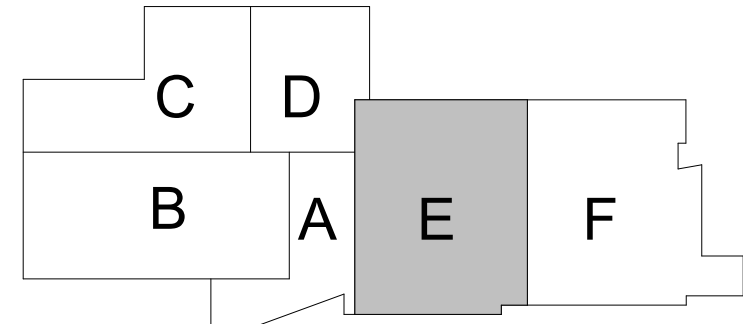


**FLOOR FRAMING NOTES:**

1. TOP OF 2" TOPPING SLAB ELEVATION = 14'-4". TOPPING SLAB SHALL BE REINFORCED WITH 6x6 W14xW14 W.W.F.
2. BOTTOM OF HOLLOW CORE PLANKS ELEVATION = 13'-4". TOP OF STEEL SHALL BE AT BOTTOM OF HOLLOW CORE PLANKS, U.O.N.
3. ——— INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "16-K-1". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
4. ——— DENOTES TOP OF DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION = 0'-0" U.O.N.
5. L# DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
6. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE UNFACTORED LOADS.
7. PL# DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
8. SEE DETAIL 6/S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO BOTTOM OF HOLLOW CORE. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. SEE DETAIL 5/S-401. COORDINATE WITH ARCH'L THE WALLS EXTENDING TO THE DECK.
9. HOLLOW CORE PENETRATION. REFER TO MECHANICAL DRAWINGS FOR OPENING SIZE AND FINAL LOCATIONS. HOLLOW CORE SUPPLIER SHALL DESIGN HOLLOW CORE PLANKS ACCOUNTING FOR OPENINGS DENOTED ON PLANS.
10. ELEC. TRANSFORMER HUNG FROM UNDERSIDE OF HOLLOW CORE PLANKS ON UNISTRUT FRAME. REFER TO ELEC DWGS FOR FINAL EQUIPMENT LOCATION. HOLLOW CORE PLANKS SHALL BE DESIGNED TO SUPPORT THE WEIGHT OF THE EQUIPMENT AND FRAME. COORD. W/ EQUIPMENT SUPPLIER FOR FINAL EQUIPMENT DIMENSIONS AND WEIGHT.

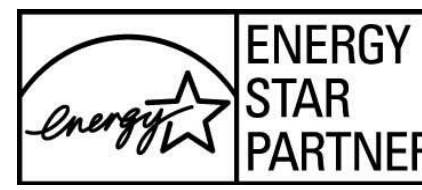


**1 SECOND FLOOR FRAMING PLAN - AREA E**  
SCALE: 1/8" = 1'-0"



CONSTRUCTION  
DOCUMENTS

**PAMLICO COUNTY  
PAMLICO 6-12 SCHOOL**  
601 Main Street, Bayboro, NC, 28515



No.	Date	Description
06/12/2024	06/12/2024	BID DOCUMENTS
ISSUE DATE:	06/12/2024	
PROJECT #:	23.08.034	
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SECOND FLOOR  
FRAMING PLAN  
AREA E

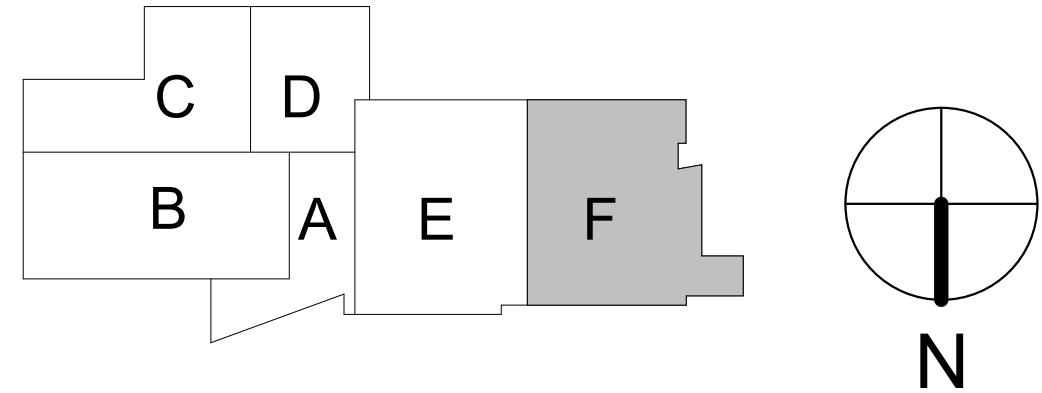
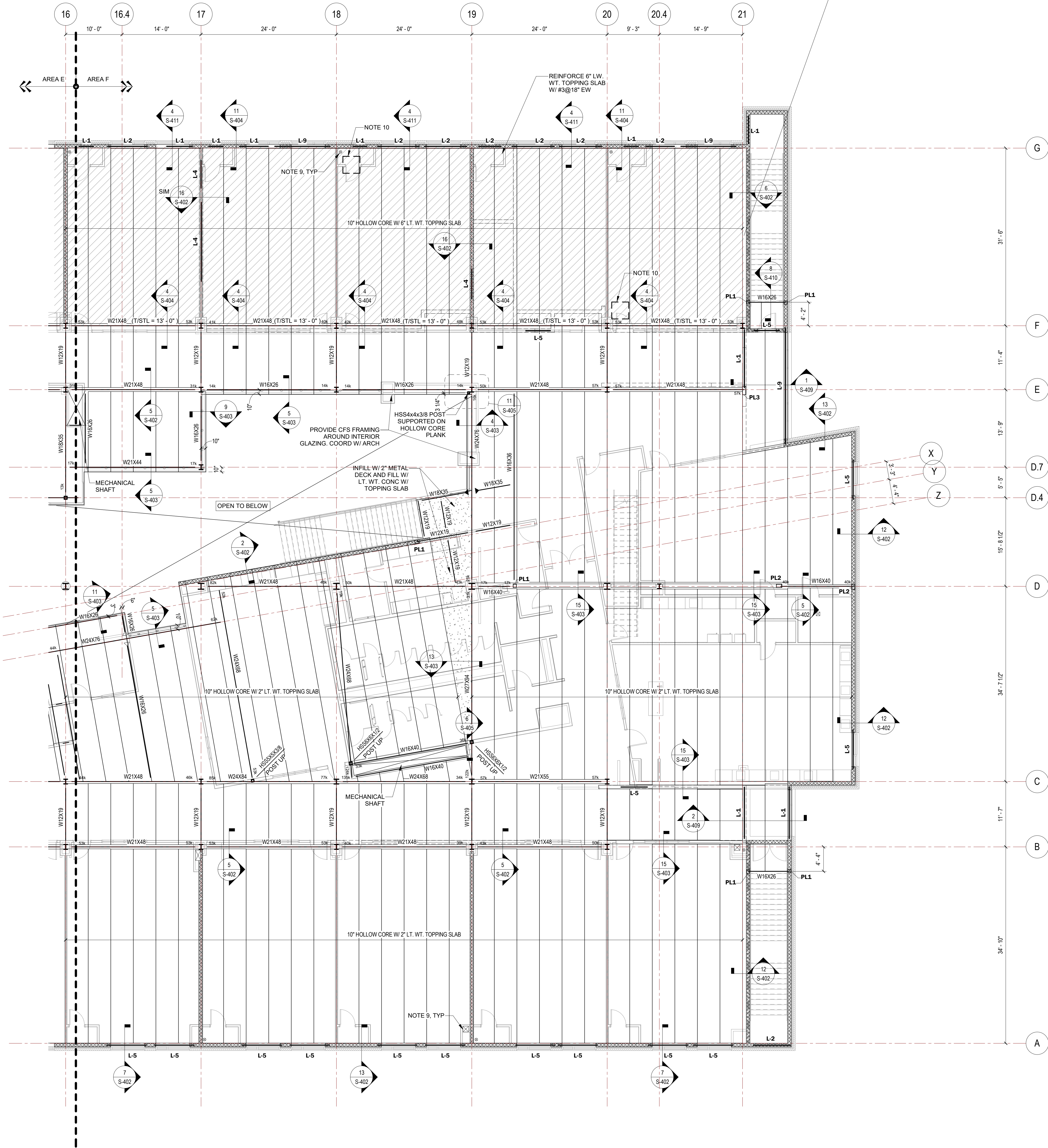


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FLOOR FRAMING NOTES:

- TOP OF 2" TOPPING SLAB ELEVATION = 14'-4". TOPPING SLAB SHALL BE REINFORCED WITH 6x6 W14XW14 W.W.F.
- BOTTOM OF HOLLOW CORE PLANKS ELEVATION = 13'-4". TOP OF STEEL SHALL BE AT BOTTOM OF HOLLOW CORE PLANKS, U.O.N.
- INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "N&R". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
- DENOTES TOP OF DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION = 0'-0" U.O.N.
- L# DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
- BEAM VERTICAL SHEAR DENOTED BY XX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE UNFACTORED LOADS.
- PL# DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
- SEE DETAIL 6/S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO BOTTOM OF HOLLOW CORE. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. SEE DETAIL 5/S-401. COORDINATE WITH ARCH'L THE WALLS EXTENDING TO THE DECK.
- HOLLOW CORE PENETRATION. REFER TO MECHANICAL DRAWINGS FOR OPENING SIZE AND FINAL LOCATIONS. HOLLOW CORE SUPPLIER SHALL DESIGN HOLLOW CORE PLANKS ACCOUNTING FOR OPENINGS DENOTED ON PLANS.
- ELEC. TRANSFORMER HUNG FROM UNDERSIDE OF HOLLOW CORE PLANKS ON UNISTRUT FRAME. REFER TO ELEC DWGS FOR FINAL EQUIPMENT LOCATION. HOLLOW CORE PLANKS SHALL BE DESIGNED TO SUPPORT THE WEIGHT OF THE EQUIPMENT AND FRAME. COORD. W/ EQUIPMENT SUPPLIER FOR FINAL EQUIPMENT DIMENSIONS AND WEIGHT.

1 SECOND FLOOR FRAMING PLAN - AREA F  
SCALE: 1/8" = 1'-0"



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PROJECT #:		23.08.034
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SECOND FLOOR  
FRAMING PLAN  
AREA F

S-102F

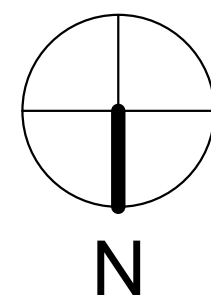
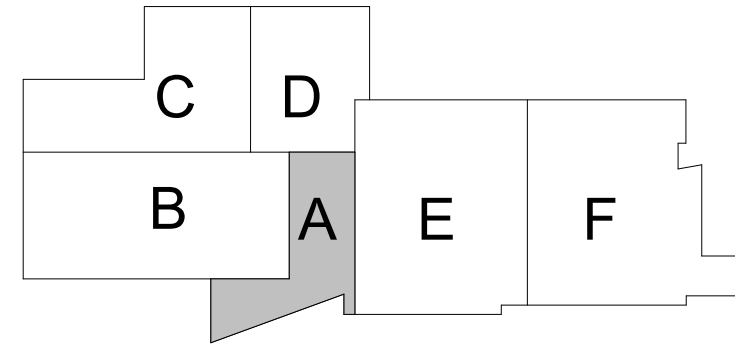
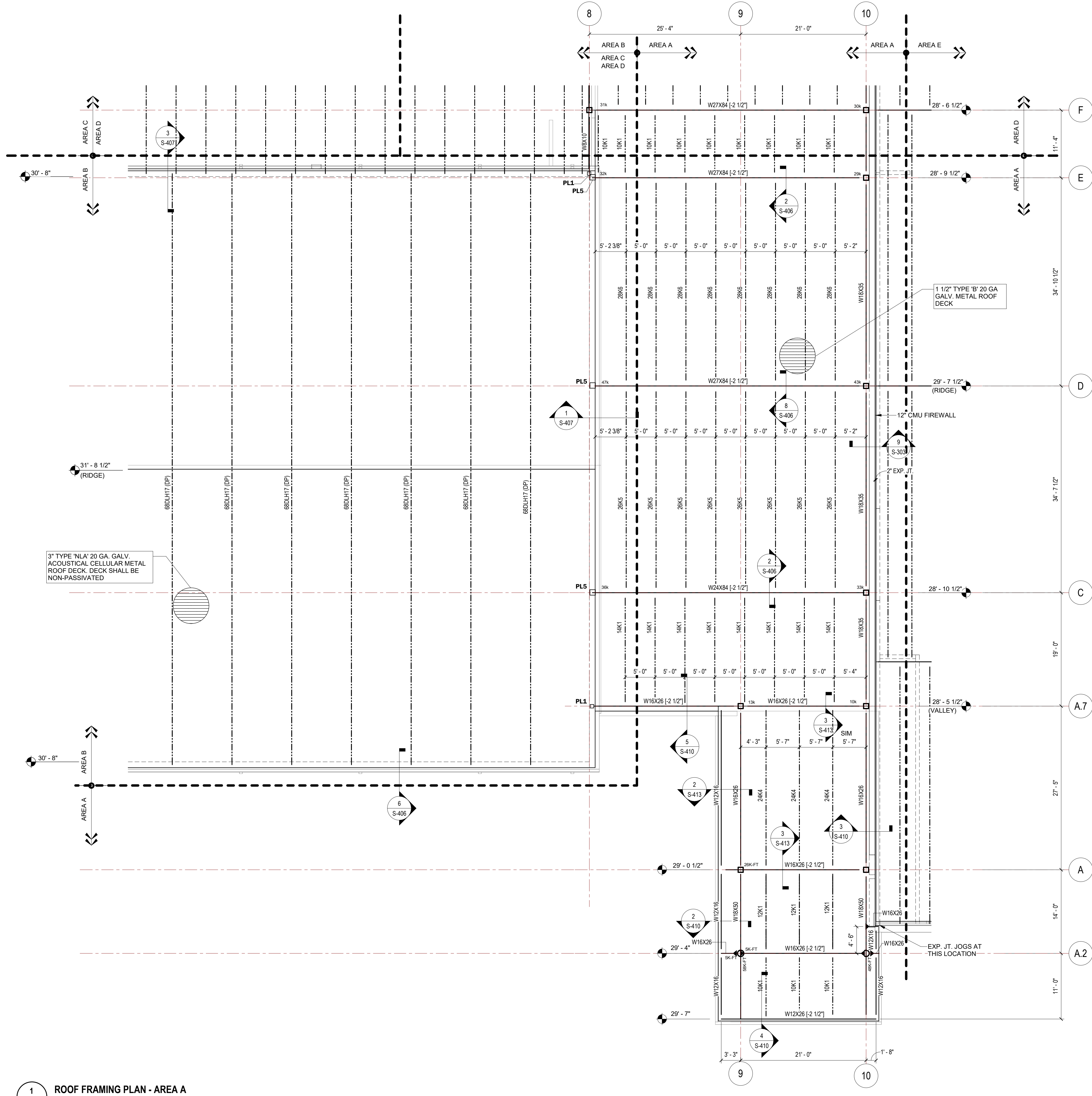


ROOF FRAMING PLAN NOTES:

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECHL. SEE DETAIL. 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECHL. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCHL. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "K-k". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "K8" DENOTES KNEE BRACE. SEE DETAIL 10/S-406.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCHL. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

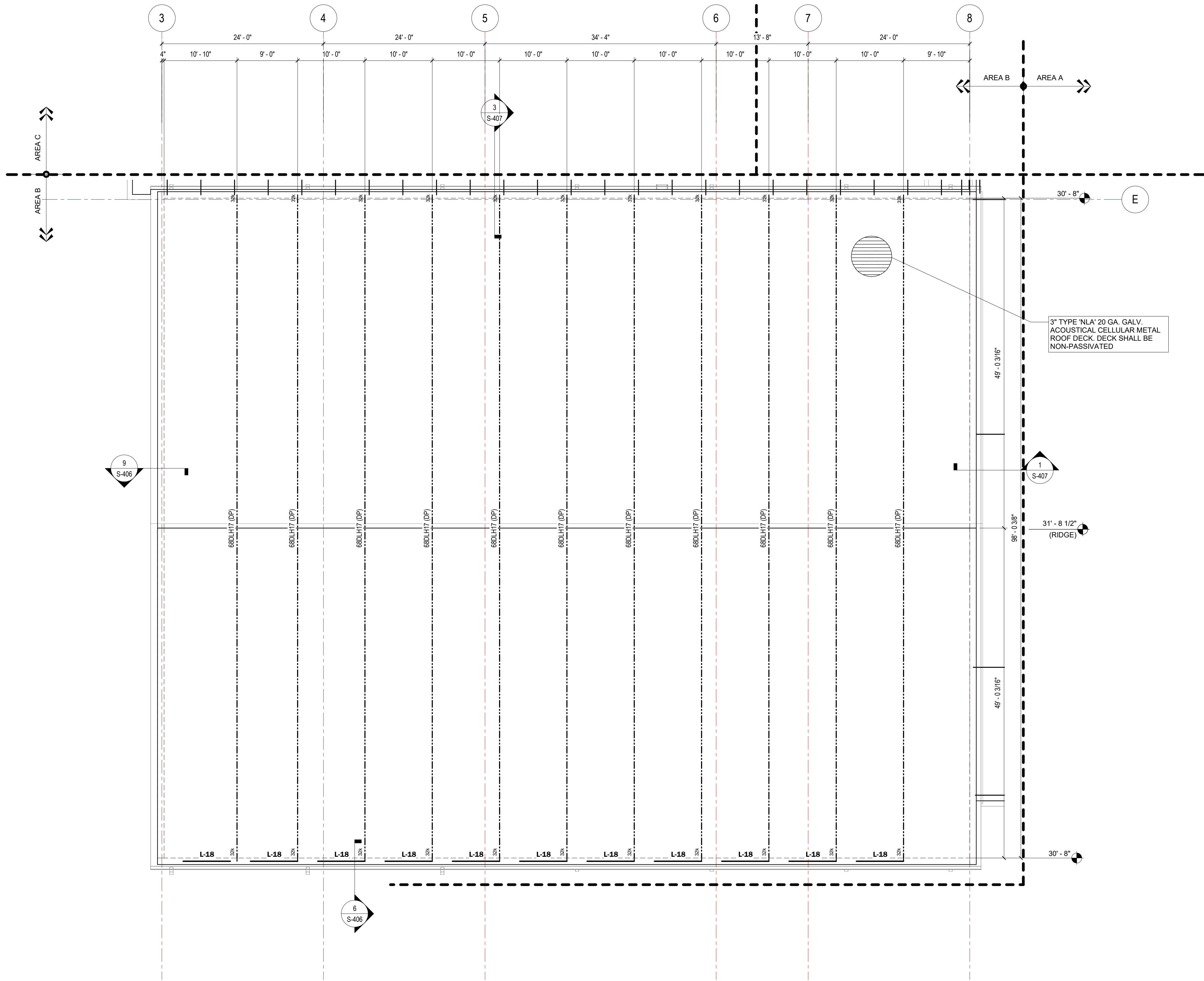
1 ROOF FRAMING PLAN - AREA A

SCALE: 1/8" = 1'-0"





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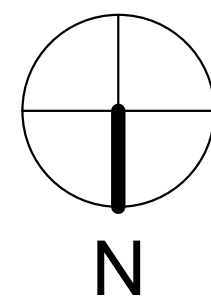
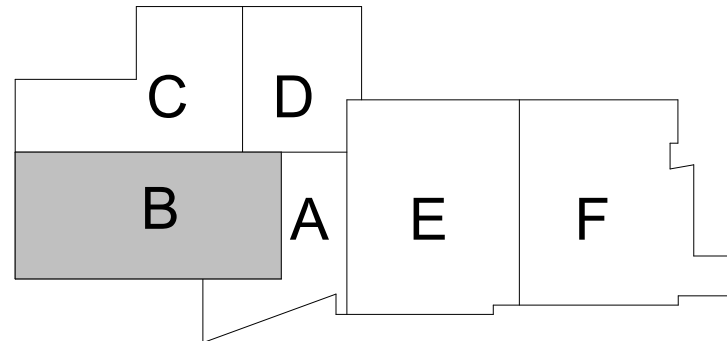
**ROOF FRAMING PLAN NOTES:**

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. **L-#** DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. **[#]** INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECH'L. SEE DETAIL 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH'L PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCH'L FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJ REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY VXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "K+K". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "KB" DENOTES KNEE BRACE. SEE DETAIL 10/S-406.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCH'L DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LBS AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. **PL#** DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-406 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. CO TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

1  
S-103B

**ROOF FRAMING PLAN - AREA B**

SCALE: 1/8" = 1'-0"



...Becoming the  
Leading Designer of  
High Performance Facilities  
In the Nation with a  
Specialty in Alternative  
Delivery Methods

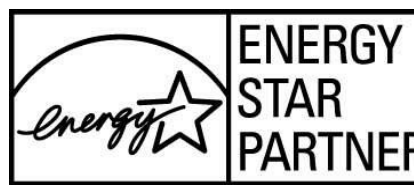
333 Fayetteville St., Ste. 225  
Raleigh, NC 27601  
P: 919.573.6350  
F: 919.573.6355  
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ARCHITECTS



CONSTRUCTION  
DOCUMENTS

**PAMLICO COUNTY**  
**PAMLICO 6-12 SCHOOL**  
601 Main Street, Bayboro, NC, 28515

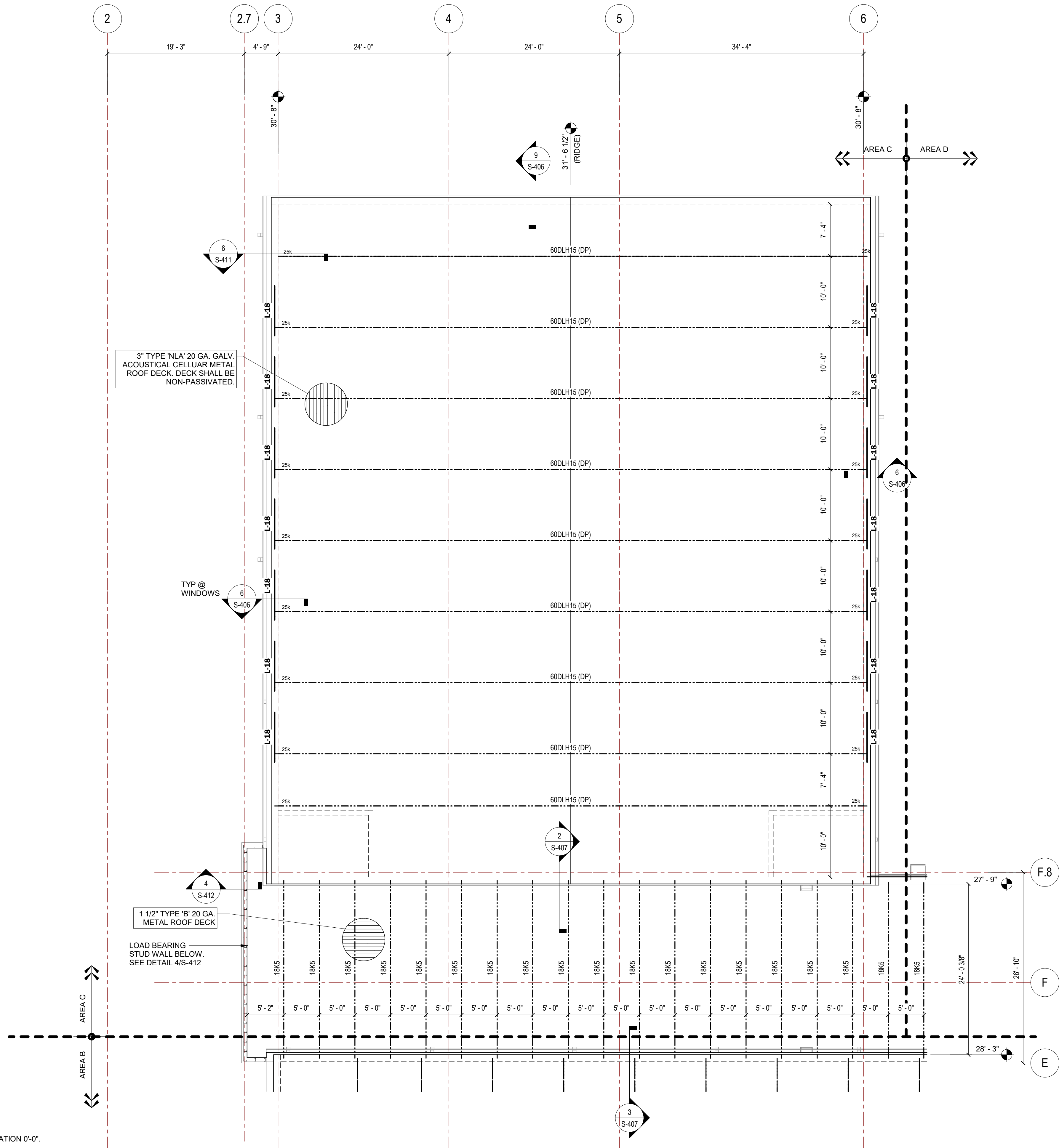


No.	Date	Description
06/12/2024		BID DOCUMENTS
ISSUE DATE: 06/12/2024		
PROJECT #:	23.08.034	
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CHECKED BY:	TH	

ROOF FRAMING  
PLAN AREA B

**S-103B**

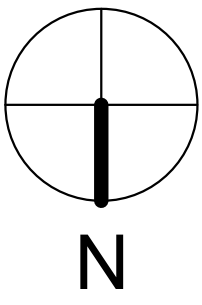
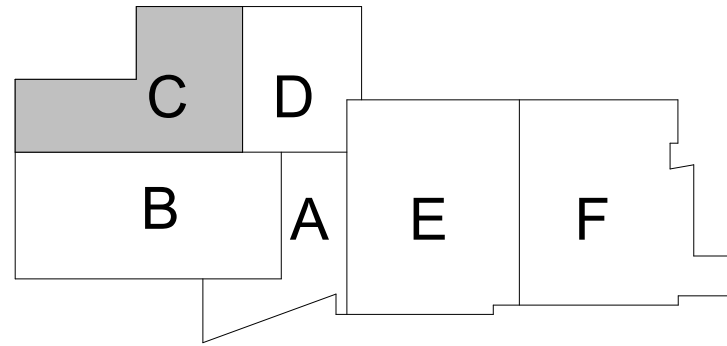




**ROOF FRAMING PLAN NOTES:**

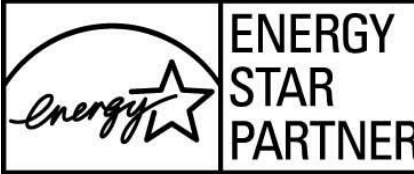
1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECH. L. SEE DETAIL 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCH. L. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY XXx. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "k-k". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "KB" DENOTES KNEE BRACE. SEE DETAIL 10/S-406.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCH. L. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

**1 ROOF FRAMING PLAN - AREA C**  
SCALE: 1/8" = 1'-0"



CONSTRUCTION  
DOCUMENTS

**PAMLICO COUNTY**  
**PAMLICO 6-12 SCHOOL**  
601 Main Street, Bayboro, NC, 28515

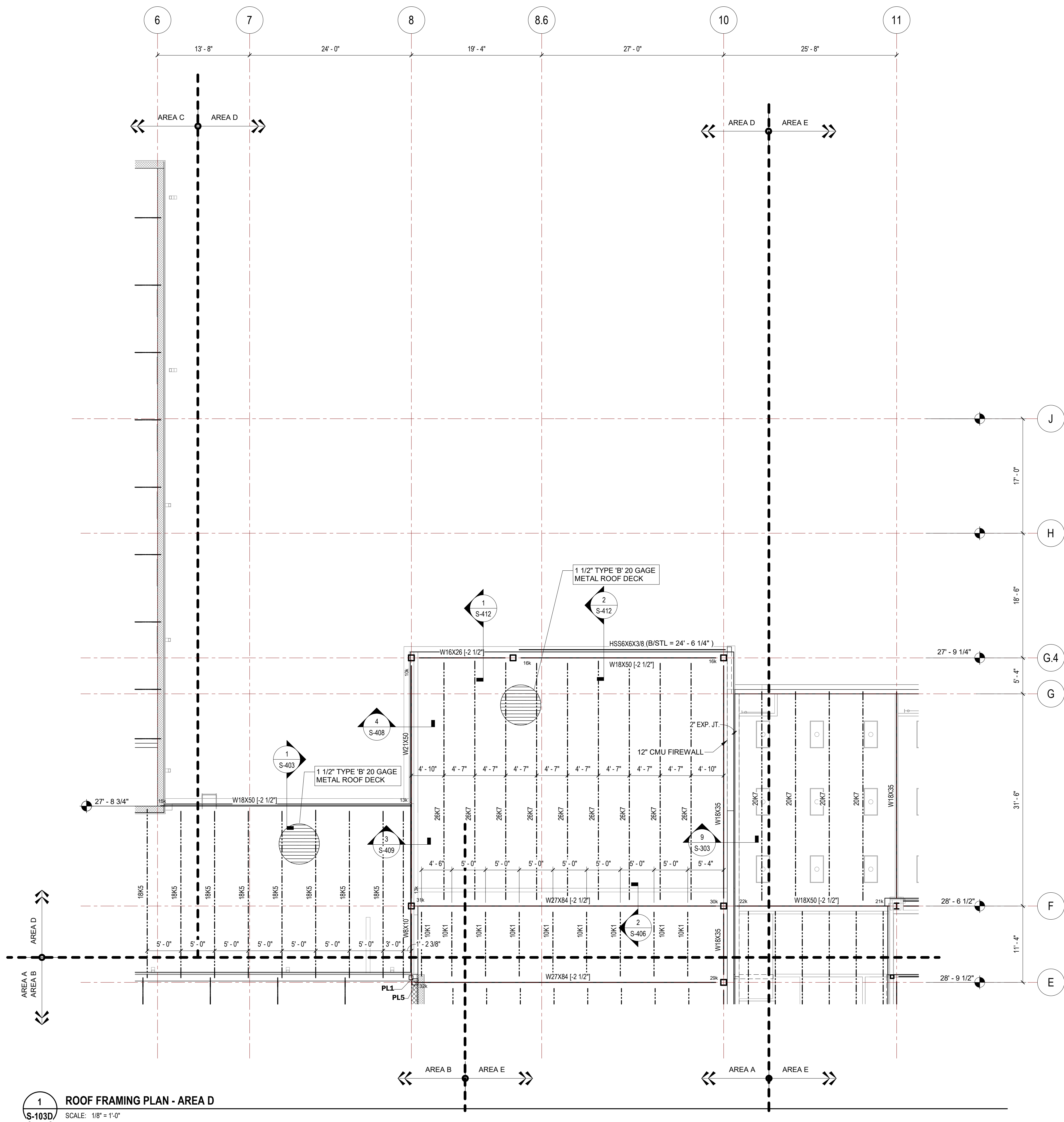


No.	Date	Description
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ISSUE DATE:	06/12/2024	
PROJECT #:	23.08.034	
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ROOF FRAMING  
PLAN AREA C

**S-103C**





ROOF FRAMING PLAN NOTES:

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. (H) INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECHL. SEE DETAIL 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECHL. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCHT. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY XXk. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10k VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "k-k". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "KB" DENOTES KNEE BRACE. SEE DETAIL 10/S-405.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCHT. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. **PL#** DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DIVS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.



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#### FLOOR FRAMING NOTES:

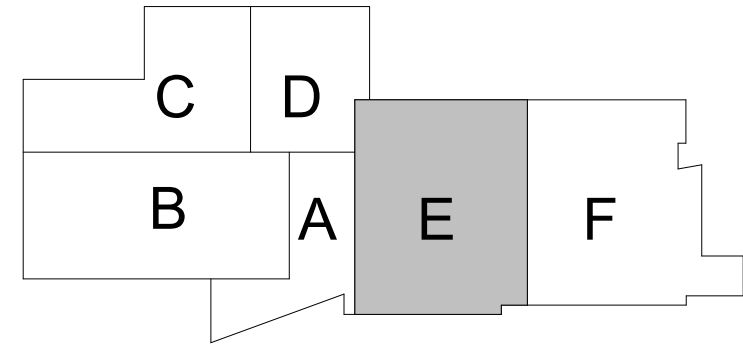
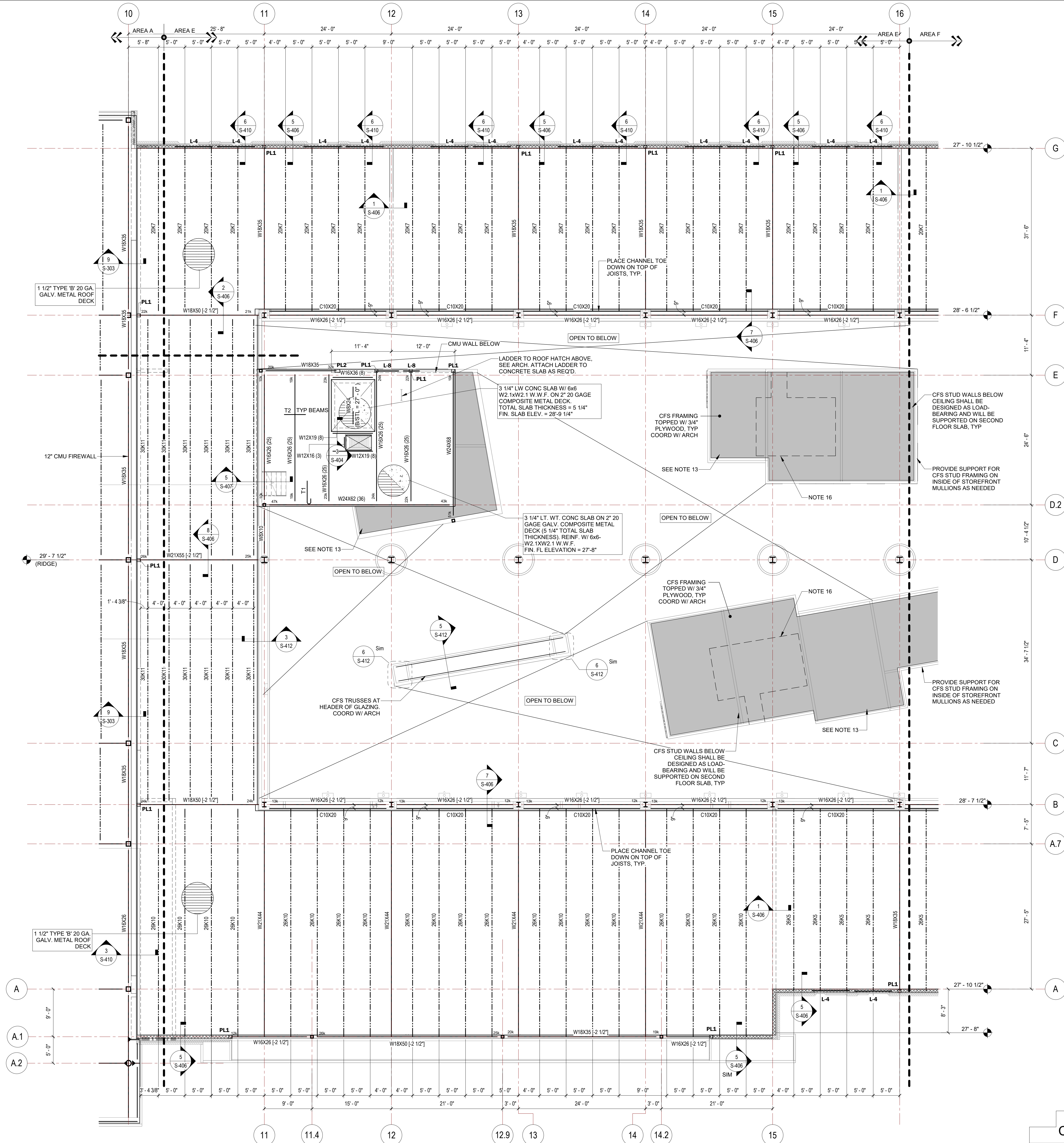
1. TOP OF STEEL ELEVATION = 5' 1/4" BELOW FIN. FLOOR ELEV. U.O.N. SEE PLAN FOR FINISHED FLOOR ELEVATION.
2. ALL ELEVATIONS REFERENCED FROM 0'-0" U.O.N.
3. NUMBERS IN PARENTHESIS DENOTES QUANTITY OF 3/4"x4" STUDS EQUALLY SPACED ON BEAM. SEE 3/S-406 FOR COMPOSITE BEAM LEGEND.
4. BEAM VERTICAL SHEAR DENOTED BY 'XX'. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
5. INDICATES MOMENT CONNECTION. SEE DETAIL. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN. IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
6. L# DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
7. T# DENOTES SLAB TOP BARS. SEE DETAIL 10/S-402 & 11/S-402.
8. PL# DENOTES BEAM BEARING PL. SEE SCHEDULE AND DETAILS.
9. FLOOR FRAMING AND COMPOSITE FLOOR DECK ARE DESIGNED TO REMAIN UNSHORED DURING CONCRETE PLACEMENT. ACCOUNT FOR AN EXPECTED DEFLECTION IN BEAMS AND GIRDERS OF UP TO 1/360 OF THE SPAN LENGTH (IN INCHES) OR 1", WHICHEVER IS LESS WHEN CALCULATING CONCRETE QUANTITIES. FINISH SUPPORTED SLABS FLAT AND LEVEL.
10. CONSTRUCTION JOINTS IN ELEVATED CONCRETE ON METAL DECK POURS SHALL BE SUBMITTED FOR REVIEW PRIOR TO CONSTRUCTION. SEE DETAIL 8/S-402.
11. FRAMED FLOOR OPENINGS COORD. LOCATION W/ MECH. CONTRACTOR. SEE DETAIL 7/S-405.

#### ROOF FRAMING PLAN NOTES:

1. # DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. L# DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. #H INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECH. SEE DETAIL 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCH. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJ REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY 'XX'. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. # INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN. IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. #B DENOTES KNEE BRACE. SEE DETAIL 10/S-406.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCH. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. PL# DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GO TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

#### 1 ROOF FRAMING PLAN - AREA E

SCALE: 1/8" = 1'-0"



#### CONSTRUCTION DOCUMENTS

## PAMLICO COUNTY PAMLICO 6-12 SCHOOL

601 Main Street, Bayboro, NC, 28515



No.	Date	Description
06/12/2024	06/12/2024	BID DOCUMENTS
ISSUE DATE:	06/12/2024	
PROJECT #:	23.08.034	
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#### ROOF FRAMING PLAN AREA E

S-103E



#### FLOOR FRAMING NOTES:

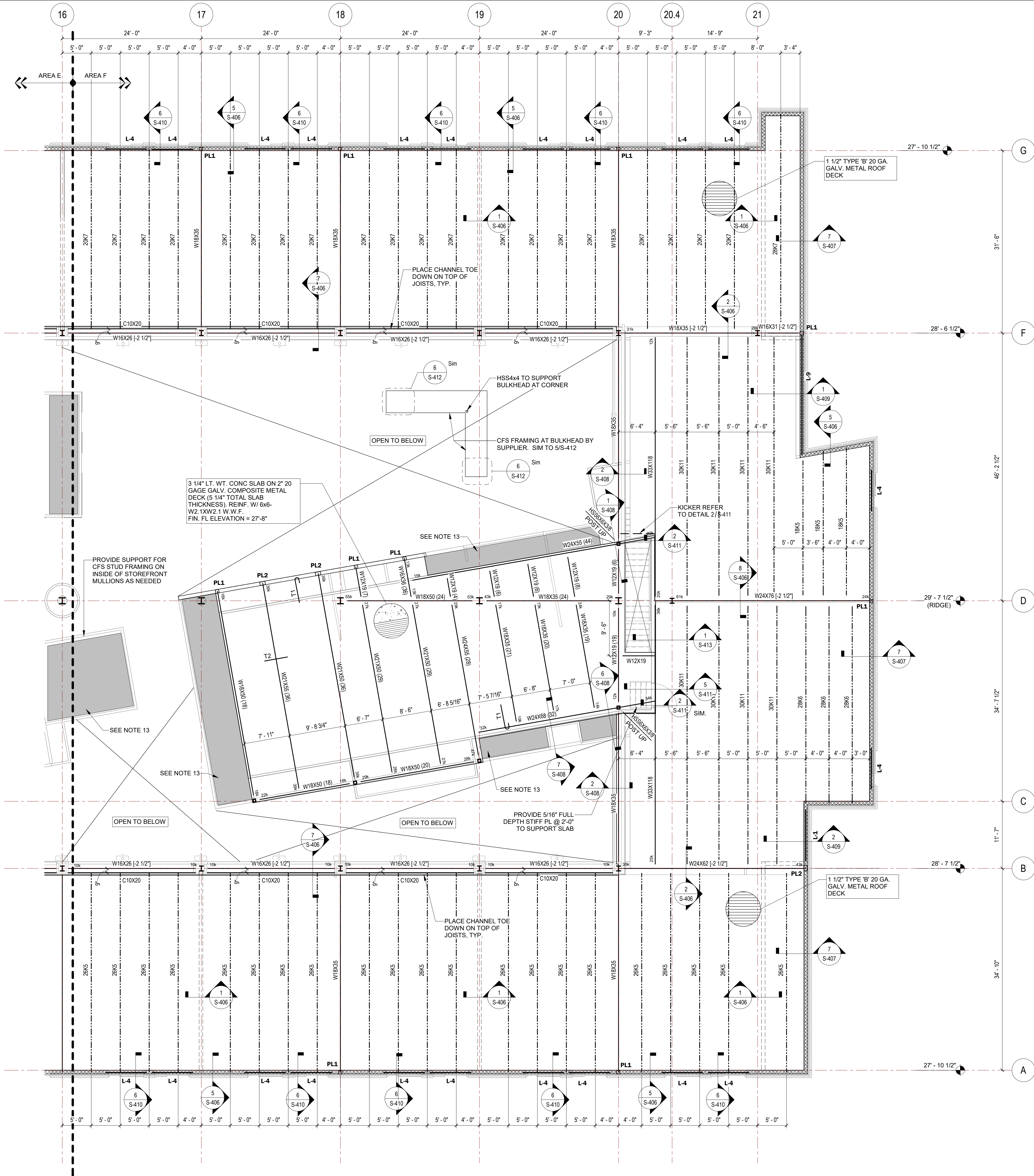
1. TOP OF STEEL ELEVATION = 5' 1/4" BELOW FIN. FLOOR ELEV. U.O.N. SEE PLAN FOR FINISHED FLOOR ELEVATION.
2. ALL ELEVATIONS REFERENCED FROM 0'-0" U.O.N.
3. NUMBERS IN PARENTHESIS DENOTES QUANTITY OF 3/4"Ø x 4" STUDS EQUALLY SPACED ON BEAM. SEE 3/S-405 FOR COMPOSITE BEAM LEGEND.
4. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
5. INDICATES MOMENT CONNECTION. SEE DETAIL. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "K-K". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
6. L# DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
7. T# DENOTES SLAB TOP BARS. SEE DETAIL 10/S-402 & 11/S-402.
8. PL# DENOTES BEAM BEARING PL. SEE SCHEDULE AND DETAILS.
9. FLOOR FRAMING AND COMPOSITE FLOOR DECK ARE DESIGNED TO REMAIN UNSHORED DURING CONCRETE PLACEMENT. ACCOUNT FOR AN EXPECTED DEFLECTION IN BEAMS AND GIRDERS OF UP TO 1/800 OF THE SPAN LENGTH (IN INCHES) OR 1", WHICHEVER IS LESS WHEN CALCULATING CONCRETE QUANTITIES. FINISH SUPPORTED SLABS FLAT AND LEVEL.
10. CONSTRUCTION JOINTS IN ELEVATED CONCRETE ON METAL DECK POURS SHALL BE SUBMITTED FOR REVIEW PRIOR TO CONSTRUCTION. SEE DETAIL 8/S-402.
11. FRAMED FLOOR OPENINGS COORD. LOCATION W/ MECHL. CONTRACTOR. SEE DETAIL 7/S-405.

#### ROOF FRAMING PLAN NOTES:

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. L# INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING, COORD. LOCATION W/ MECHL. SEE DETAIL 8/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECHL. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCHT. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY XXX. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "K-K". IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. K# DENOTES KNEE BRACE. SEE DETAIL 10/S-405.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCHT. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. PL# DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

#### ROOF FRAMING PLAN - AREA F

SCALE: 1/8" = 1'-0"





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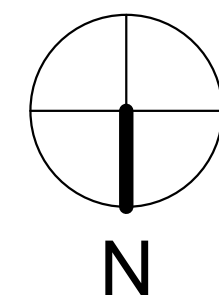
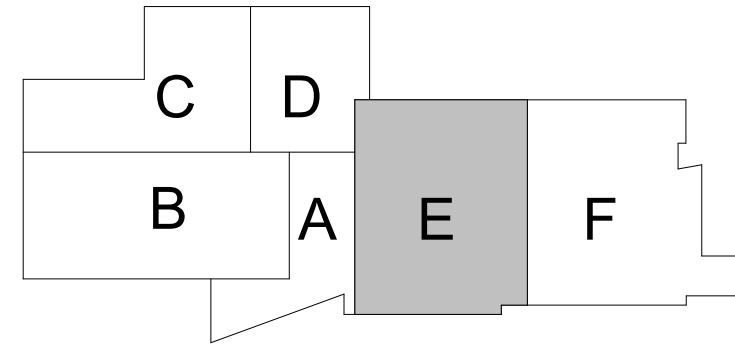
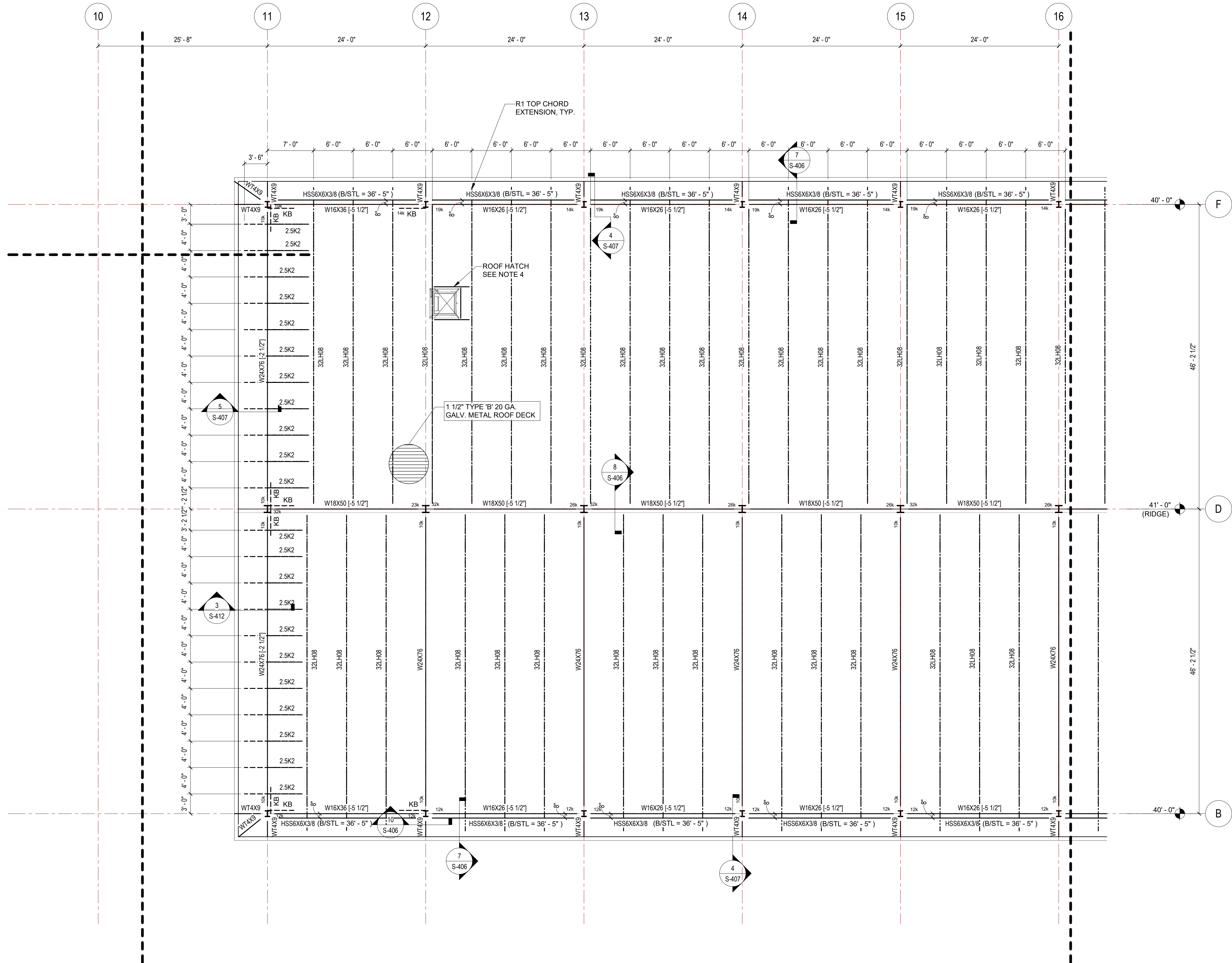
# ROOF FRAMING PLAN NOTES:

1. DENOTES DECK BEARING ELEVATION ABOVE FIN. FLOOR ELEVATION 0'-0".
2. DENOTES EXTERIOR/LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECH. SEE DETAIL. S/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCHT. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY VK. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL S/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN "K" IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. DENOTES KNEE BRACE. SEE DETAIL 10/S-406.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCHT. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-405 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

1  
S-104E

## HIGH ROOF FRAMING PLAN - AREA E

SCALE: 1/8" = 1'-0"



...Becoming the  
Leading Designer of  
High Performance Facilities  
In the Nation with a  
Specialty in Alternative  
Delivery Methods

333 Fayetteville St., Ste. 225  
Raleigh, NC 27601  
P: 919.573.6350  
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ARCHITECTS



## CONSTRUCTION DOCUMENTS

## PAMLICO COUNTY PAMLICO 6-12 SCHOOL 601 Main Street, Bayboro, NC, 28515



No.	Date	Description
06/12/2024	06/12/2024	BID DOCUMENTS
ISSUE DATE:	06/12/2024	
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CHECKED BY:	TH	

## HIGH ROOF FRAMING PLAN AREA E

S-104E

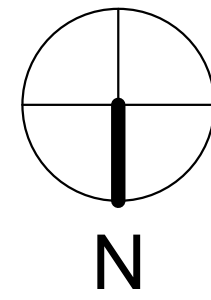
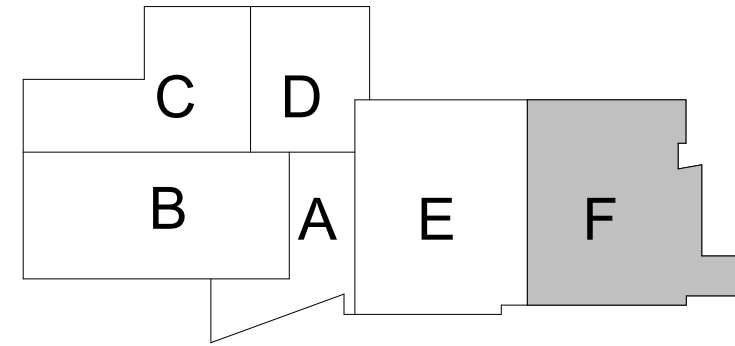
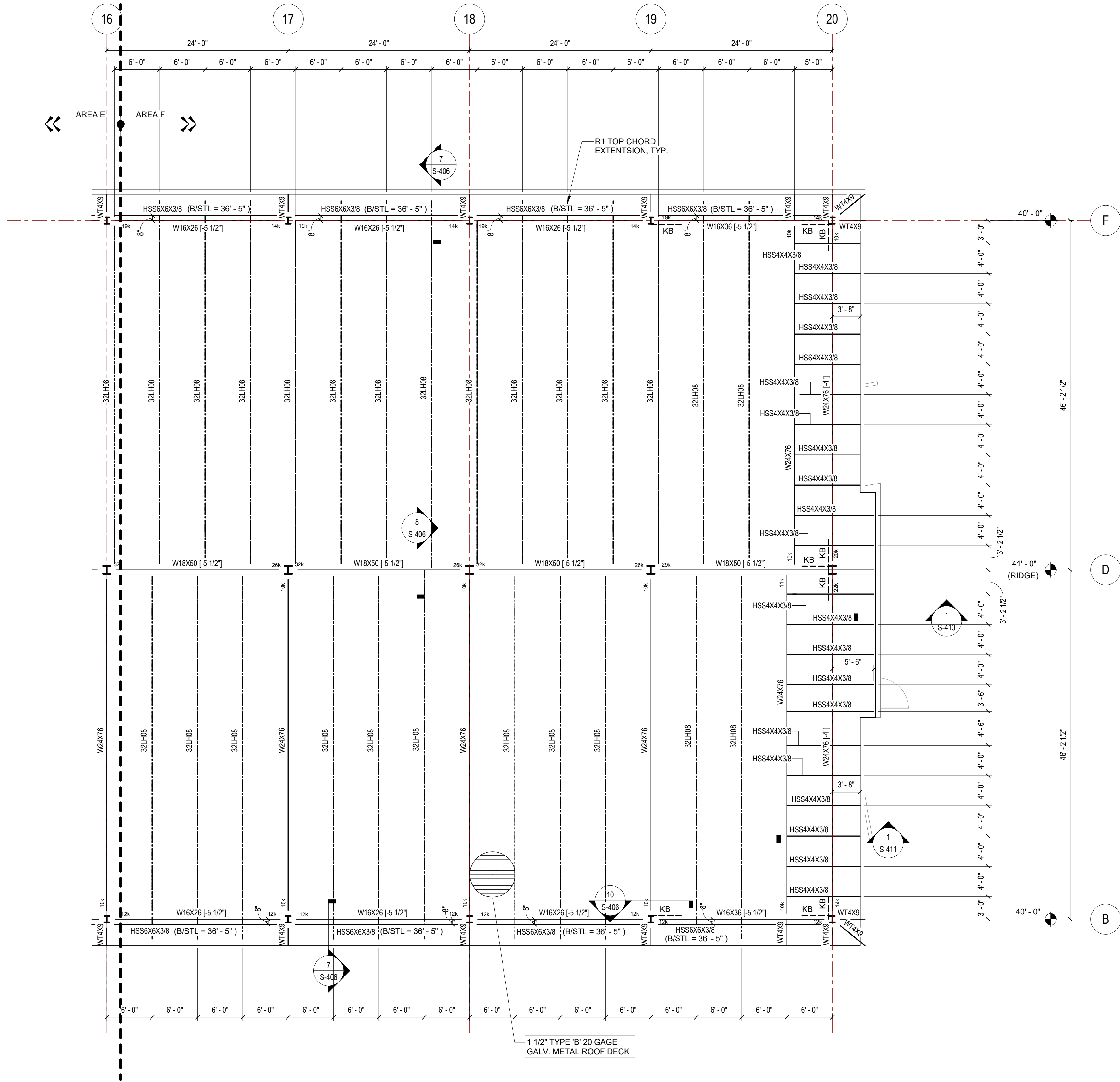


# ROOF FRAMING PLAN NOTES:

- 1.
2. L# DENOTES EXTERIOR LOAD BEARING LINTEL. SEE S-401 FOR SCHEDULE.
3. [-#] INDICATES TOP OF STEEL ELEVATION BELOW DECK BEARING.
4. FRAME ROOF OPENINGS (INCLUDING ROOF DRAINS) W/ ANGLE FRAMING. COORD. LOCATION W/ MECH. SEE DETAIL 2/S-401.
5. DO NOT HANG ANYTHING FROM THE ROOF DECK OR JOIST BRIDGING.
6. MECH. PIPING SHALL BE SUPPORTED AT JOIST PANEL POINTS OR ADDITIONAL BRACING SHALL BE PROVIDED. SEE DETAIL.
7. UNLESS OTHERWISE NOTED ON PLAN, REFER TO DETAILS ON S-401 FOR BRACING OF NON-LOAD BEARING CMU WALLS EXTENDING TO DECK. INTERIOR NON-LOAD BEARING WALLS NOT EXTENDING TO DECK SHALL BE BRACED BY INTERSECTING WALL OR ANGLE BRACING TO JOIST AT 12'-0" O.C. MAX. COORDINATE WITH ARCH. FOR THE PARTITION WALLS EXTENDING TO THE DECK.
8. JOIST BRIDGING NOT SHOWN FOR CLARITY. JOIST MANUFACTURERS SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE W/ SJI REQUIREMENTS.
9. COORDINATE ROOF OPENING SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR. SEE DETAIL.
10. BEAM VERTICAL SHEAR DENOTED BY VOK. BEAM CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM OF 10K VERTICAL SHEAR U.O.N. ON PLAN. NOTED REACTIONS ARE SERVICE/UNFACTORED LOADS.
11. INDICATES MOMENT CONNECTION. SEE DETAIL 5/S-405. FABRICATOR TO DESIGN AND DETAIL CONNECTION FOR THE MOMENT INDICATED ON THE PLAN W/ K-R. IF NO MOMENT IS PROVIDED, THE CONNECTION SHALL BE DESIGNED FOR THE FULL MOMENT CAPACITY OF THE BEAM.
12. "KB" DENOTES KNEE BRACE. SEE DETAIL 10/S-408.
13. HATCHED AREA INDICATES CFS JOISTS @ 2'-0" TOPPED WITH 3/4" PLYWOOD TO SUPPORT CEILING. COLD FORMED STEEL STUD CEILING JOISTS, AS WELL AS ANY REQUIRED GIRDERS SHALL BE DESIGNED FOR 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE W/ ARCH. DRAWINGS.
14. BASKETBALL GOALS SHALL BE HUNG FROM JOISTS AT PANEL POINTS. JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 2,500 LB AT ANY LOCATION ALONG THE JOIST. ANY SUPPLEMENTAL FRAMING REQUIRED SHALL BE PROVIDED BY BASKETBALL GOAL SUPPLIER.
15. PL# DENOTES BEAM BEARING PL. SEE DETAIL 2/S-401.
16. DASHED OUTLINE INDICATES LOCATION OF MECHANICAL EQUIPMENT AND ACCESS. REFER TO MECH. DWGS FOR FINAL EQUIPMENT LOCATIONS. CFS JOISTS SHALL BE DESIGNED FOR THE EQUIPMENT WEIGHT IN ADDITION TO 10 PSF DEAD LOAD AND 40 PSF LIVE LOAD. COORDINATE EQUIPMENT WEIGHT WITH MECHANICAL CONTRACTOR.
17. REFER TO MEP DRAWINGS FOR LOCATIONS OF DESTRATIFICATION FANS. REFER TO DETAIL 10/S-406 FOR SUPPLEMENTAL STEEL SUPPORT FOR FANS. GC TO COORDINATE FINAL FAN SIZE AND LOCATIONS WITH MECHANICAL CONTRACTOR AND STEEL FABRICATOR.

## 1 HIGH ROOF FRAMING PLAN - AREA F

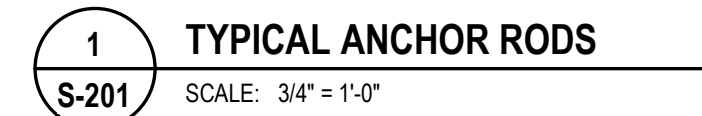
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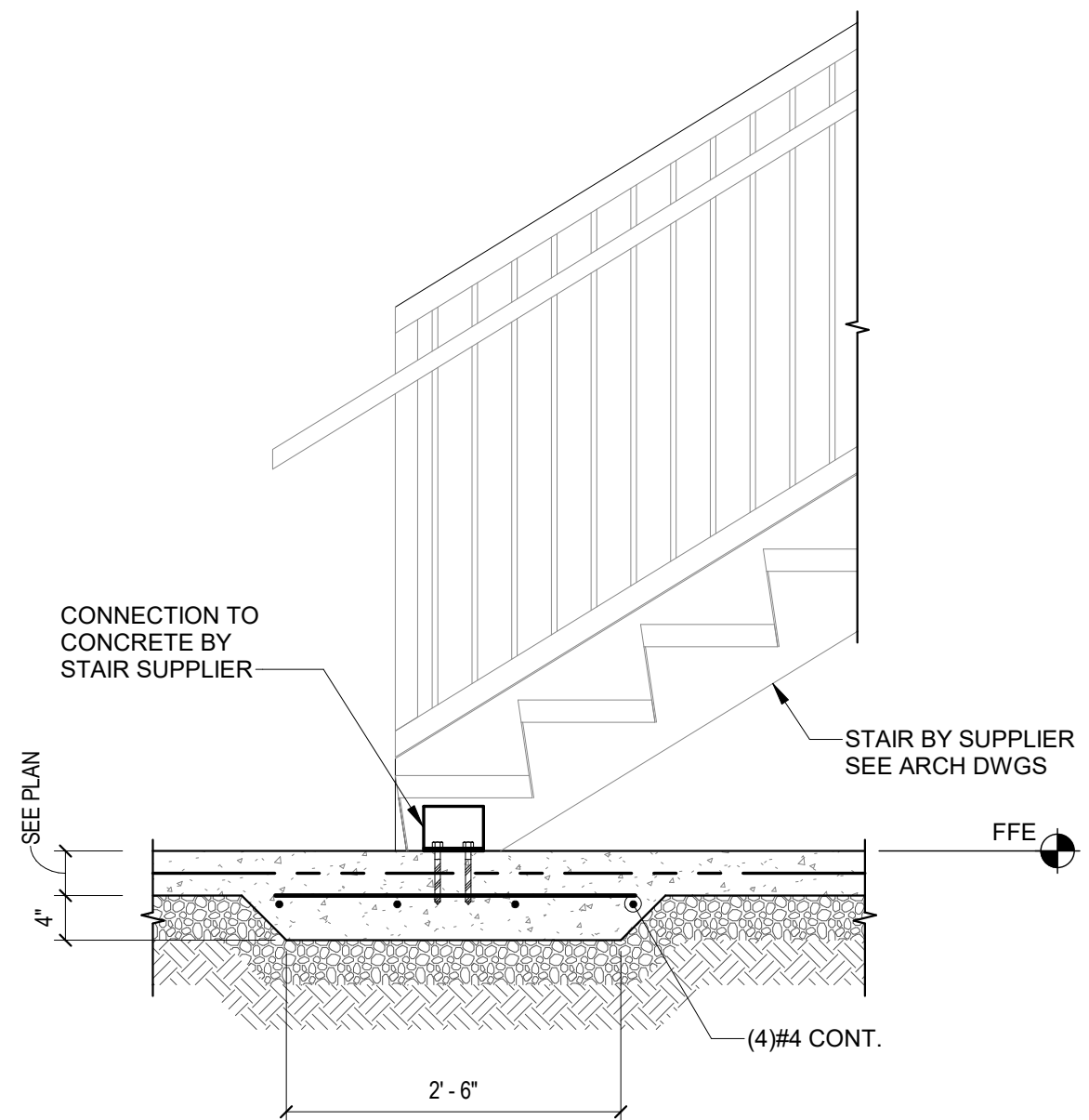




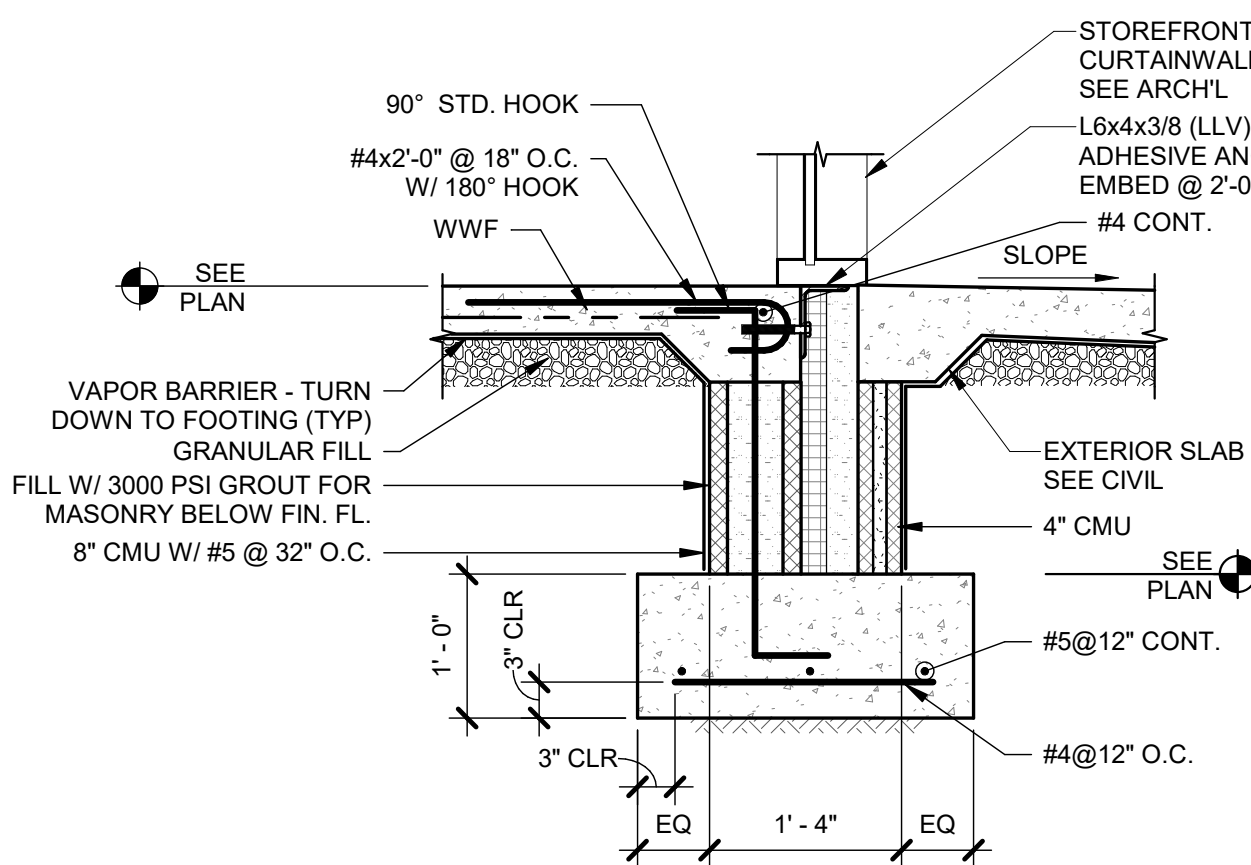
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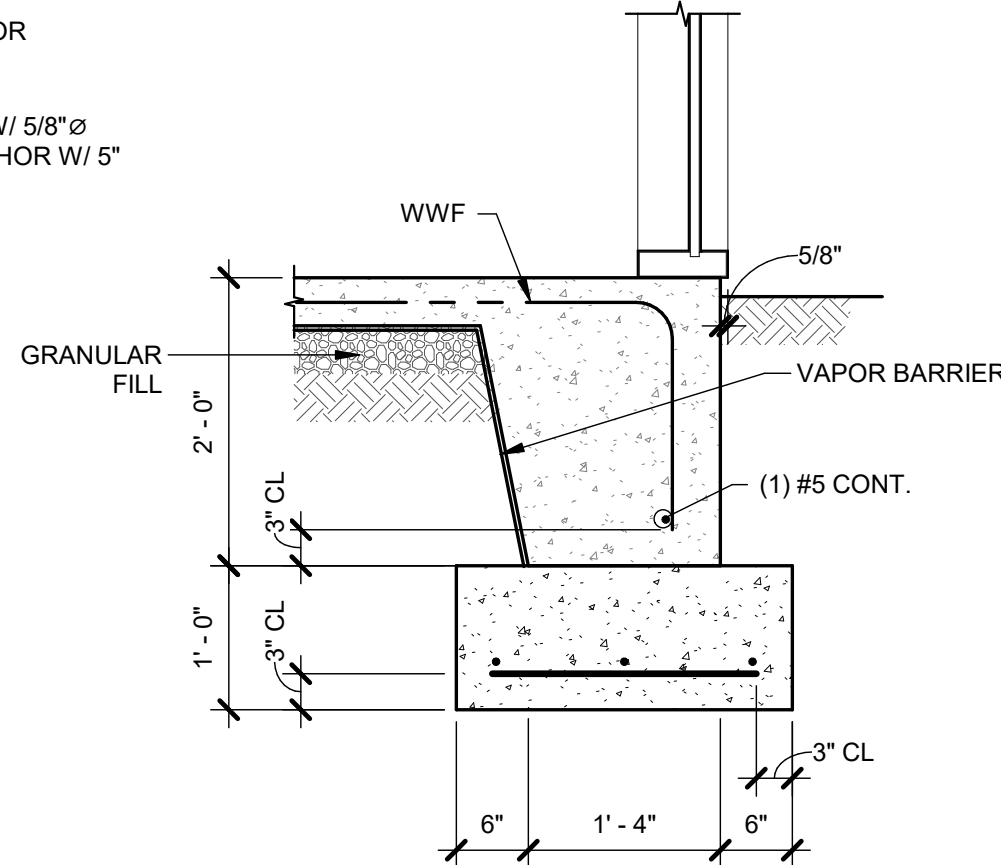




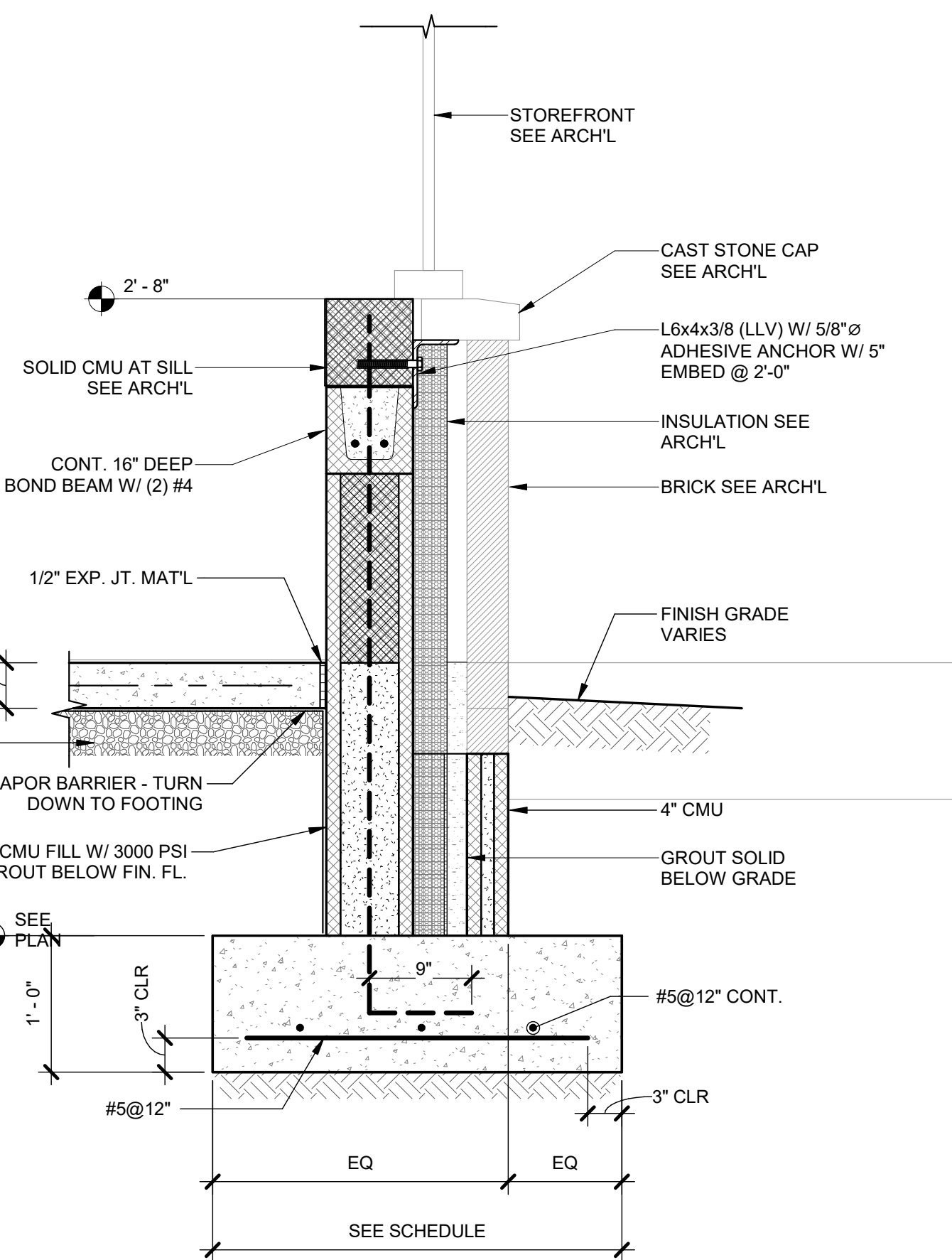
15 TYPICAL STAIR STRINGER @ THICKENED SLAB  
S-301 SCALE: 3/4" = 1'-0"



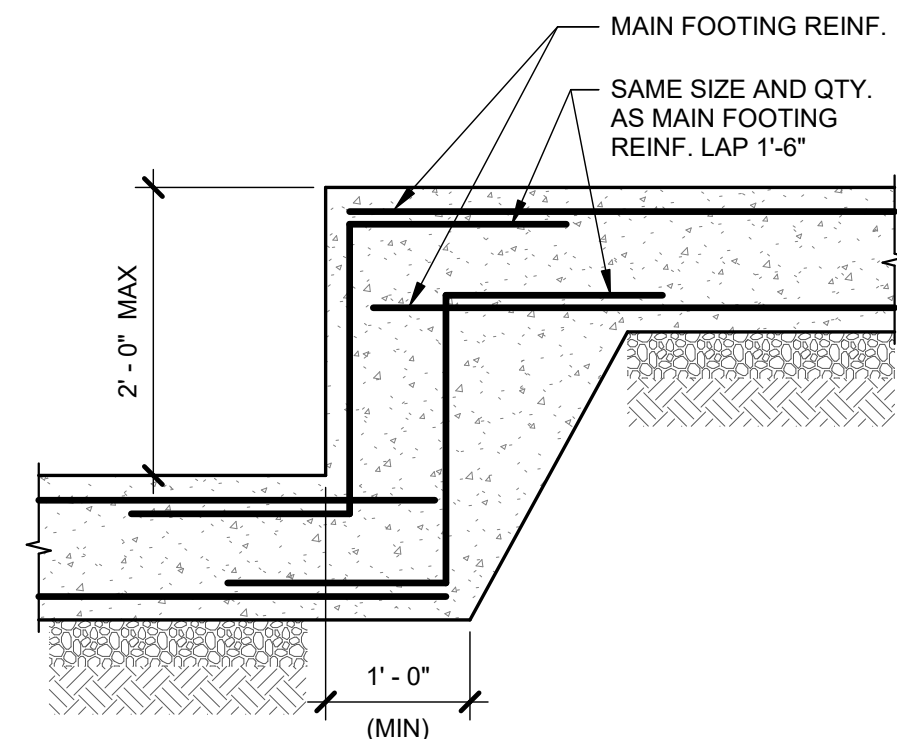
14 TYPICAL CURTAIN WALL  
S-301 SCALE: 3/4" = 1'-0"



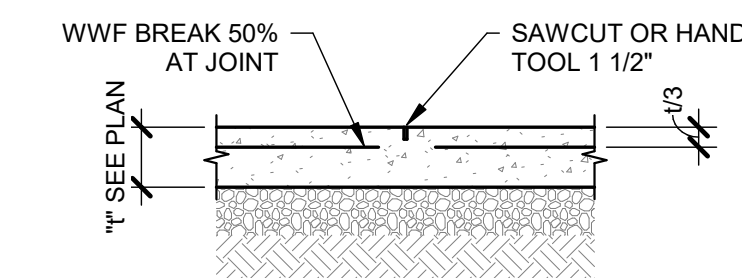
13 TYPICAL LUG FOOTING  
S-301 SCALE: 3/4" = 1'-0"



16 SECTION THRU STOREFRONT  
S-301 SCALE: 1" = 1'-0"

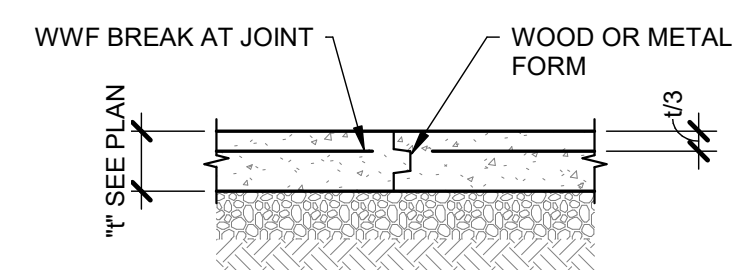


12 STEPPED FOOTING DETAIL  
S-301 SCALE: 3/4" = 1'-0"



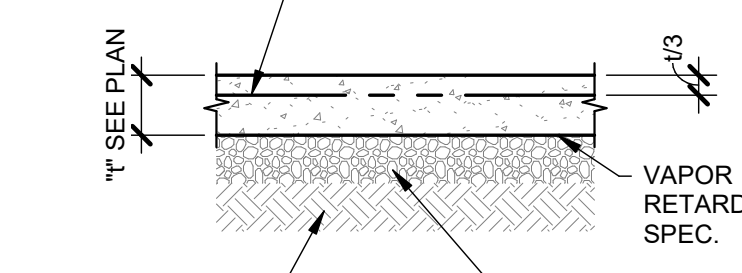
NOTE:  
IN LIEU OF SAWCUT OR HAND TOOL  
CONTRACTOR MAY USE FULL-DEPTH  
PREFORMED JOINT.

CONTROL JOINT

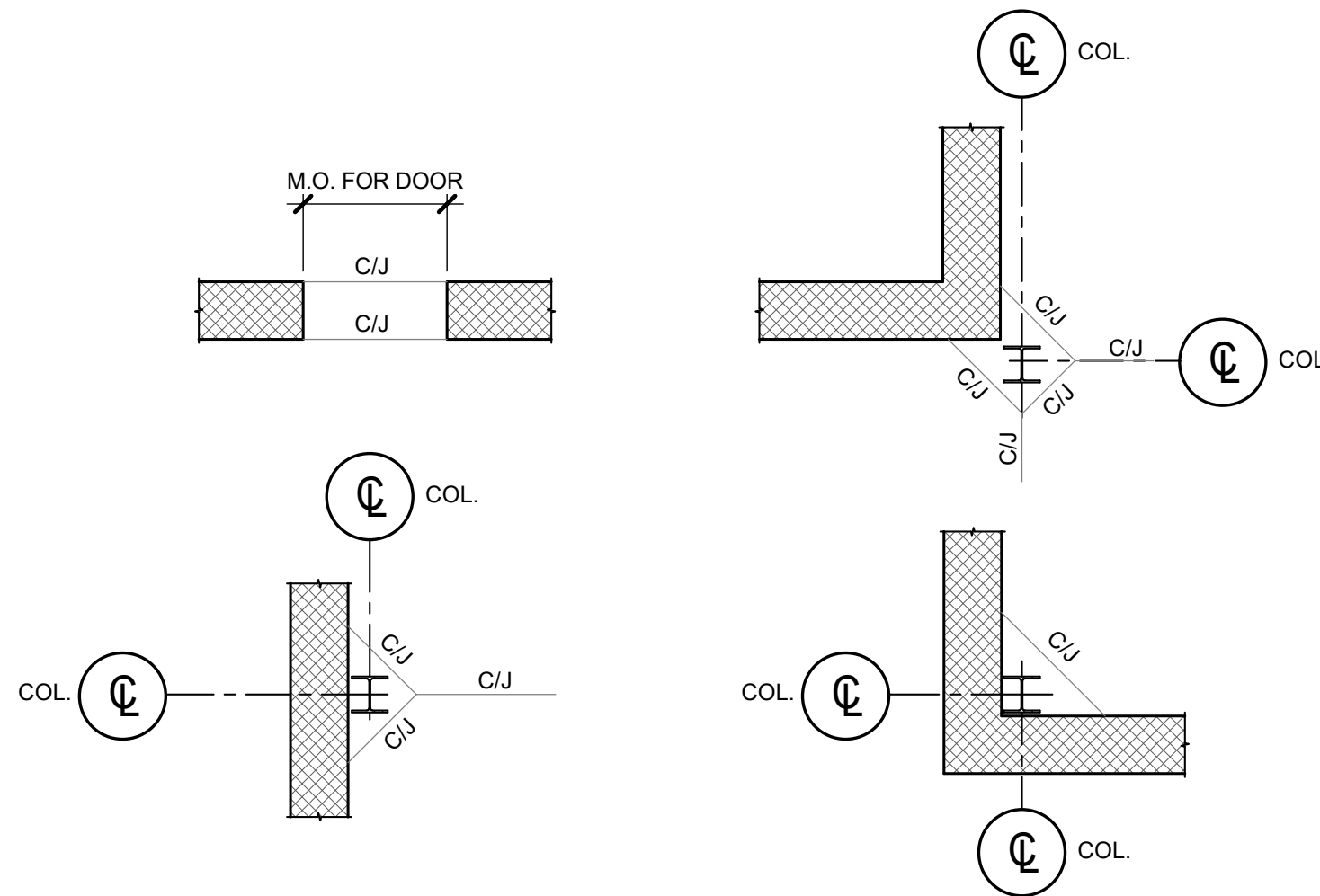


NOTE:  
IN LIEU OF FORMING KEY WAY CONTRACTOR MAY USE  
1/2"x2'-0" LONG SMOOTH DOWELS WITH ONE END  
GREASED. PLACE AT 24" O.C. SPEED DOWELS OR  
DIAMONDS WILL BE CONSIDERED BUT MUST BE  
SUBMITTED FOR APPROVAL.

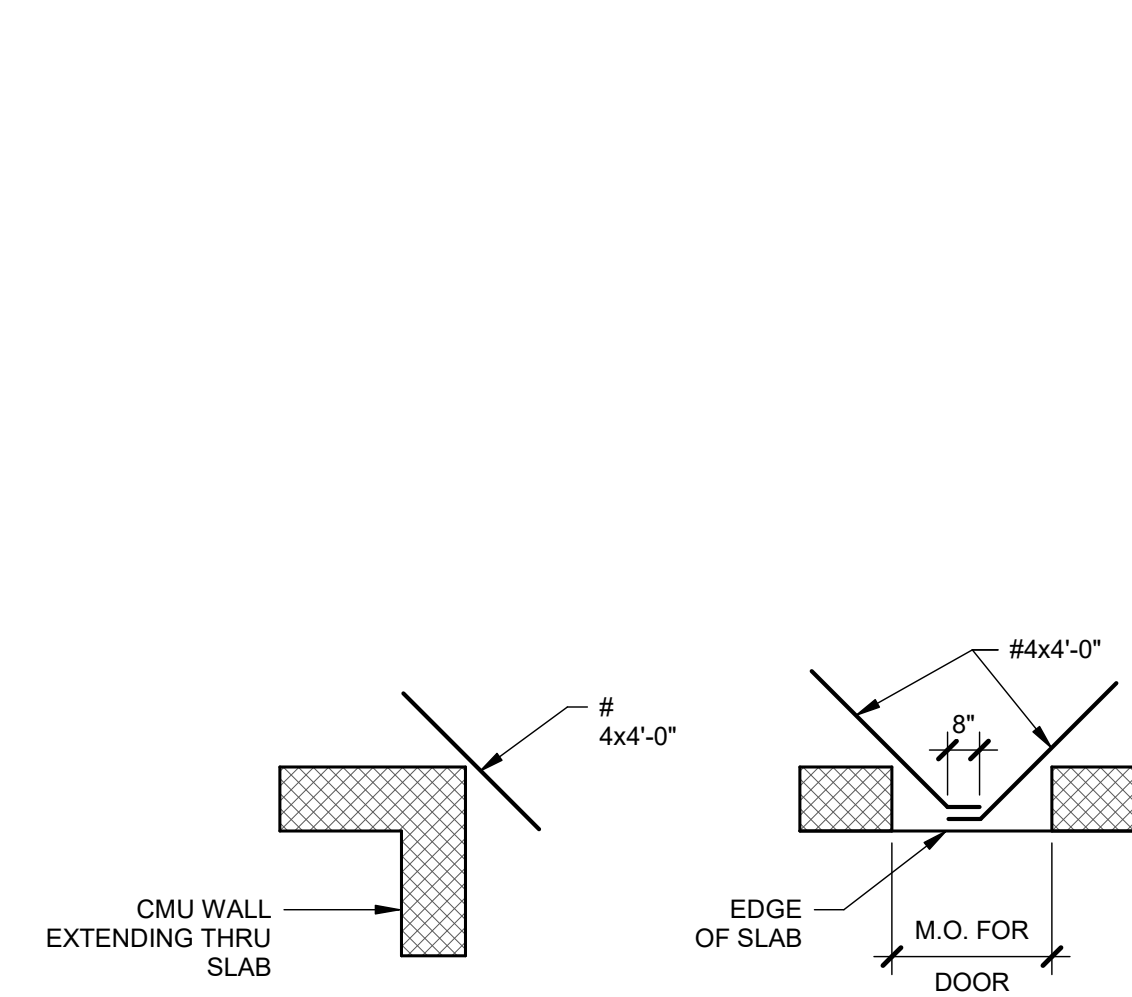
CONSTRUCTION JOINT



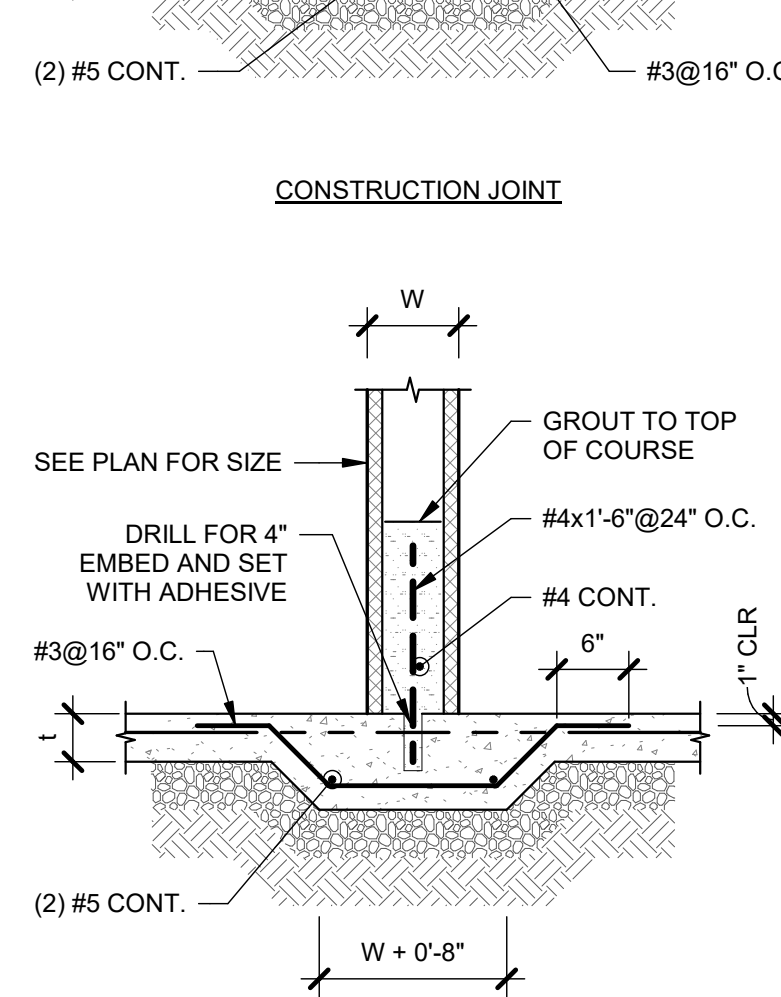
11 TYPICAL SLAB ON GRADE DETAIL  
S-301 SCALE: 3/4" = 1'-0"



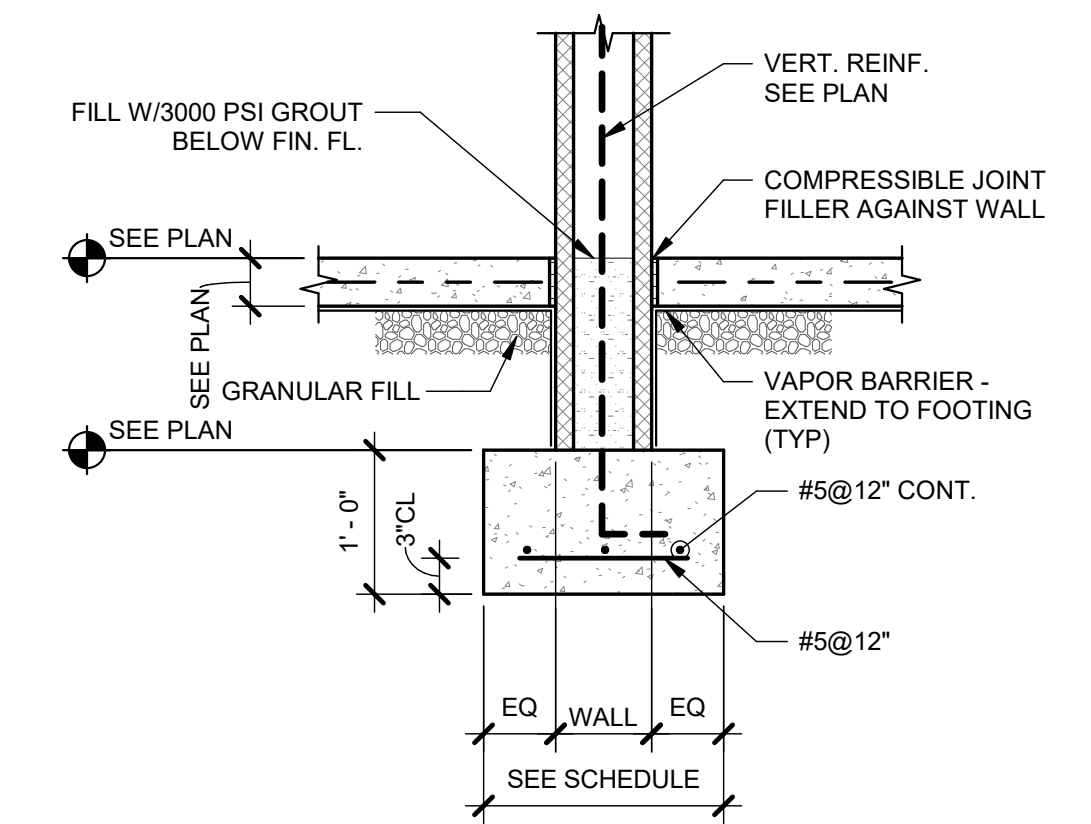
10 CONTROL JOINT DETAILS  
S-301 SCALE: 1/4" = 1'-0"



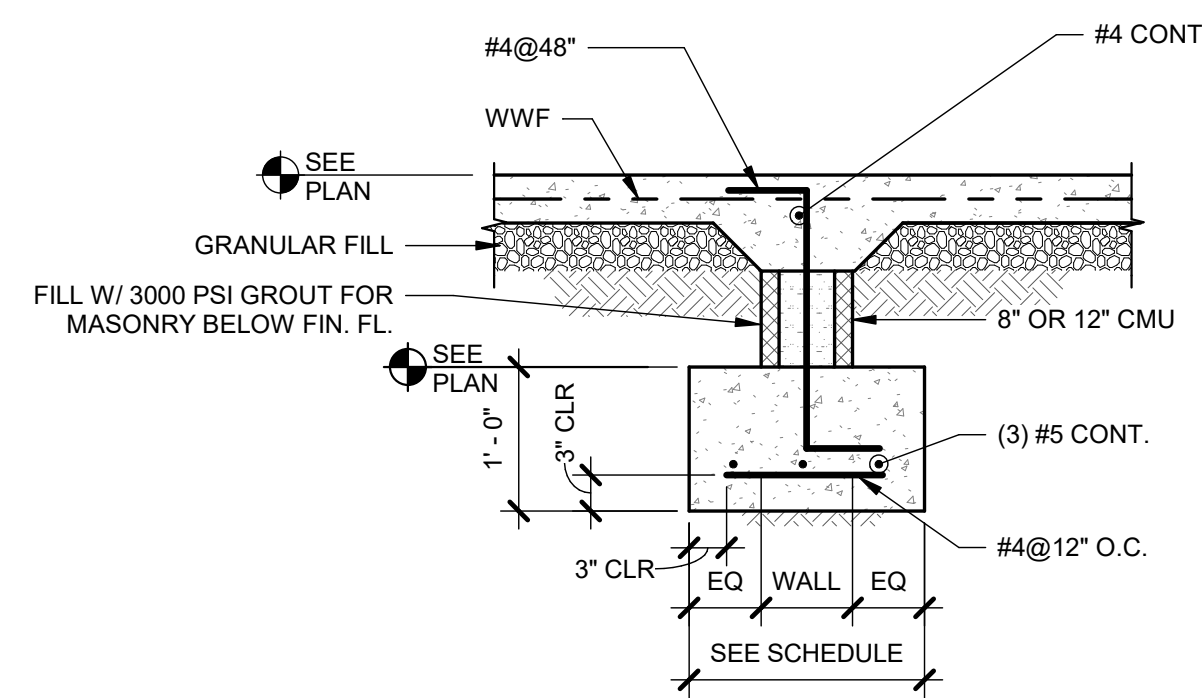
9 SLAB PLAN @ RE-ENTRANT CORNER  
S-301 SCALE: 1/4" = 1'-0"



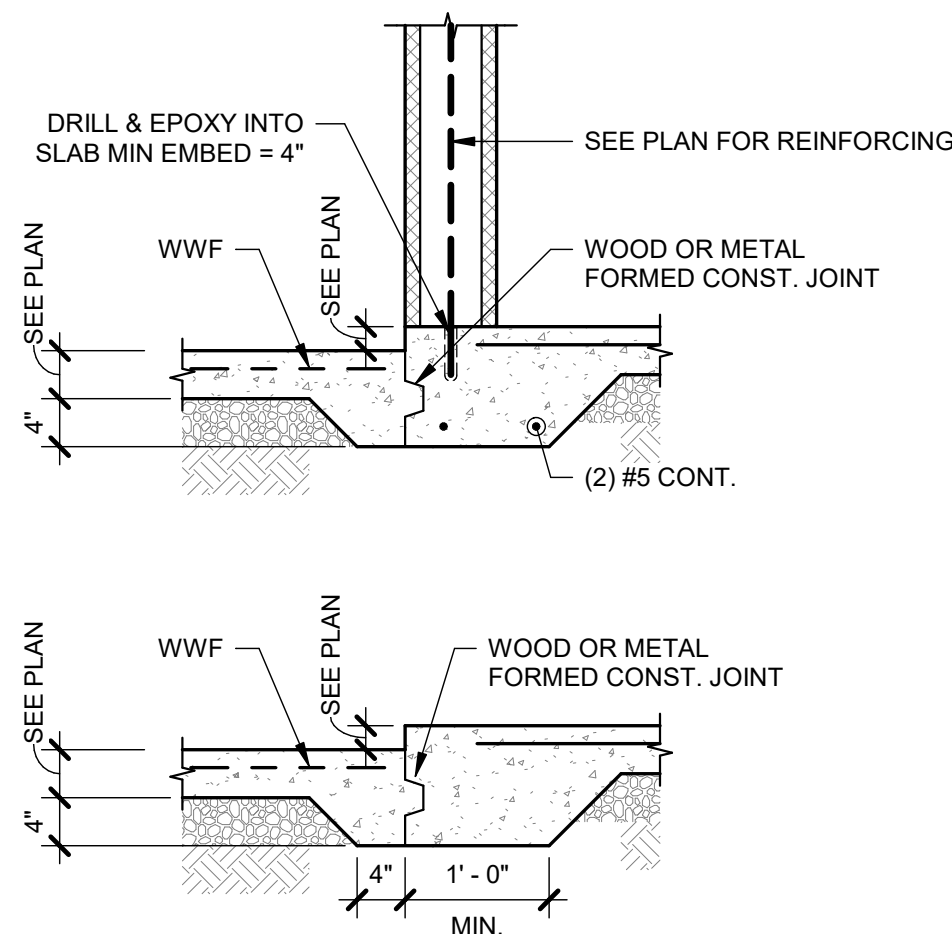
8 TYPICAL THICKENED SLAB DETAIL  
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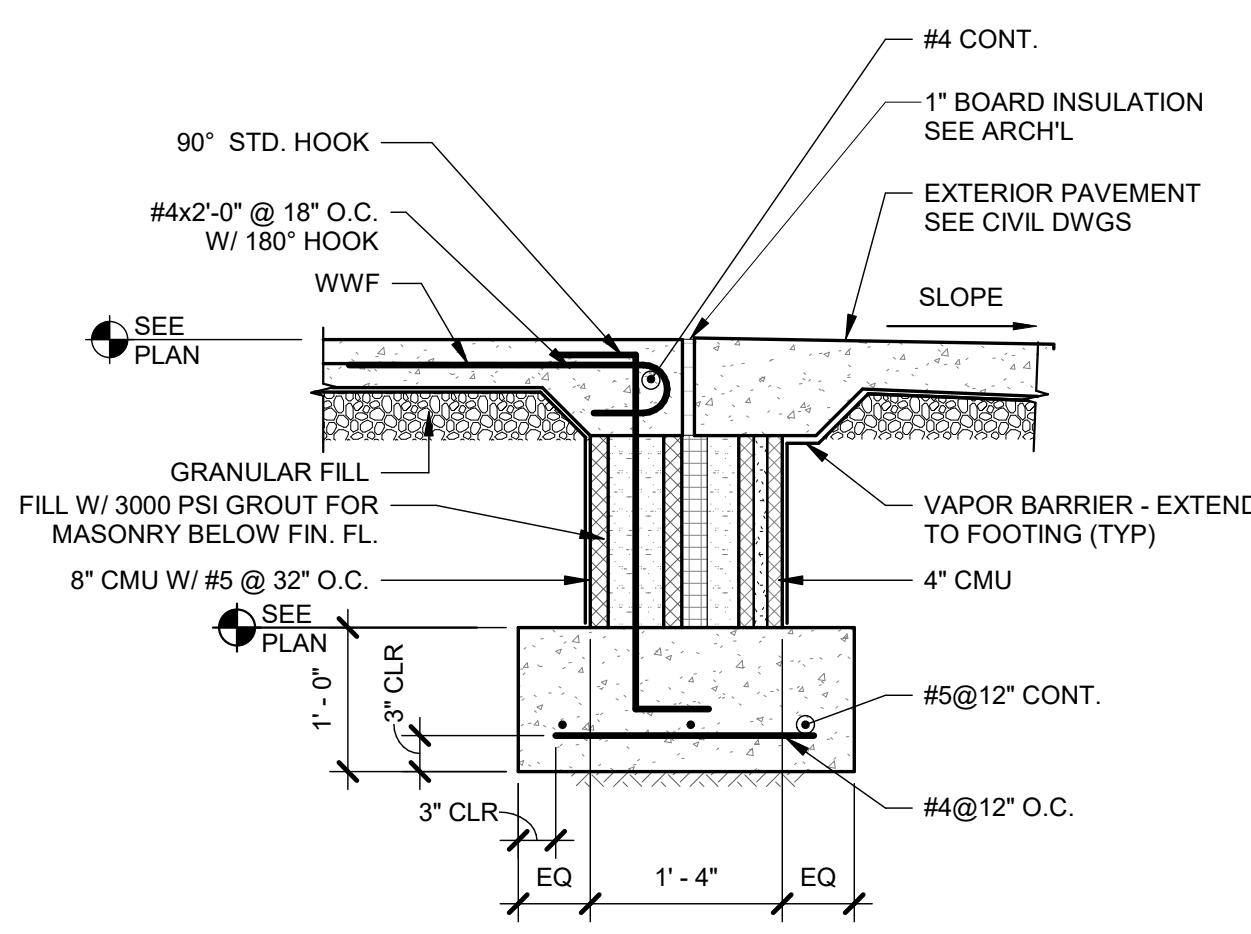
7 INTERIOR WALL FOOTING DETAIL  
S-301 SCALE: 3/4" = 1'-0"



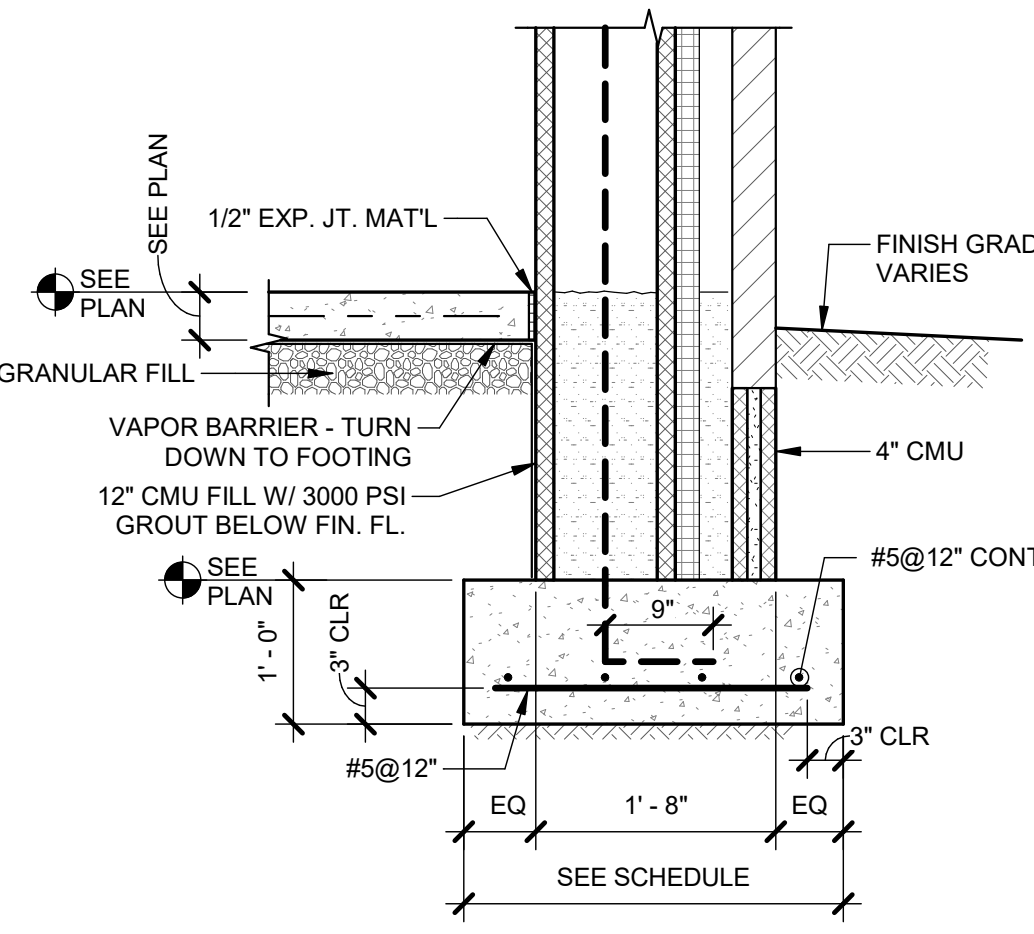
6 TYPICAL INTERIOR DOOR OPENING  
S-301 SCALE: 3/4" = 1'-0"



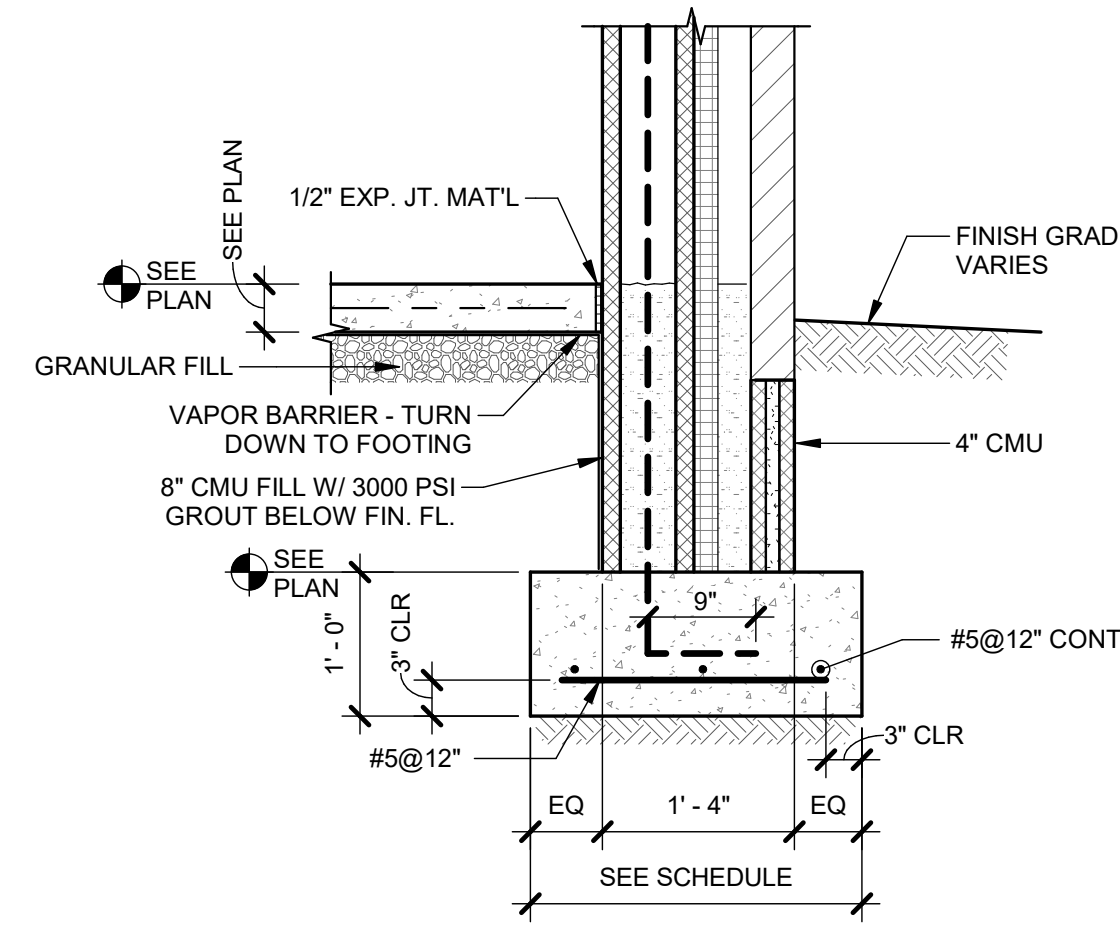
5 TYPICAL DEPRESSED SLAB  
S-301 SCALE: 3/4" = 1'-0"



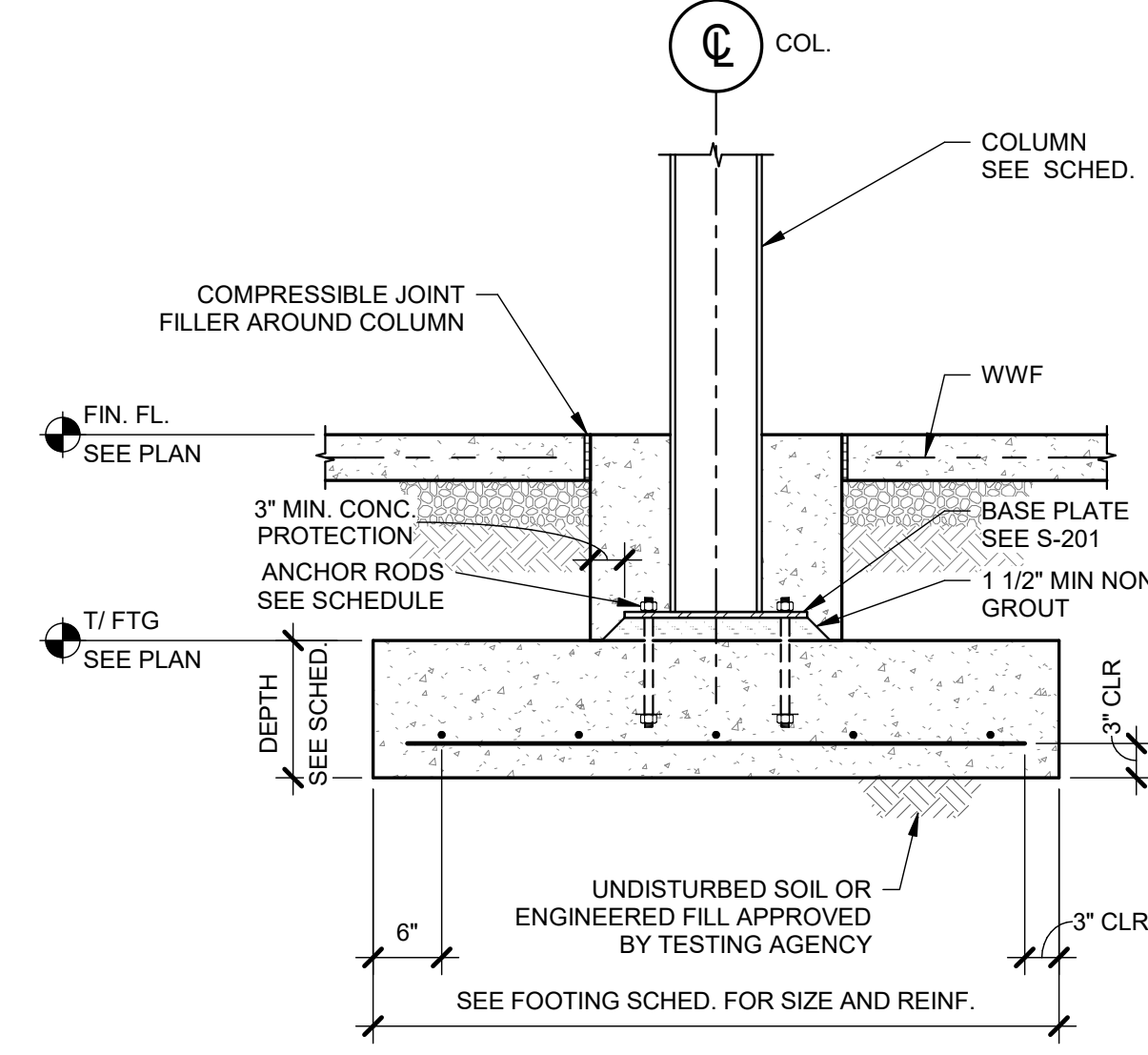
4 TYPICAL EGRESS DOOR & OVERHEAD DOOR OPENING  
S-301 SCALE: 3/4" = 1'-0"



3 12" CMU EXTERIOR WALL FOOTING DETAIL  
S-301 SCALE: 3/4" = 1'-0"



2 8" CMU EXTERIOR WALL FOOTING DETAIL  
S-301 SCALE: 3/4" = 1'-0"



1 STEEL COLUMN FOOTING DETAIL  
S-301 SCALE: 3/4" = 1'-0"

CONSTRUCTION  
DOCUMENTS

PAMLICO COUNTY  
PAMLICO 6-12 SCHOOL  
601 Main Street, Bayboro, NC, 28515

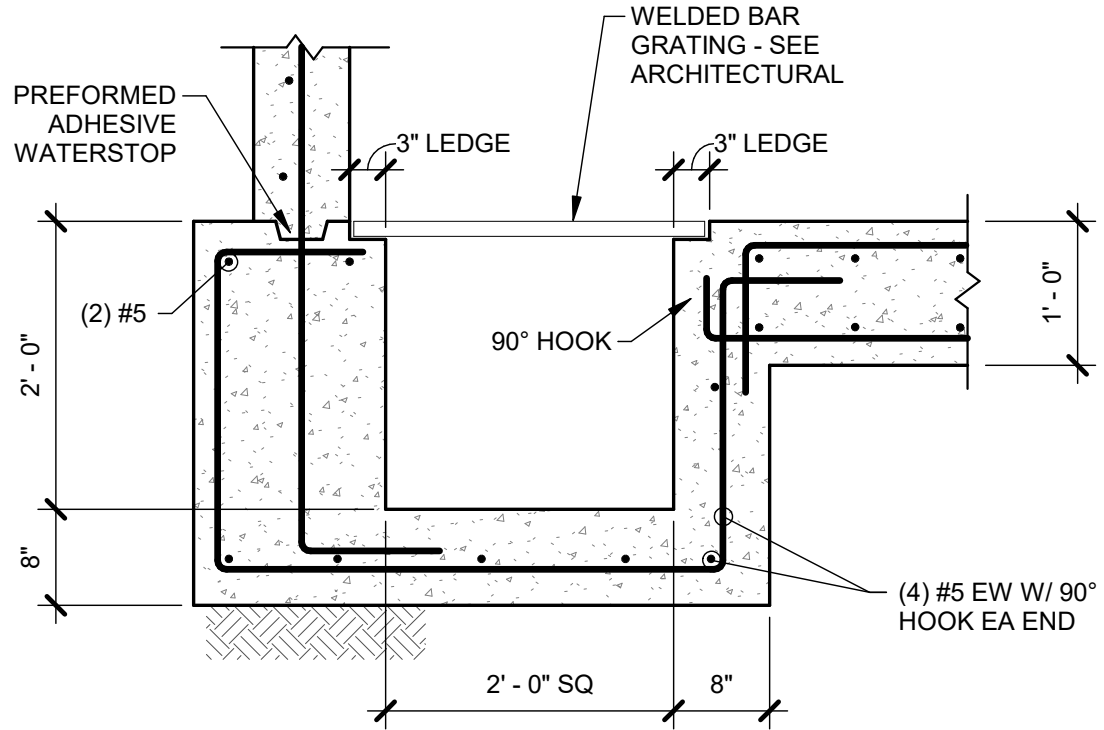


No.	Date	BID DOCUMENTS
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2	23.08.034	PROJECT #:
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4	TH	CHECKED BY:

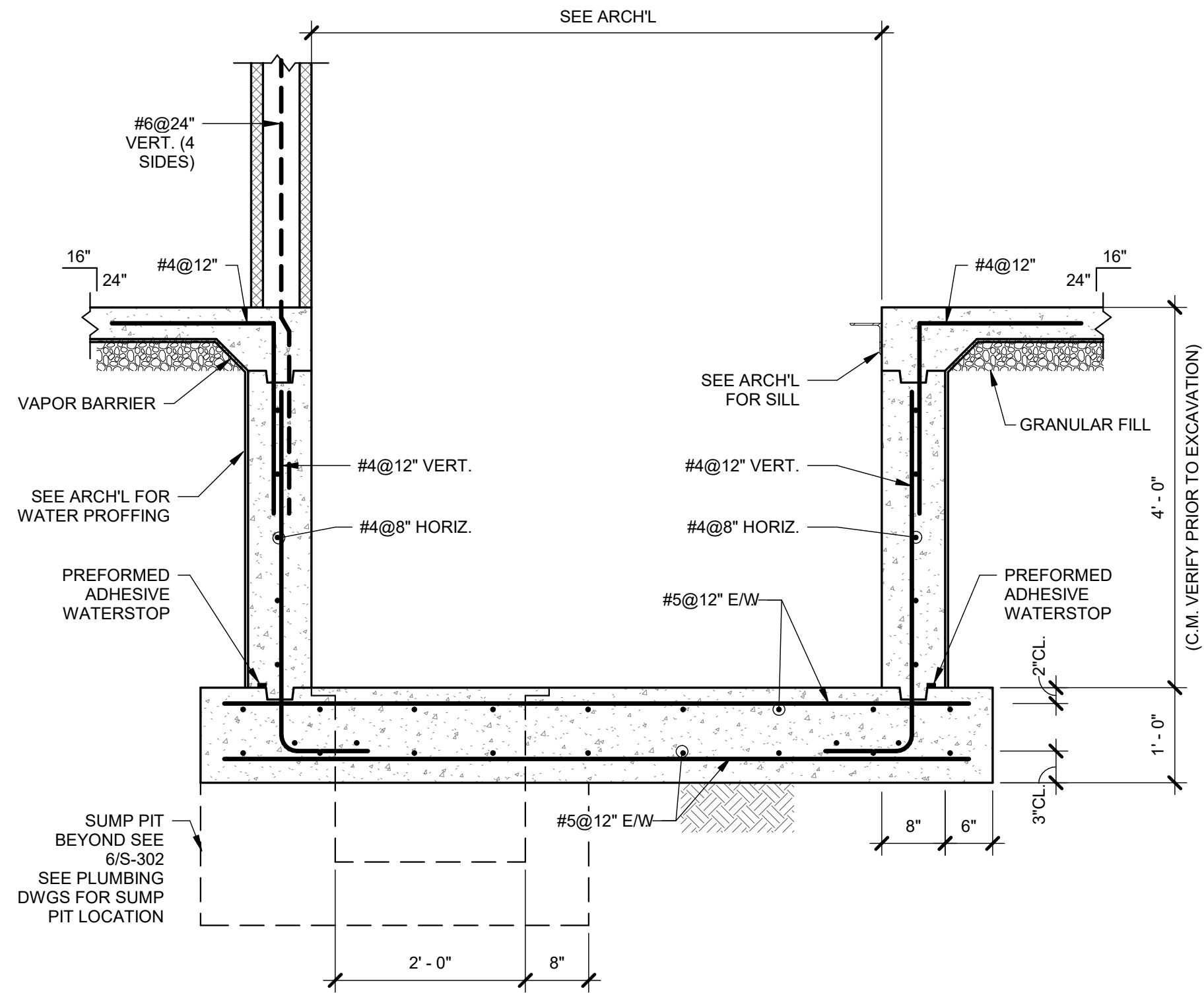
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DETAILS



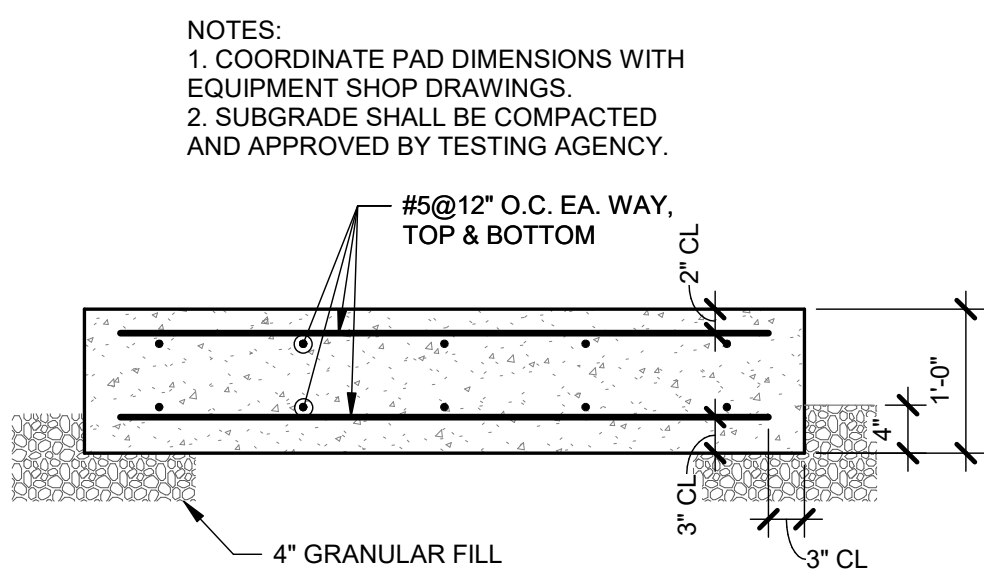
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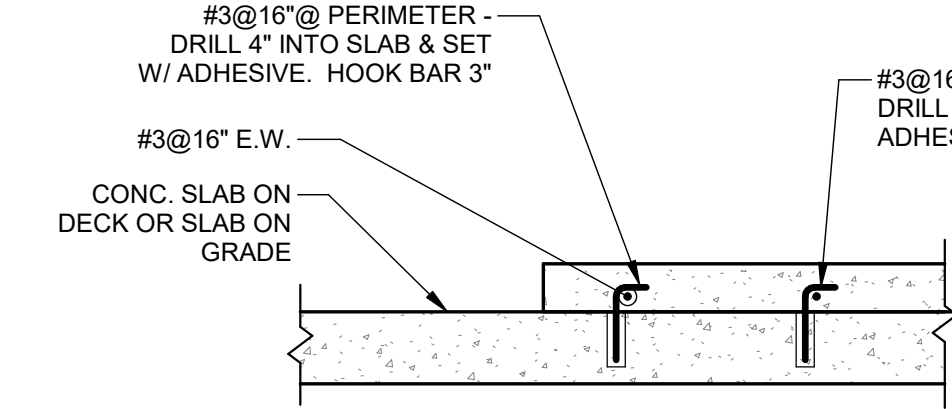
5 SUMP PIT DETAIL  
S-302 SCALE: 3/4" = 1'-0"



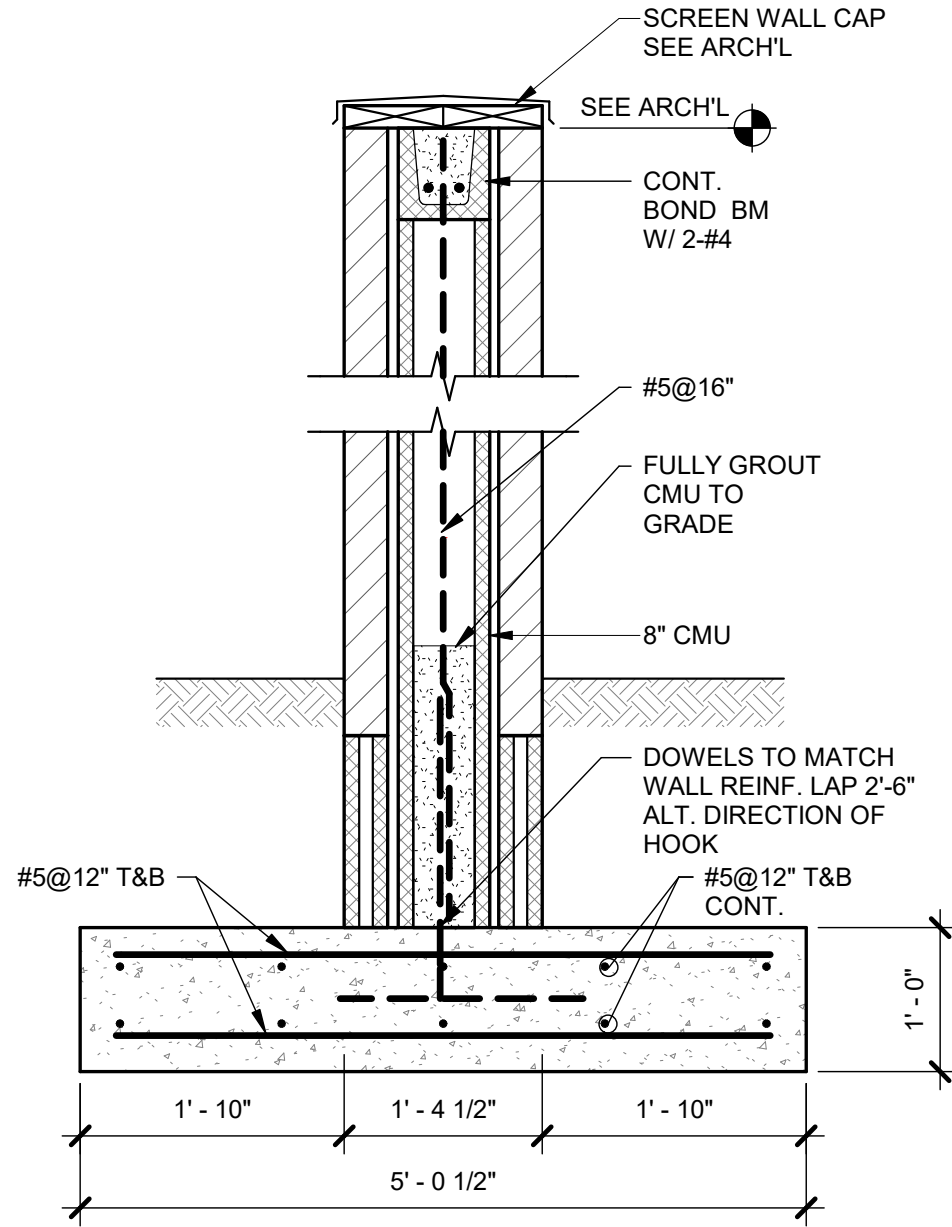
4 SECTION THRU ELEVATOR PIT  
S-302 SCALE: 3/4" = 1'-0"



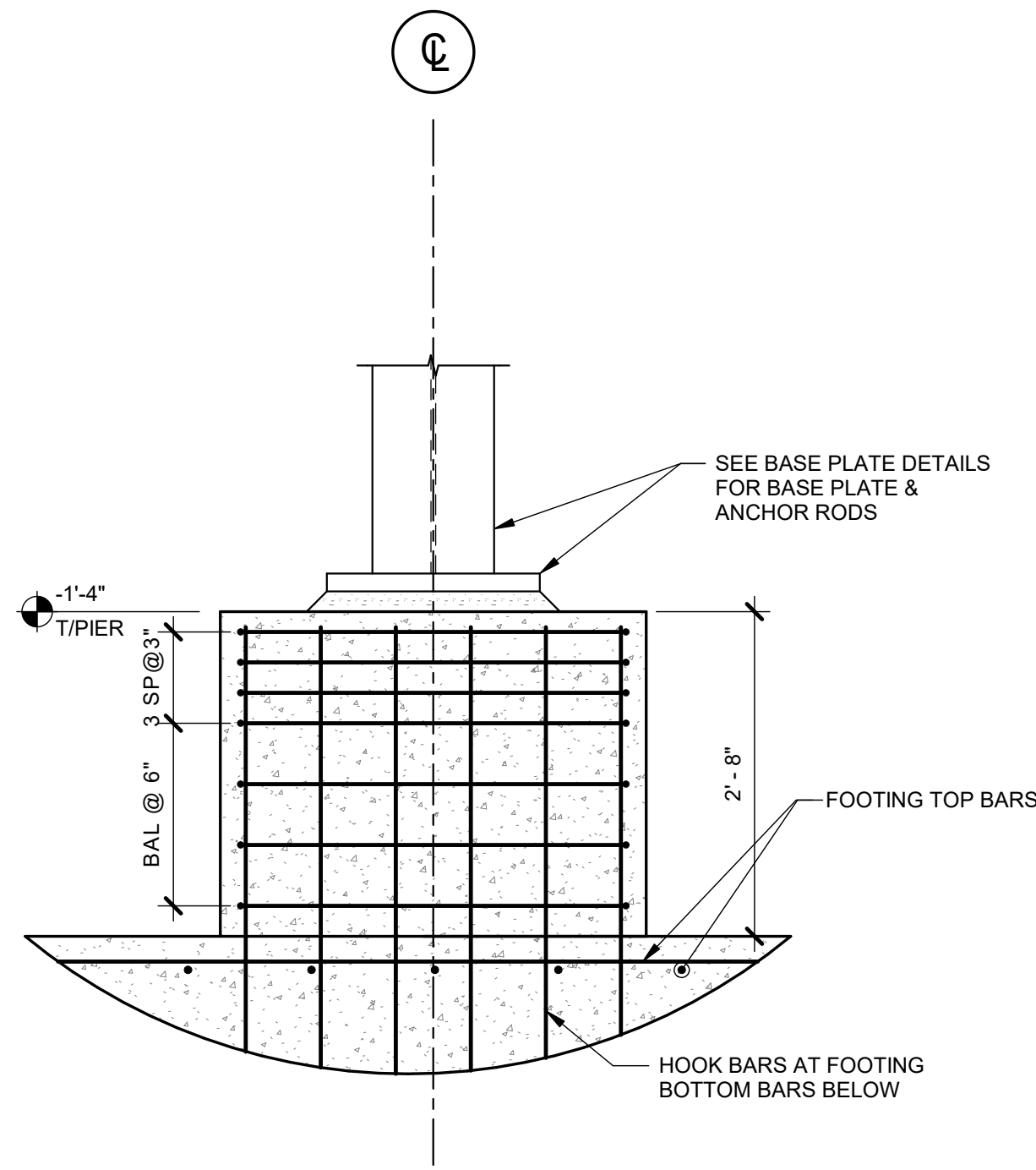
3 EXTERIOR EQUIPMENT PAD  
S-302 SCALE: 3/4" = 1'-0"



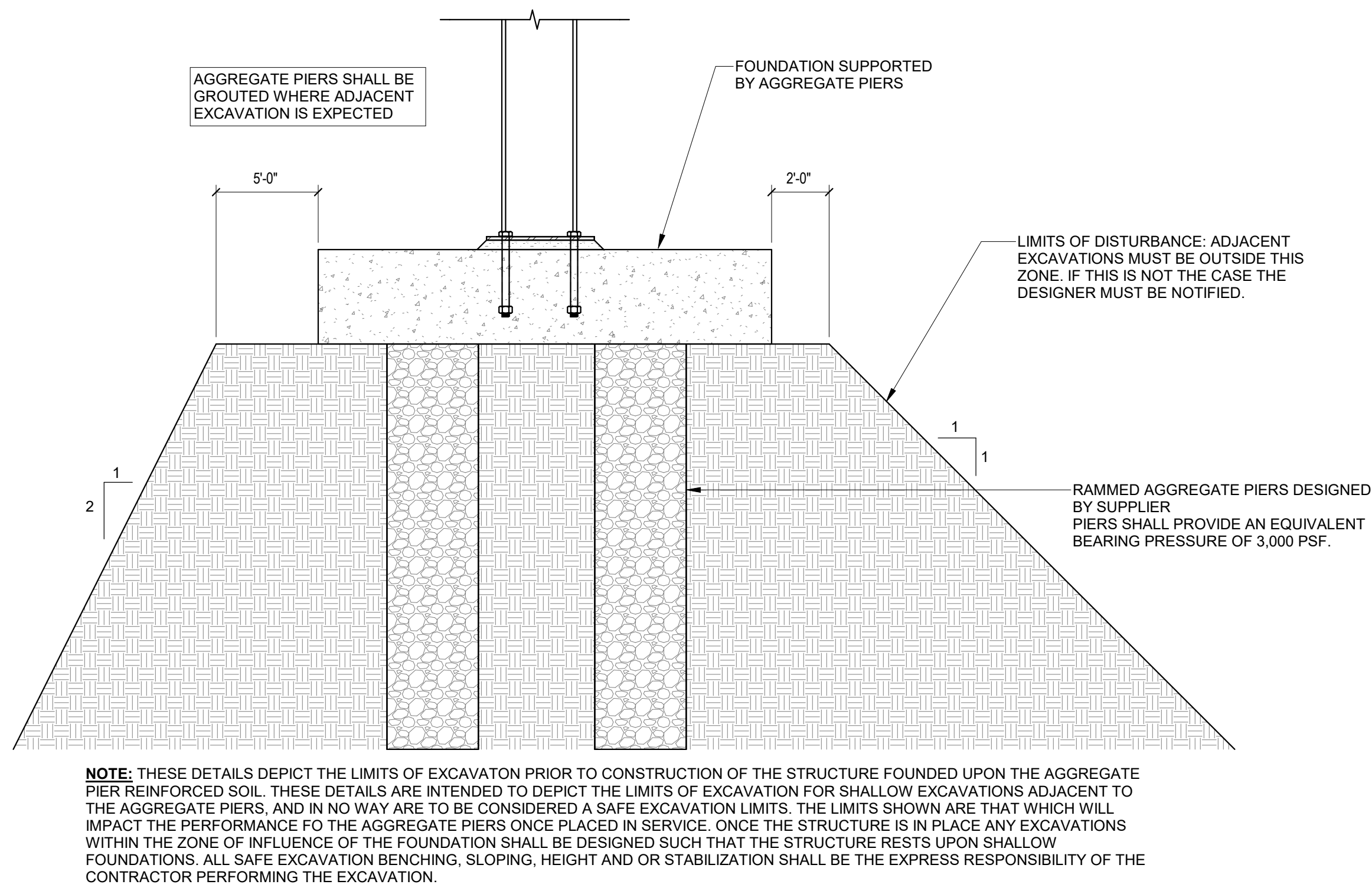
2 INTERIOR EQUIPMENT PAD  
S-302 SCALE: 3/4" = 1'-0"



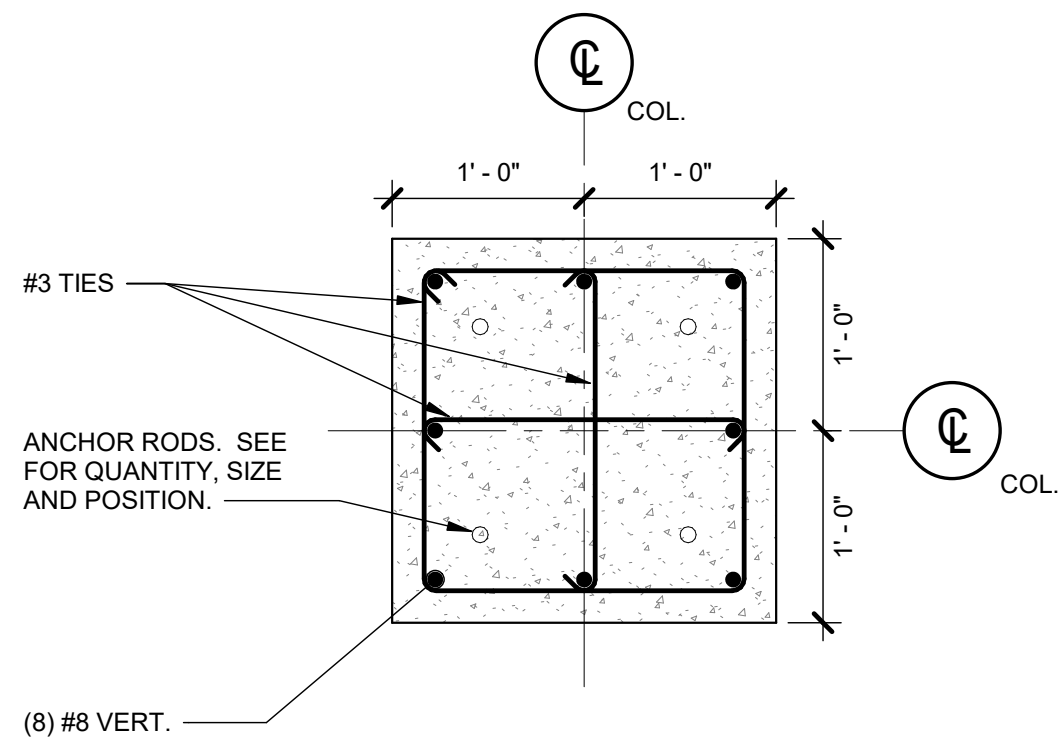
1 8" CMU SCREEN WALL DETAIL  
S-302 SCALE: 3/4" = 1'-0"



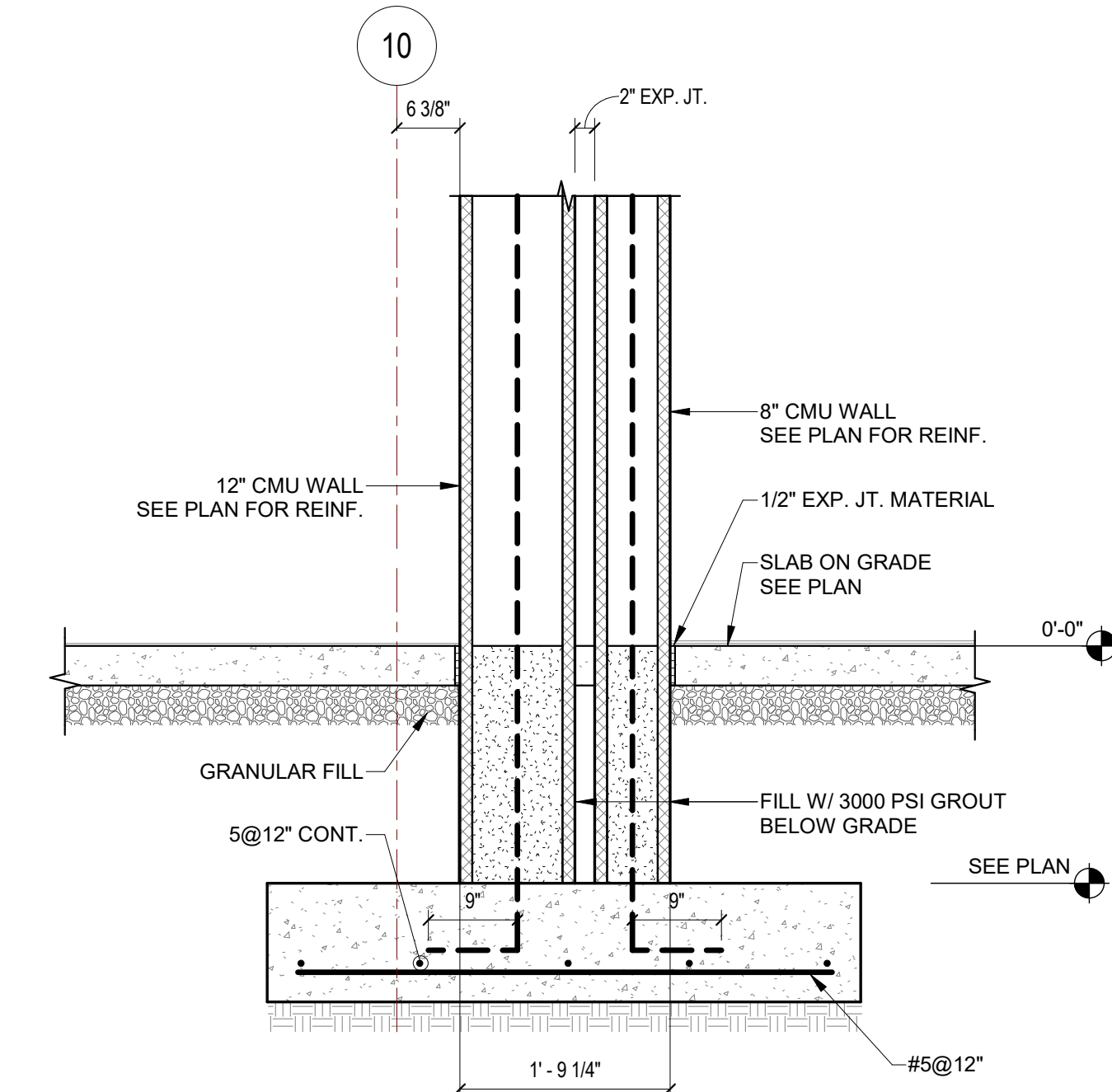
8 TYPICAL PIER SECTION  
S-302 SCALE: 3/4" = 1'-0"



7 RAMMED AGGREGATE PIERS NO-DIG ZONE DETAIL  
S-302 SCALE: 1" = 1'-0"



9 PIER P1 DETAIL  
S-302 SCALE: 1" = 1'-0"

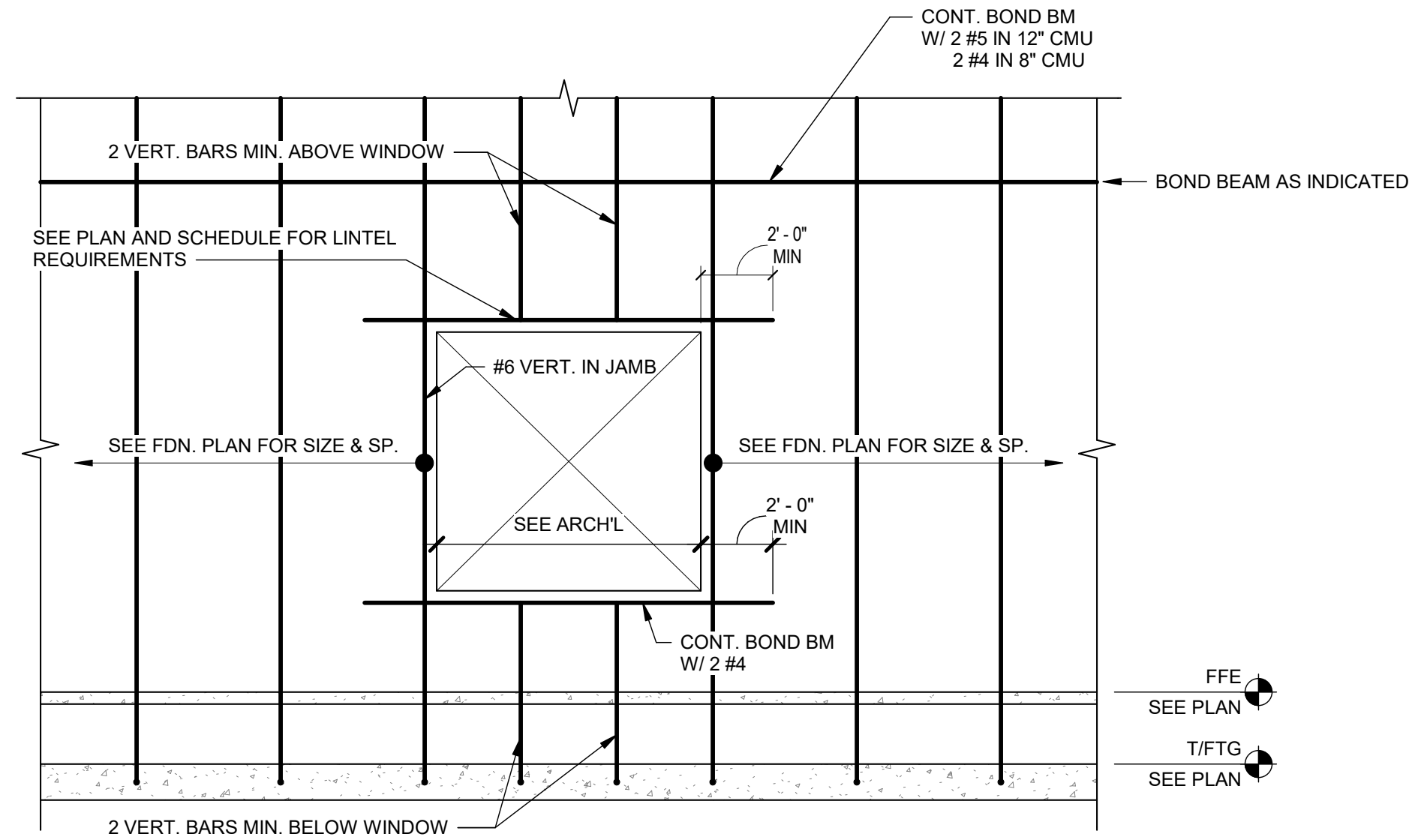


6 CMU DETAIL  
S-302 SCALE: 3/4" = 1'-0"

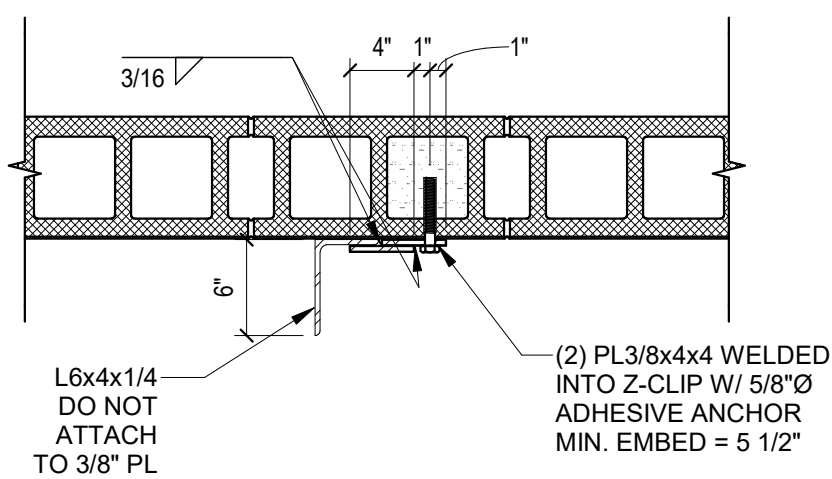


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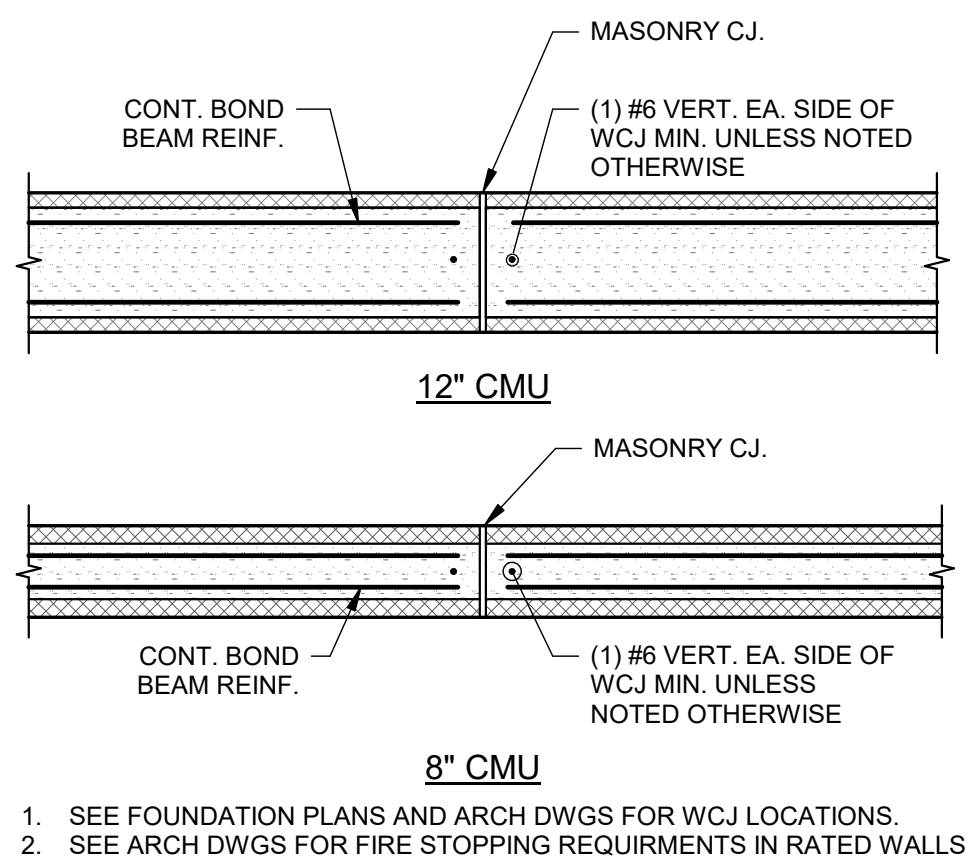
11 TYP. WALL ELEVATION - AT WINDOW  
S-303 SCALE: 1/4" = 1'-0"



10 SLIP CONNECTION AT FIREWALL  
S-303 SCALE: 1" = 1'-0"

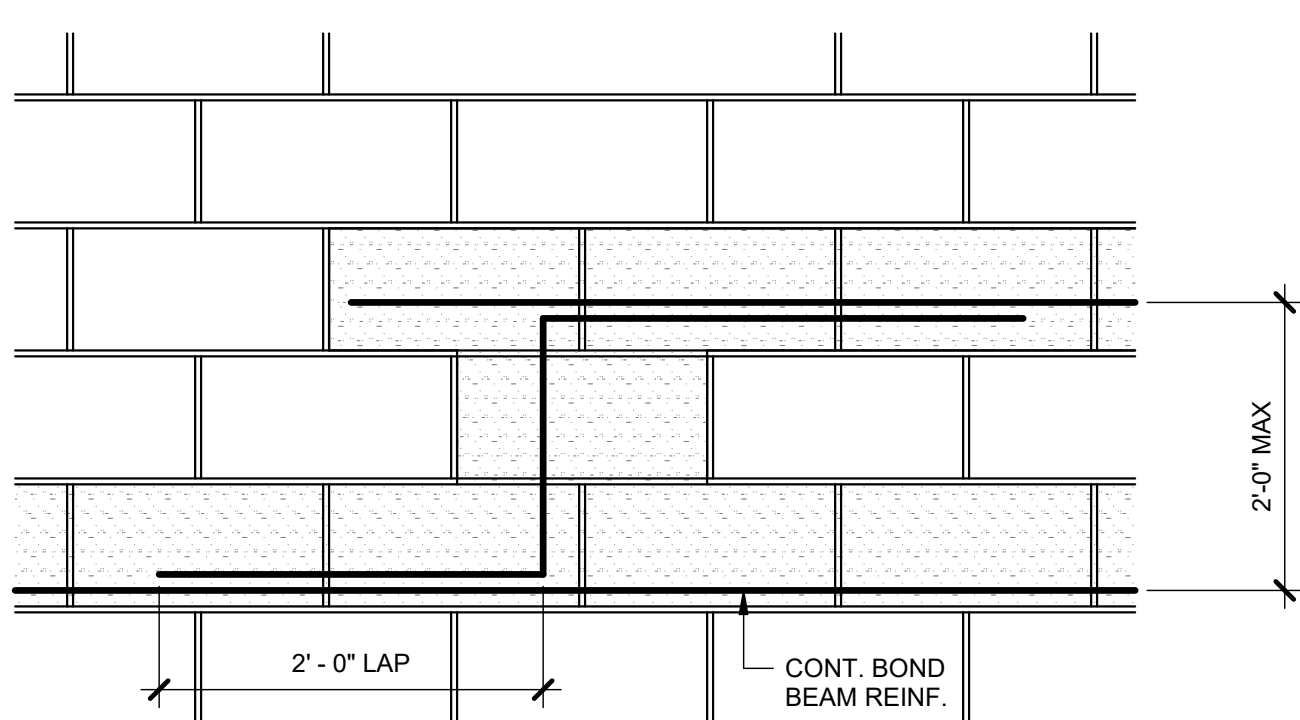


7 TYP. CMU LINTEL DETAIL  
S-303 SCALE: 1" = 1'-0"

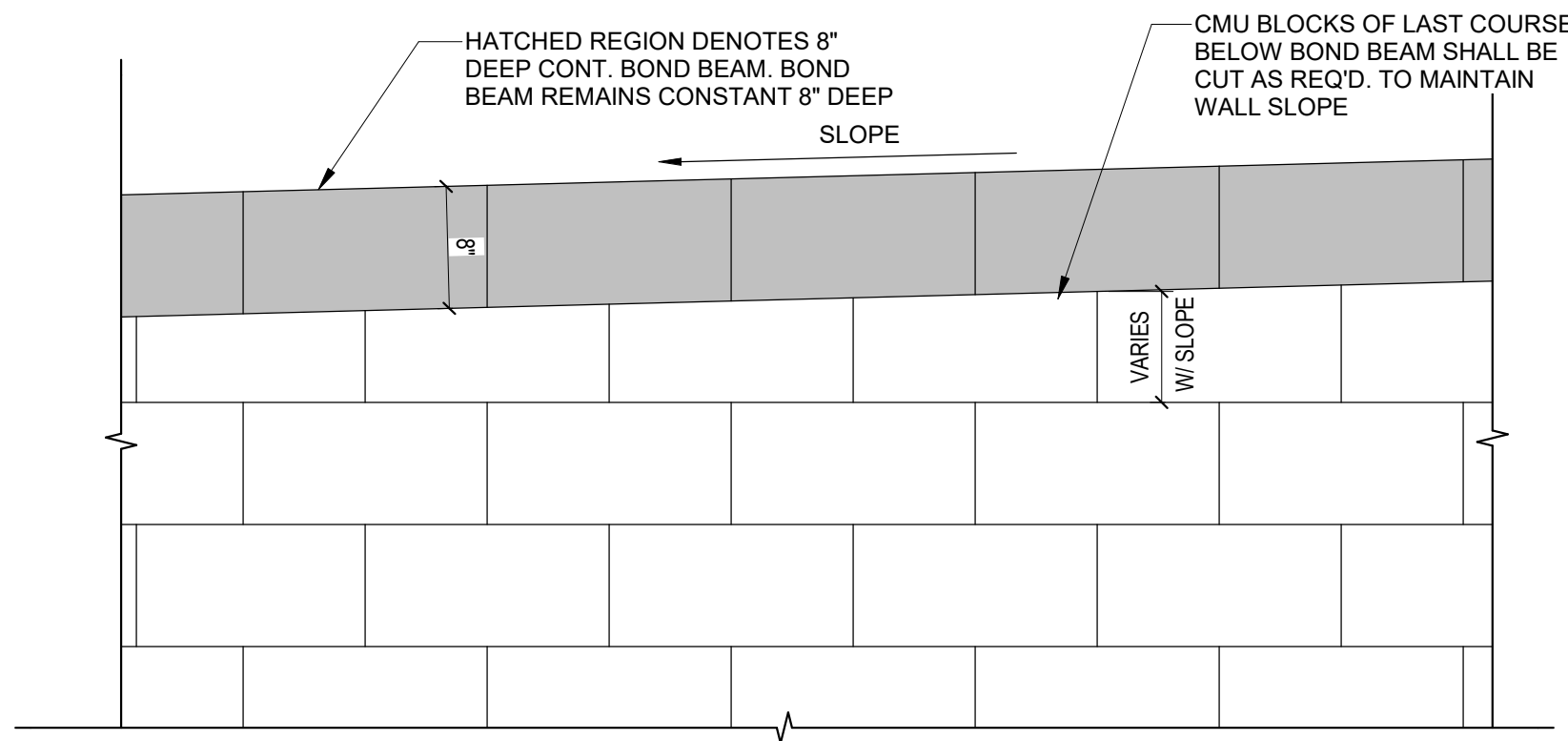


5 TYP. CONTROL JOINT DETAIL  
S-303 SCALE: 3/4" = 1'-0"

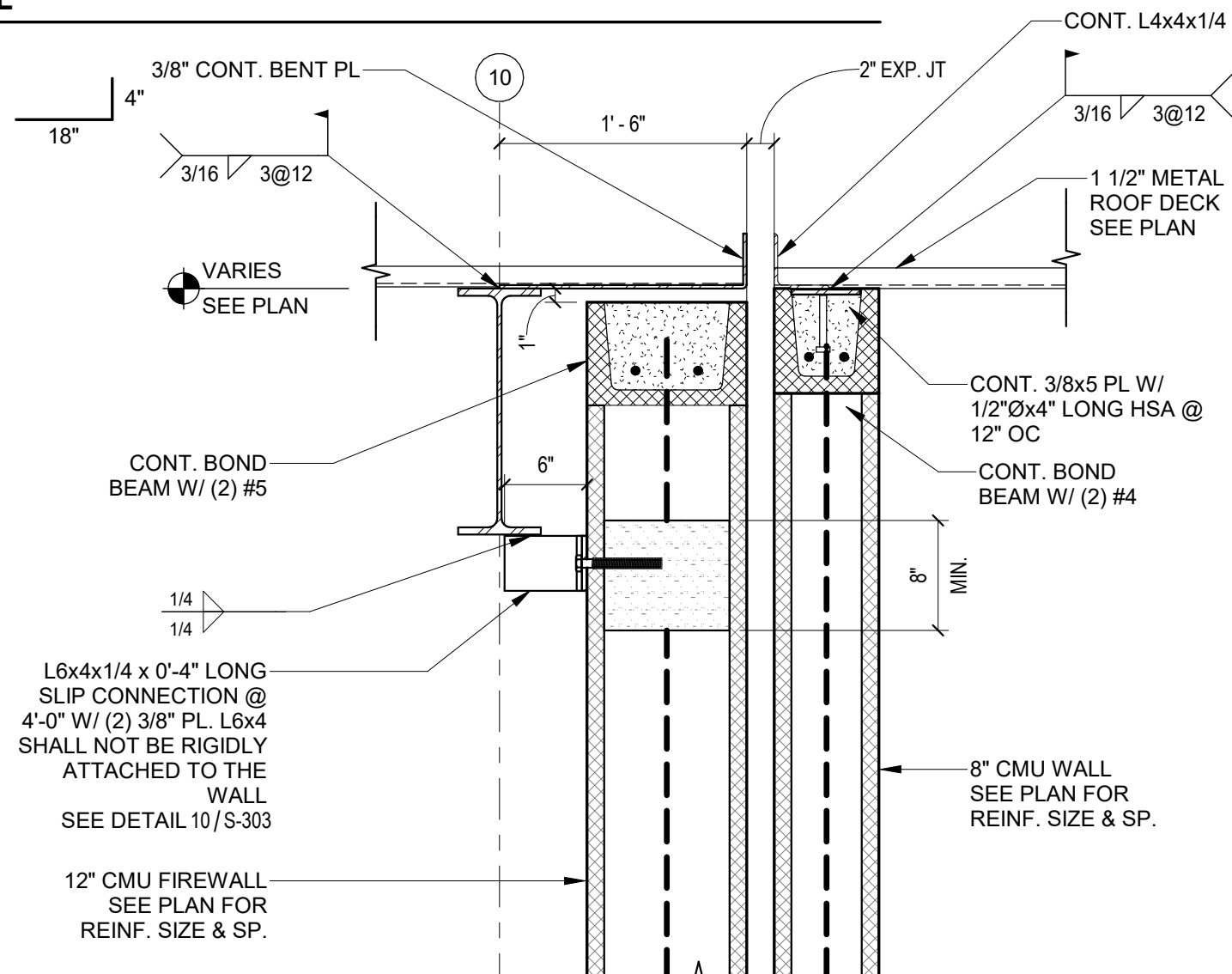
4 TYP. STEP IN BOND BEAM  
S-303 SCALE: 1" = 1'-0"



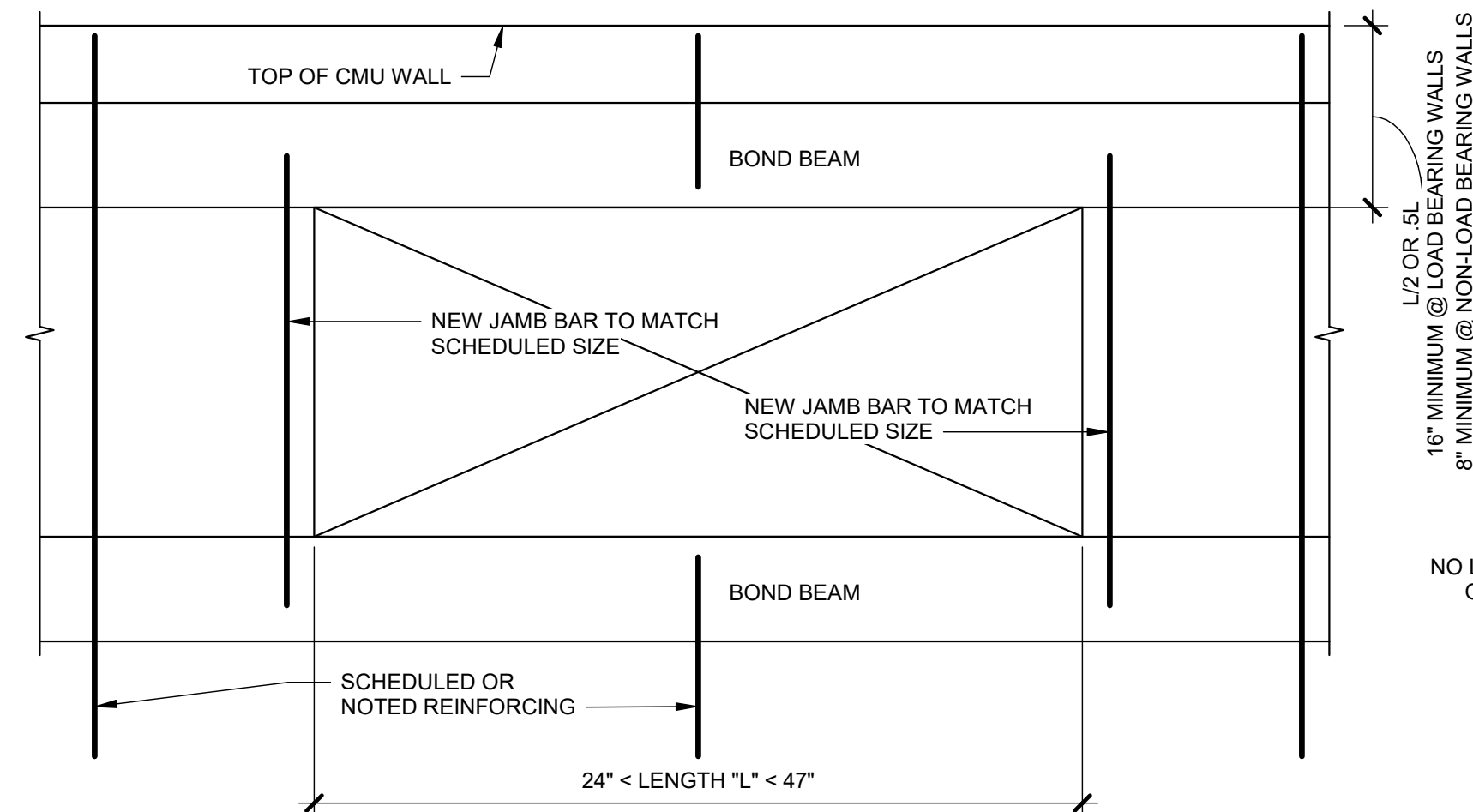
13 SLOPED BOND BEAM DETAIL  
S-303 SCALE: 1" = 1'-0"



9 DETAIL AT FIREWALL  
S-303 SCALE: 1" = 1'-0"



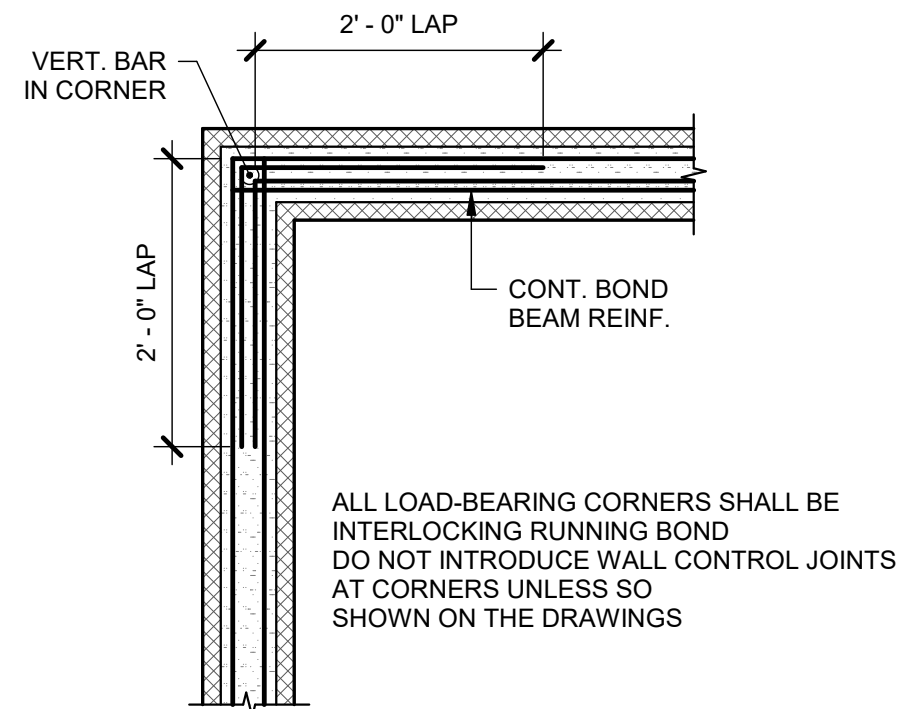
6 CMU WALL UTILITY PENETRATION AND EMBEDMENT TYPICAL DETAIL  
S-303 SCALE: 1" = 1'-0"



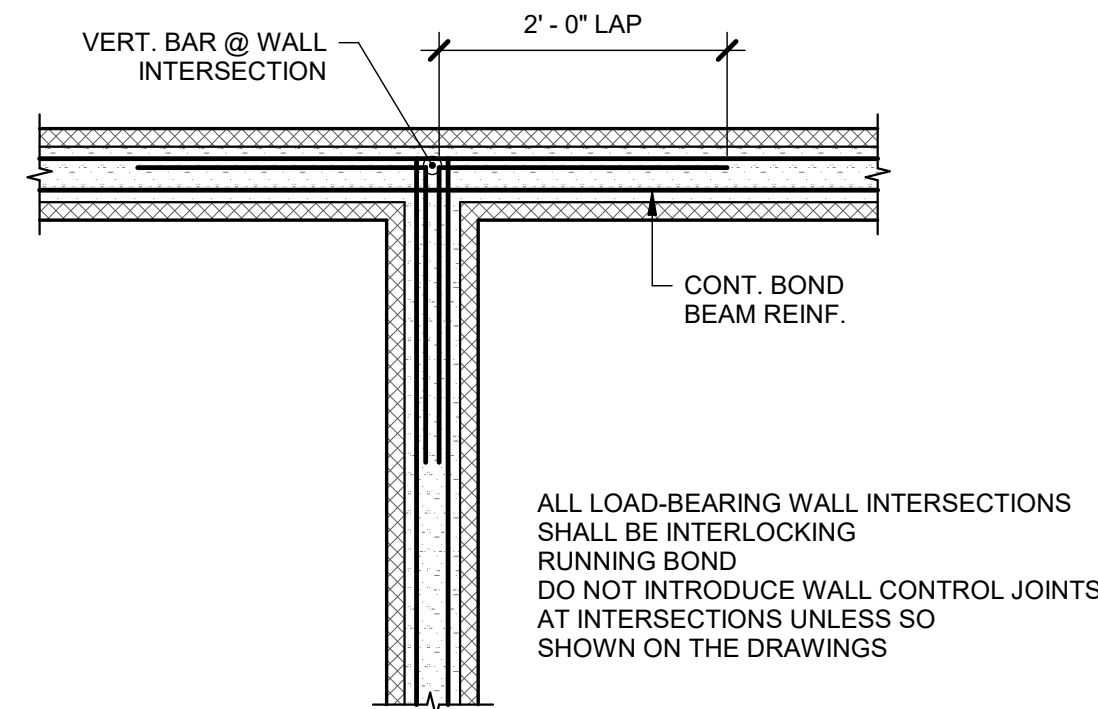
DUCT OPENINGS UNDER 47" WIDE PROVIDE BOND BEAM AT BOTTOM. JAMB BARS ON EITHER SIDE, AND BOND BEAM ACROSS THE TOP - AS LONG AS DUCT IS NOT OVER 48" AND REBAR IS NOT MORE THAN 48" APART. IF REBAR IS MORE THAN 48", JAMB REINFORCING MUST BE INSTALLED FROM BELOW. FULL HT. BOND BEAMS SHALL BE REINFORCED WITH (2) #4 BARS.

NON-LOAD BEARING INTERIOR PARTITIONS OPENING SIZE:  
LESS THAN OR EQUAL TO 24" - REFER TO ADJACENT DETAIL (ROUND OR RECTANGULAR)  
GREATER THAN 24" AND LESS THAN 48" - SEE DETAIL ABOVE  
GREATER THAN 48" - LINTEL SHALL BE PROVIDED IN ACCORDANCE WITH THE NON-LOAD BEARING LINTEL SCHEDULE ON SHEET S-401. FOR MASONRY LINTEL, THE HEIGHT OF CMU ABOVE THE OPENING MUST MEET THE L/2 OR .5L CRITERIA - OTHERWISE A STEEL LINTEL MUST BE PROVIDED IN ACCORDANCE WITH THE SCHEDULE. PROVIDE FULL HEIGHT JAMBS EACH SIDE.  
REFER TO PLANS AND SCHEDULE FOR ALL LINTELS AT LOAD BEARING OR EXTERIOR WALLS. FOR MEP PENETRATIONS NOT IDENTIFIED ON PLANS USE THE ABOVE CRITERIA.

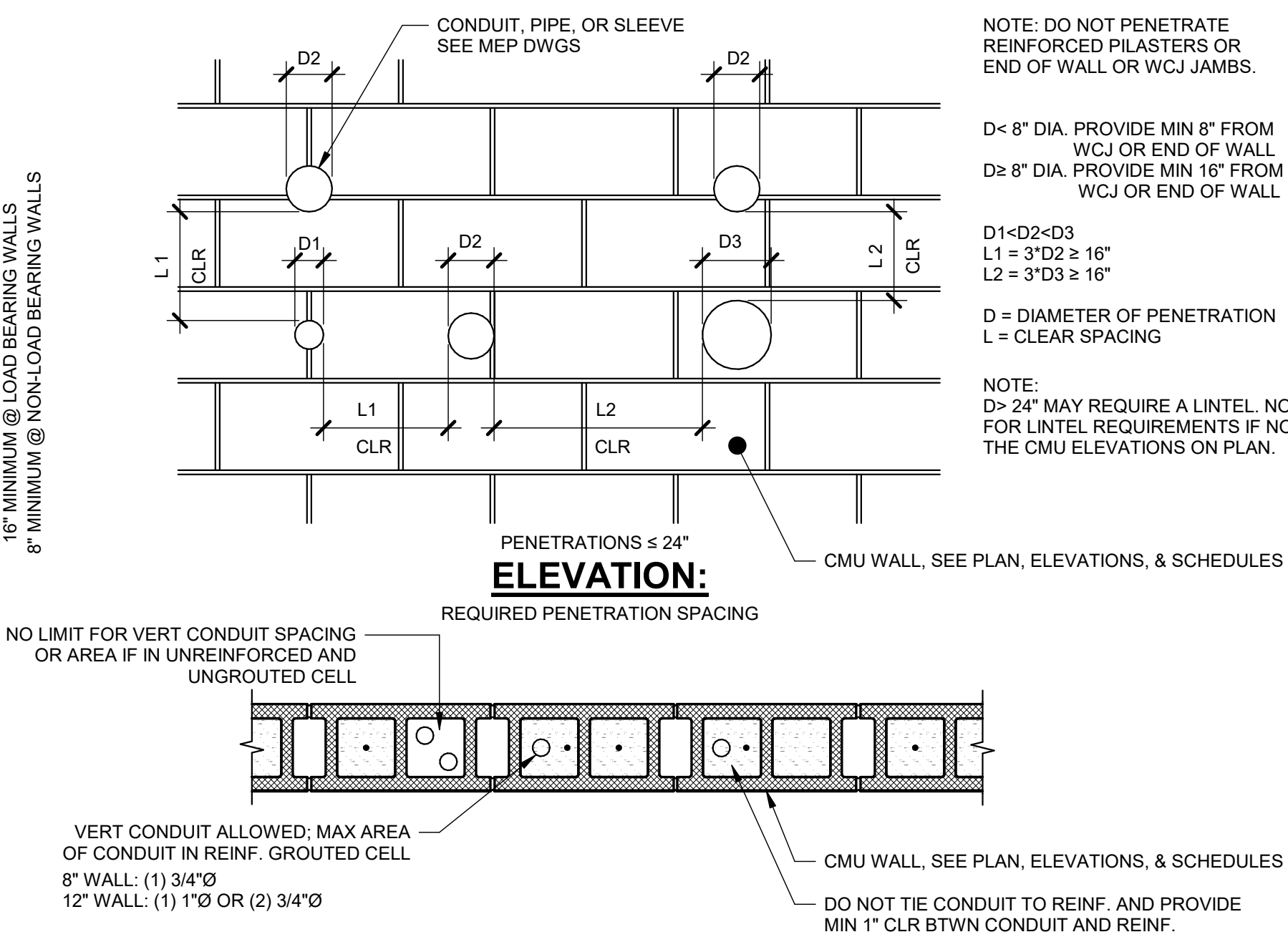
3 TYP. BOND BEAM CORNER  
S-303 SCALE: 3/4" = 1'-0"



2 TYP. BOND BEAM INTERSECTION  
S-303 SCALE: 3/4" = 1'-0"

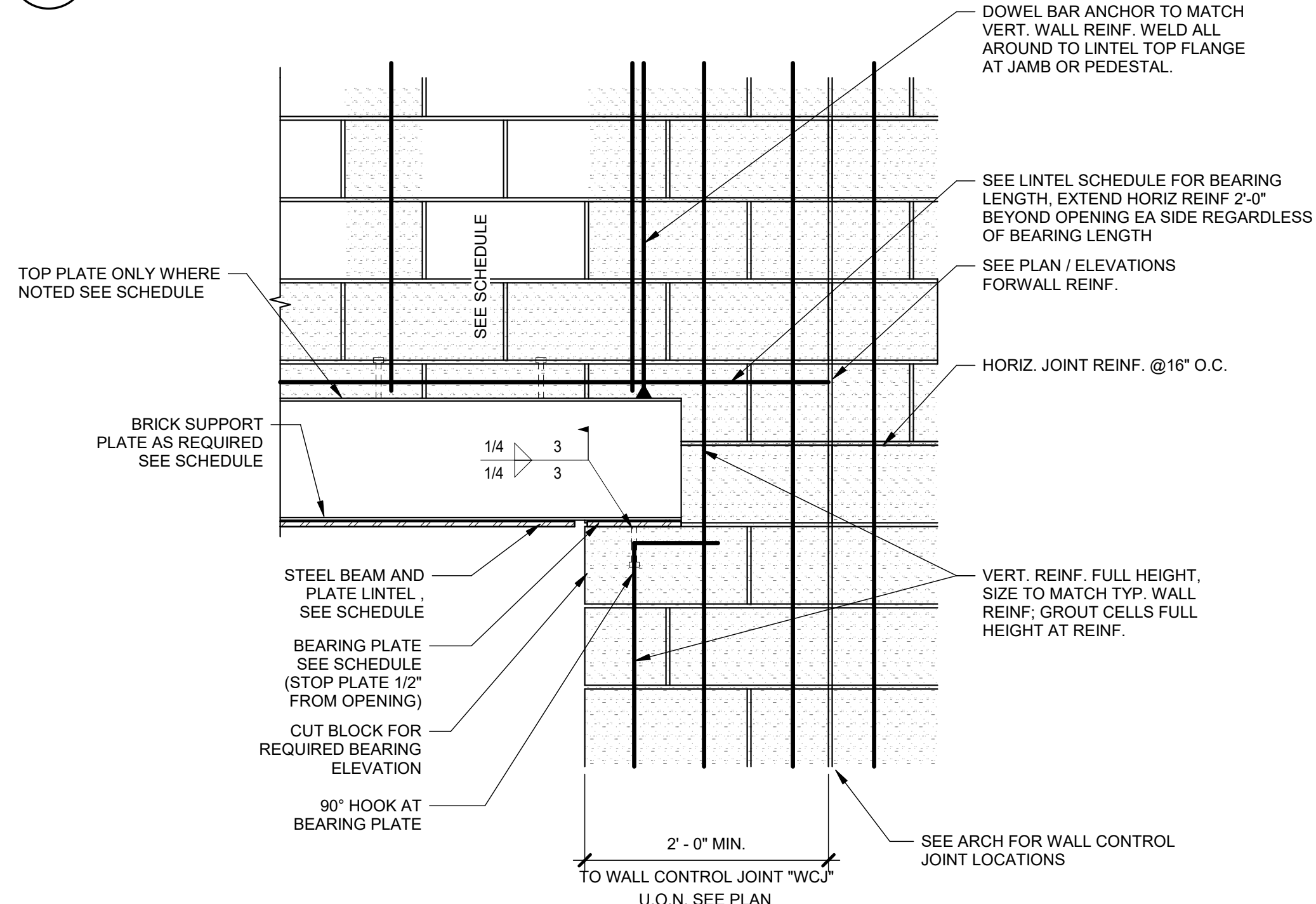


8 TYP. STEEL LINTEL DETAIL  
S-303 SCALE: 1" = 1'-0"



NOTES:  
1. REFER TO MEP DRAWINGS FOR ALL TELECOM, CONDUIT, AND PIPES 8" Ø AND SMALLER. CONTRACTOR TO FOLLOW DETAIL SPACING REQUIREMENTS FOR LAYOUT. IF SPACING CANNOT BE MAINTAINED, PENETRATIONS MUST BE GROUPED BELOW CMU LINTEL. NOTIFY THE EOR FOR LINTEL REQUIREMENTS.  
2. CONDUITS SHALL NOT PENETRATE BOND BEAMS NOR LINTELS UNLESS NOTED OTHERWISE.

12 REINFORCED PILASTERS  
S-303 SCALE: 1 1/2" = 1'-0"

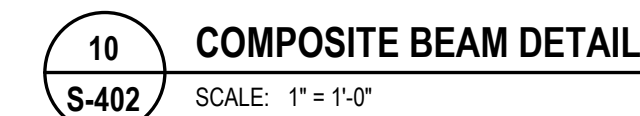
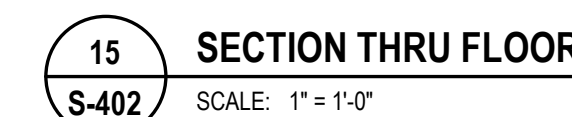








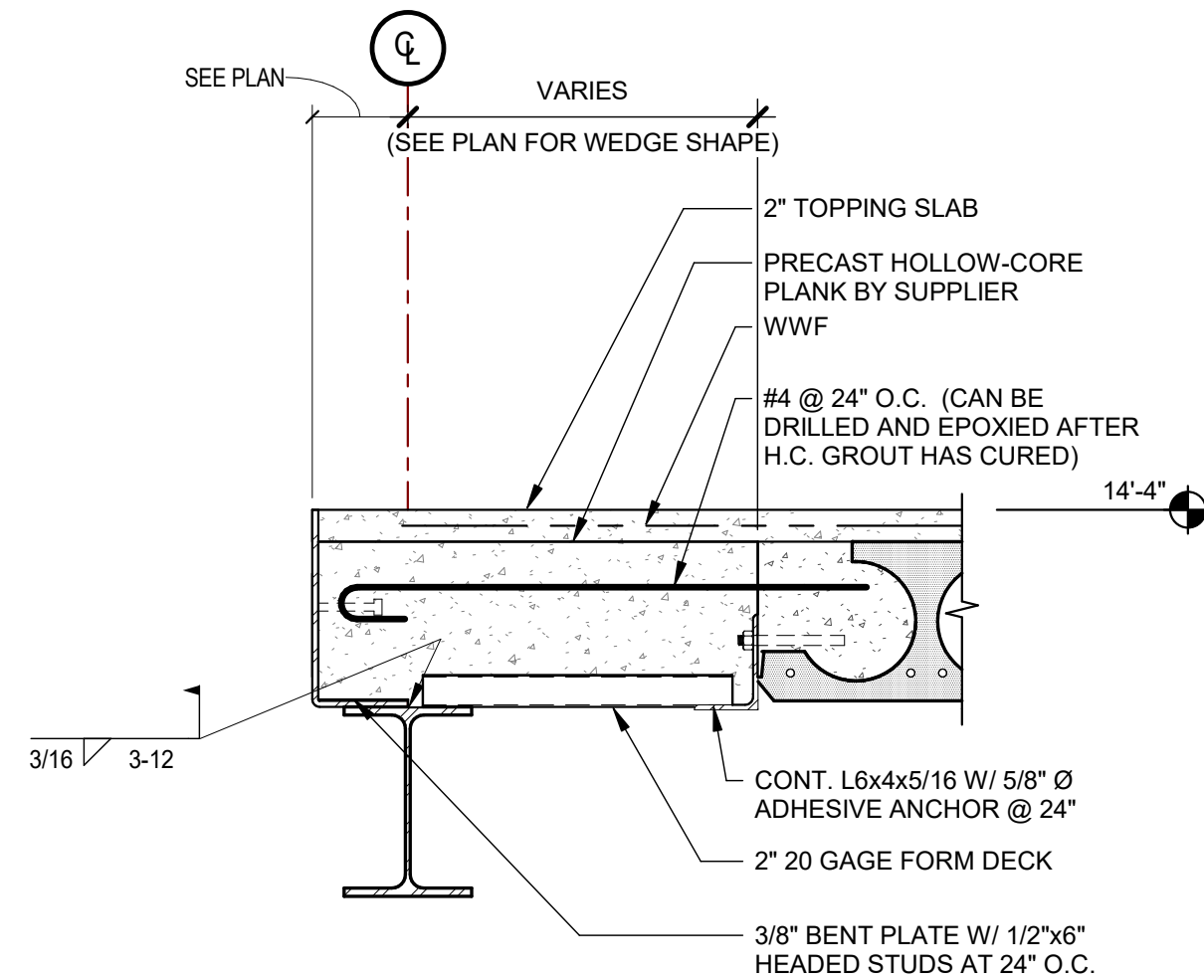
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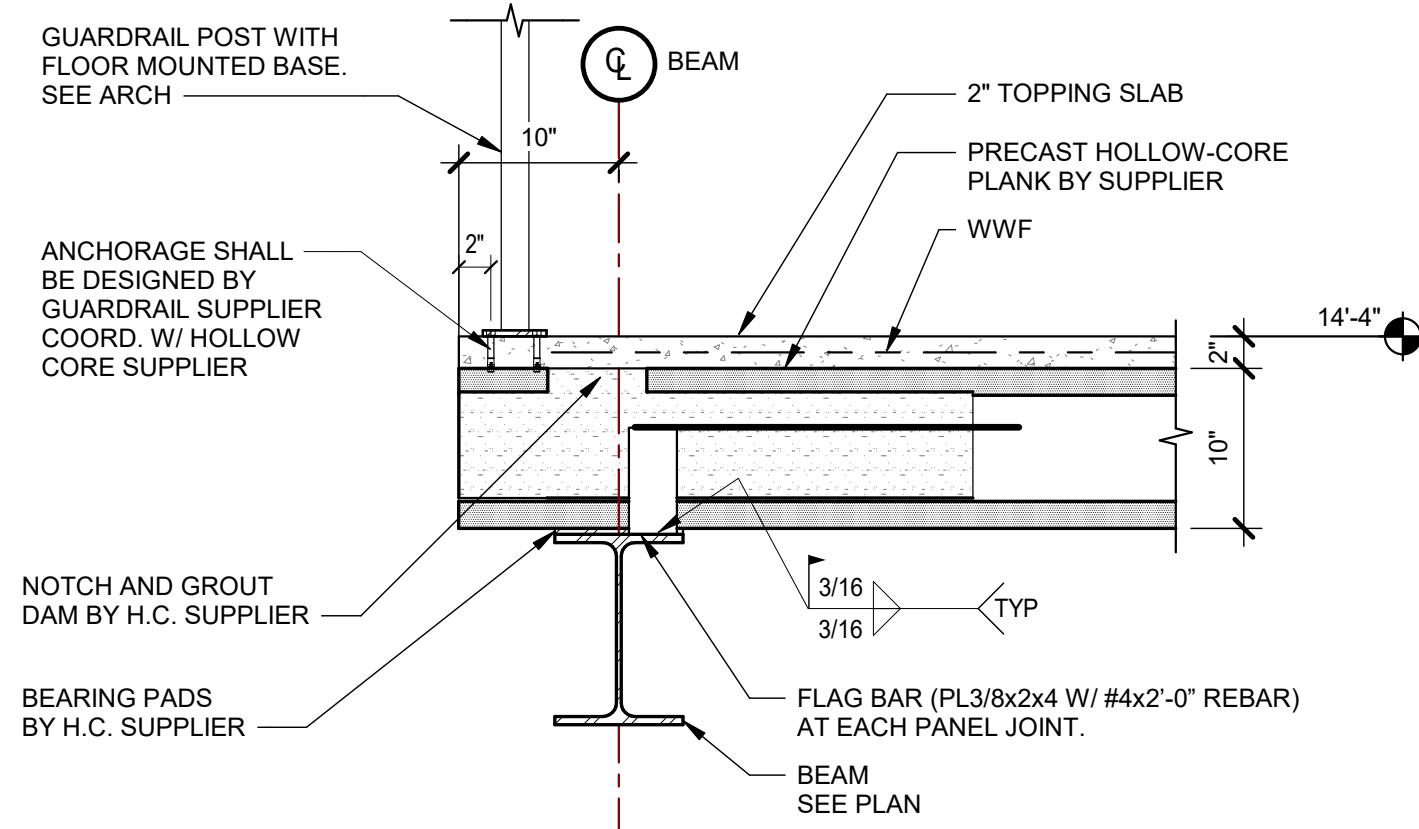


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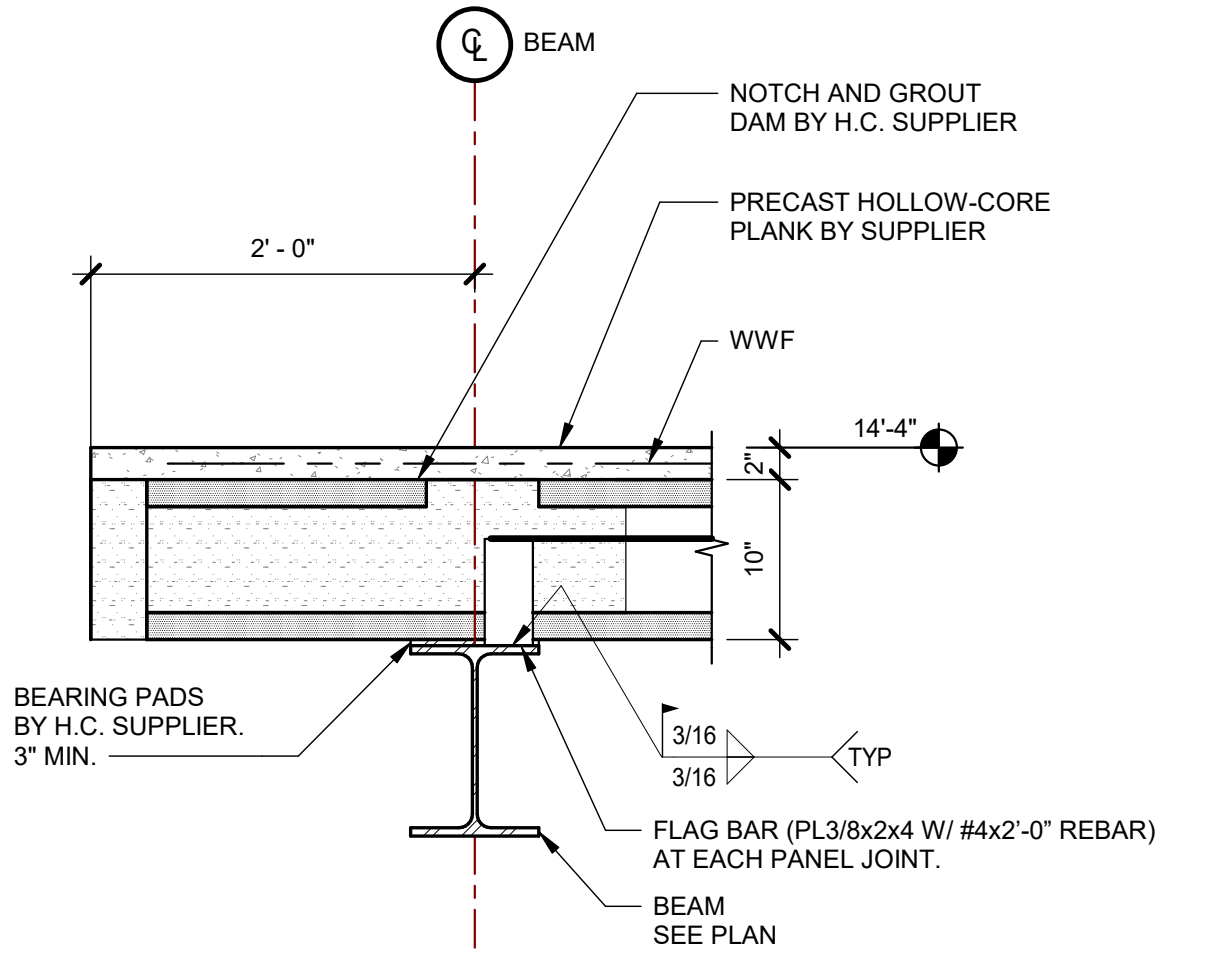
**4** **DETAIL AT CAST-IN-PLACE INFILL**  
S-403 SCALE: 1" = 1'-0"



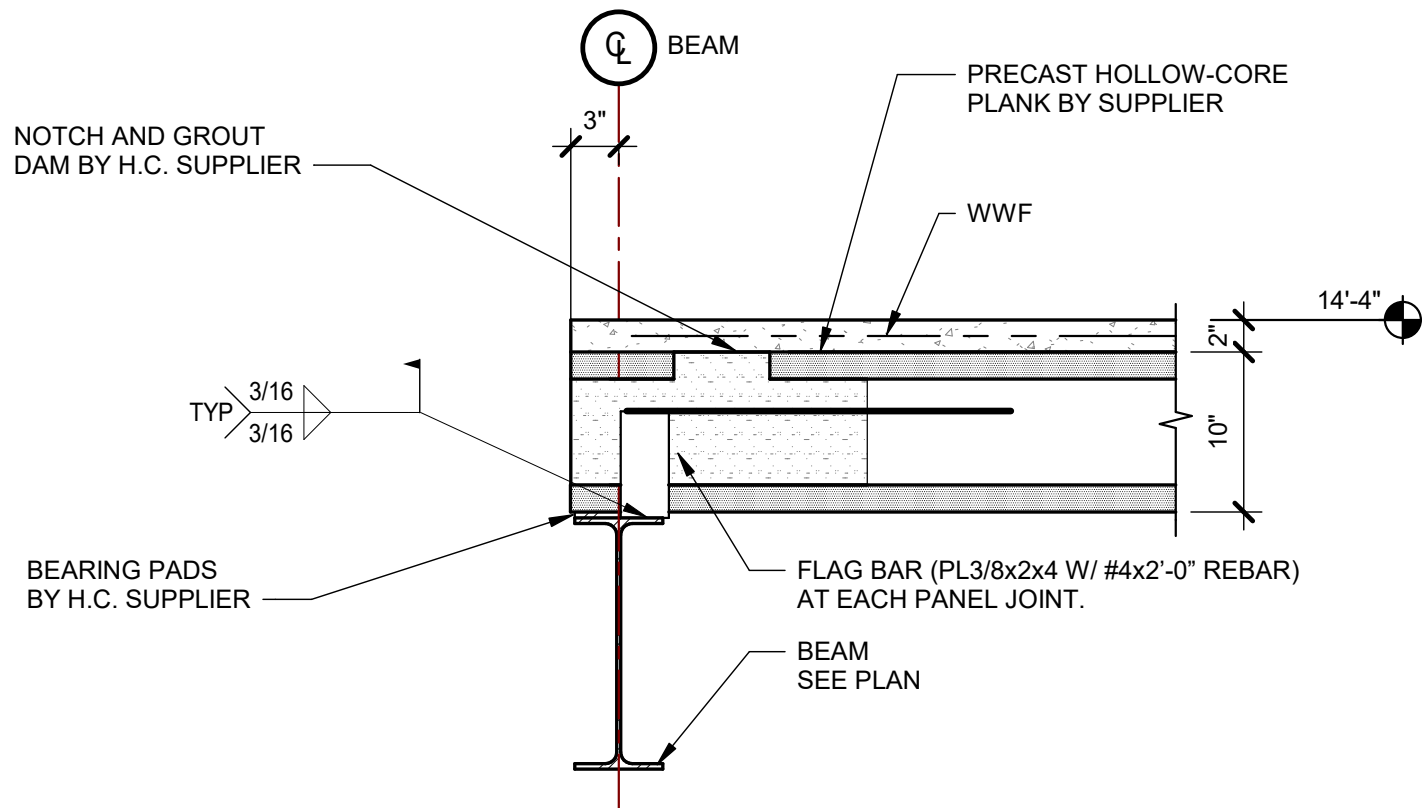
**5** **DETAIL AT HANDRAIL**  
S-403 SCALE: 1" = 1'-0"



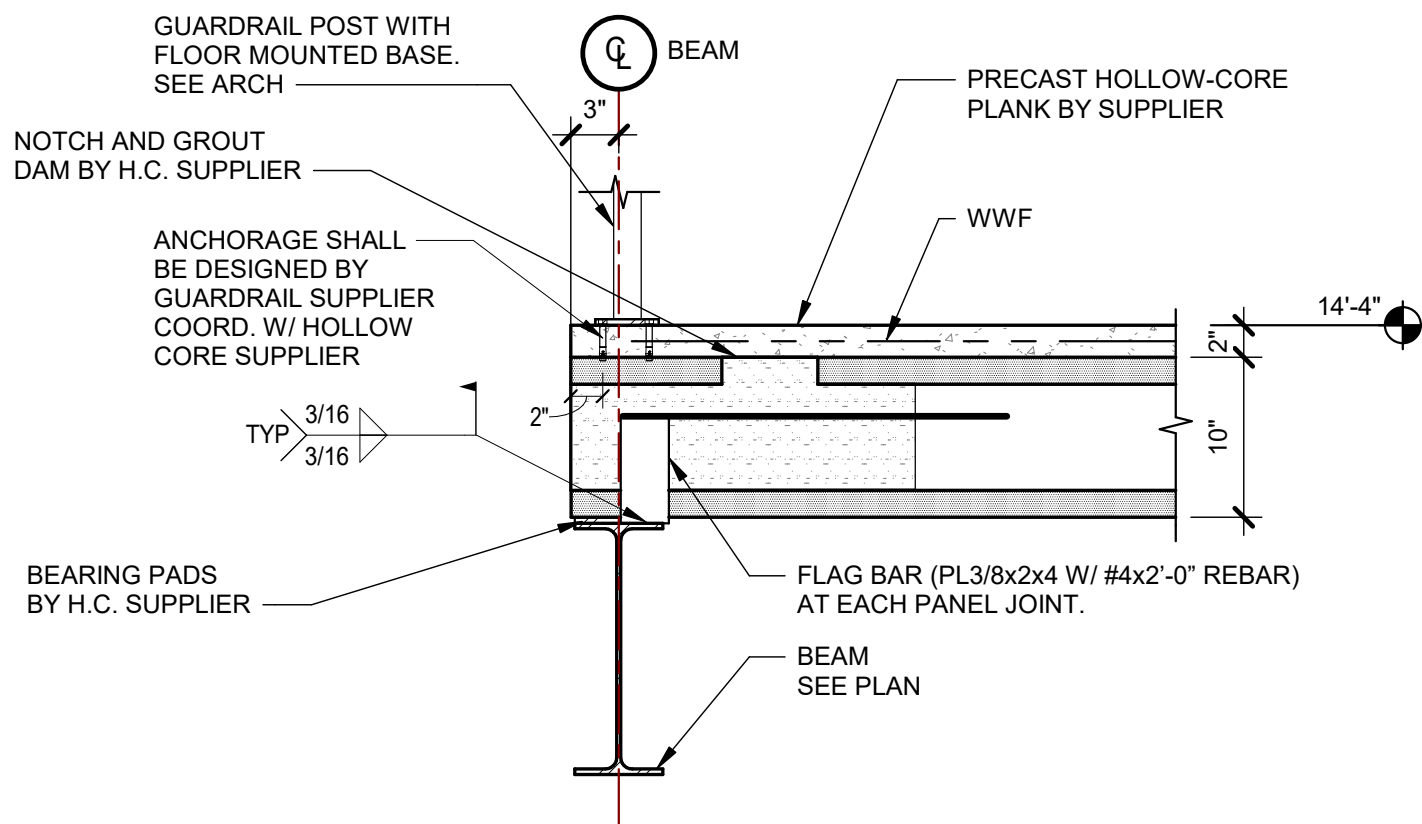
**8** **HOLLOW-CORE DETAIL**  
S-403 SCALE: 1" = 1'-0"



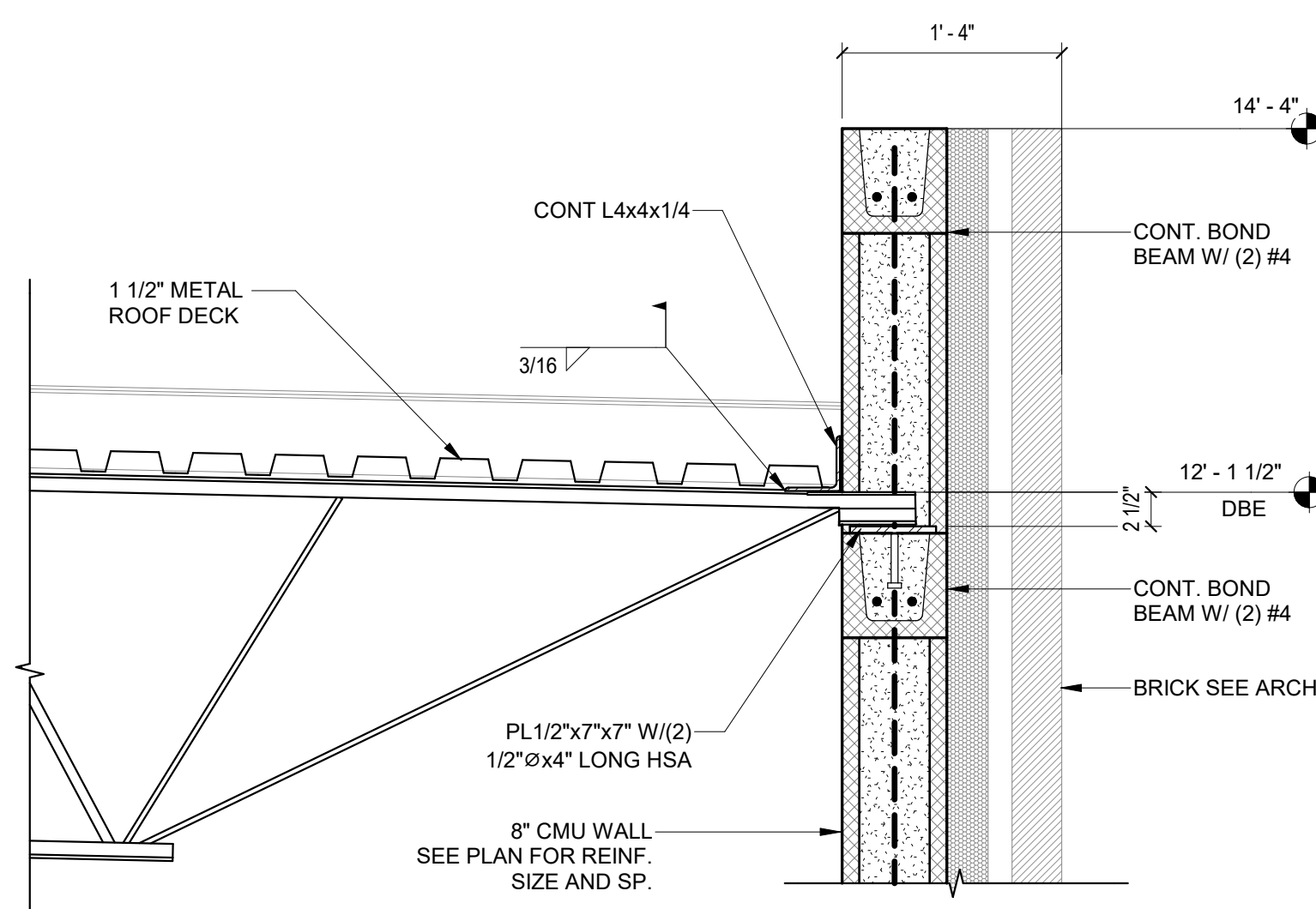
**11** **HOLLOW-CORE DETAIL**  
S-403 SCALE: 1" = 1'-0"



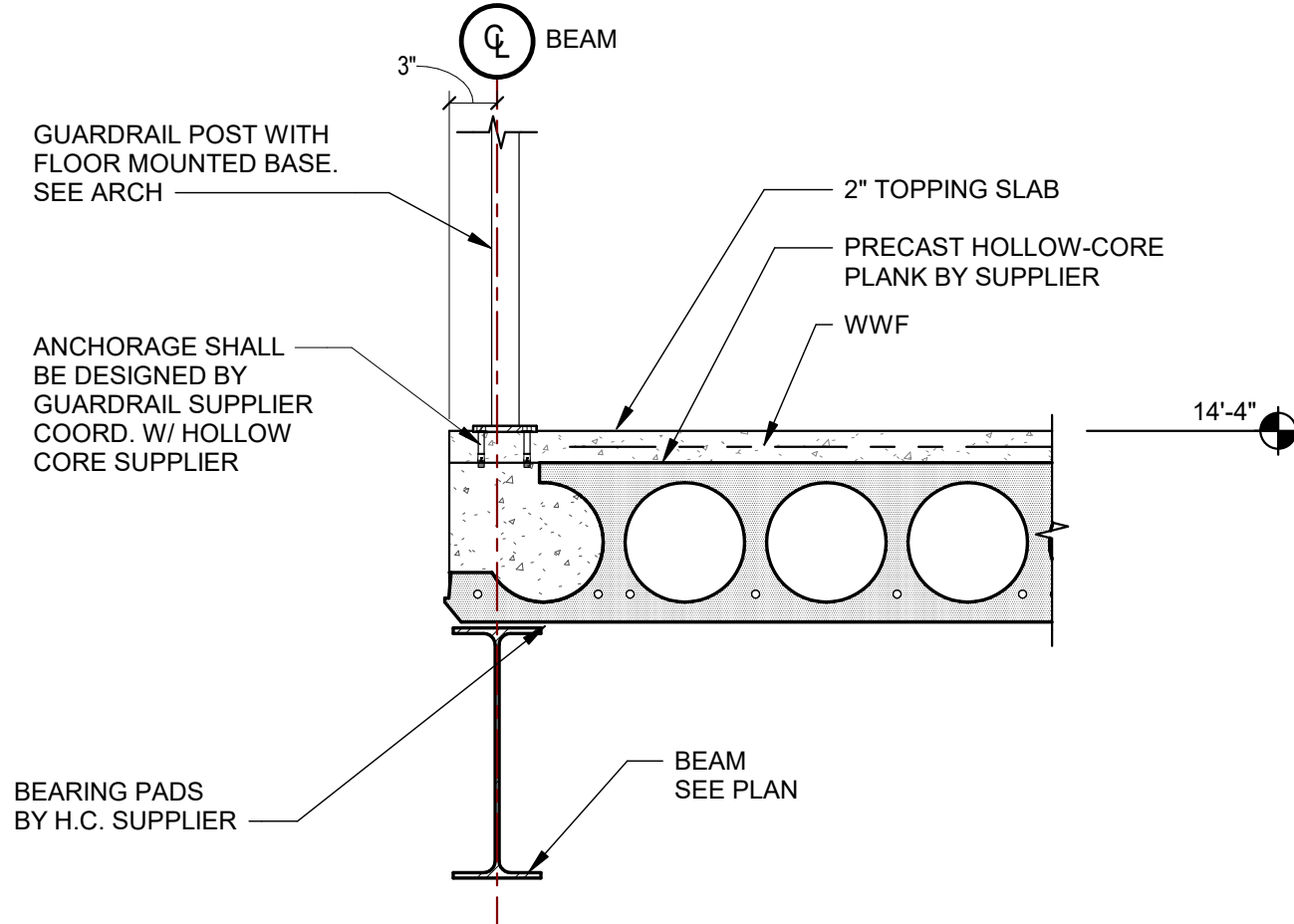
**12** **HOLLOW-CORE DETAIL**  
S-403 SCALE: 1" = 1'-0"



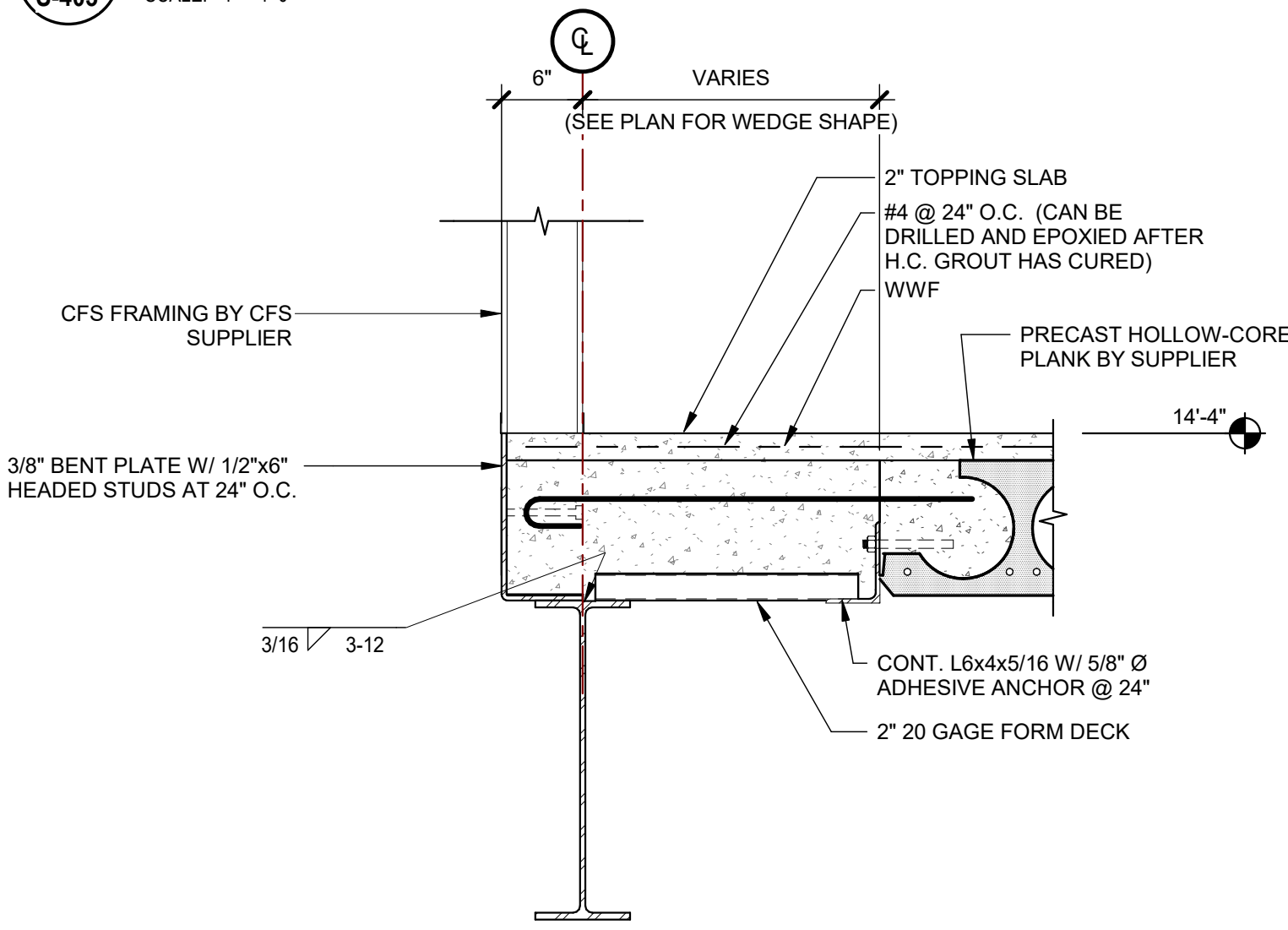
**3** **SECTION THRU LOW ROOF**  
S-403 SCALE: 1" = 1'-0"



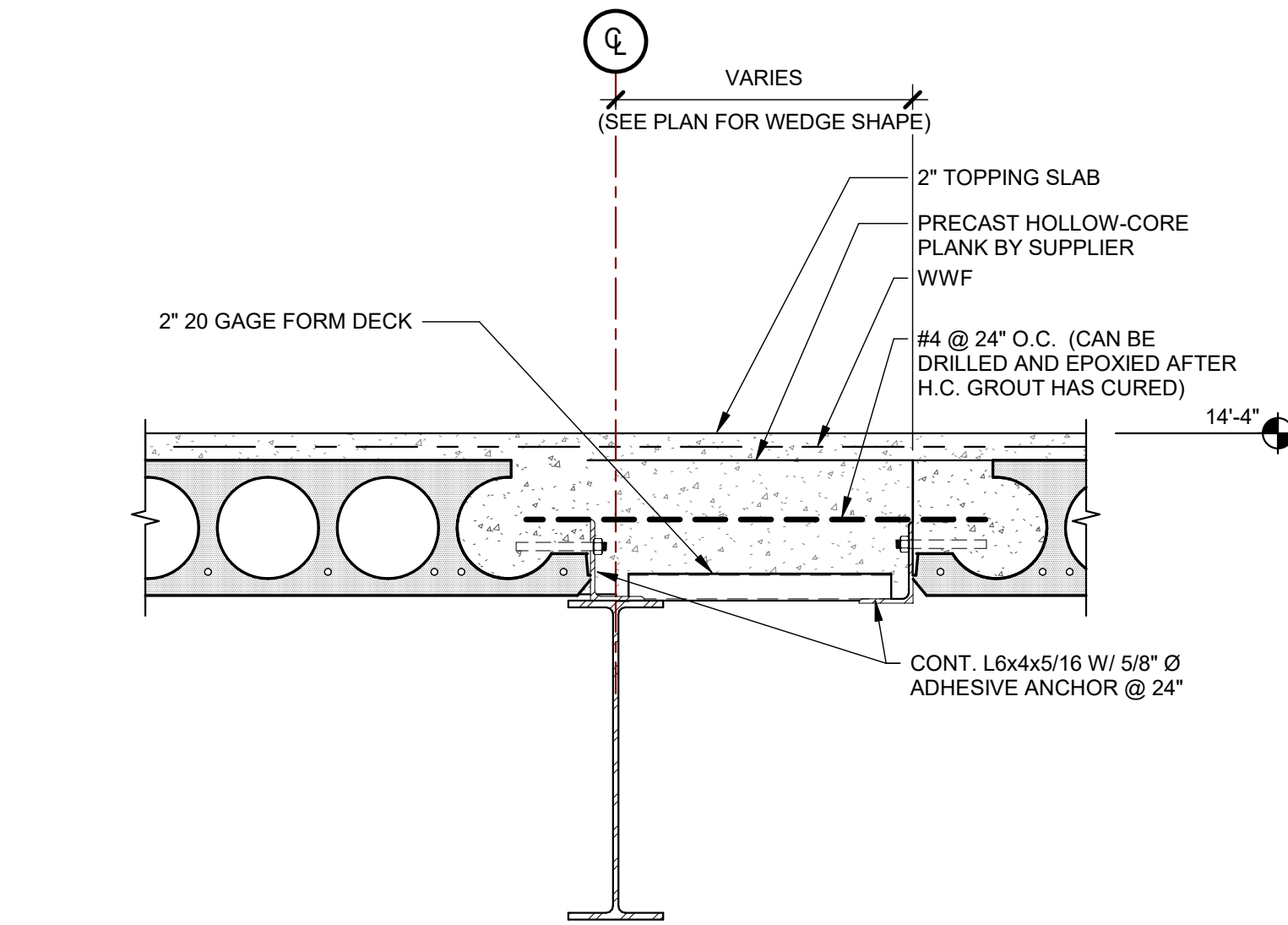
**7** **DETAIL AT HANDRAIL**  
S-403 SCALE: 1" = 1'-0"



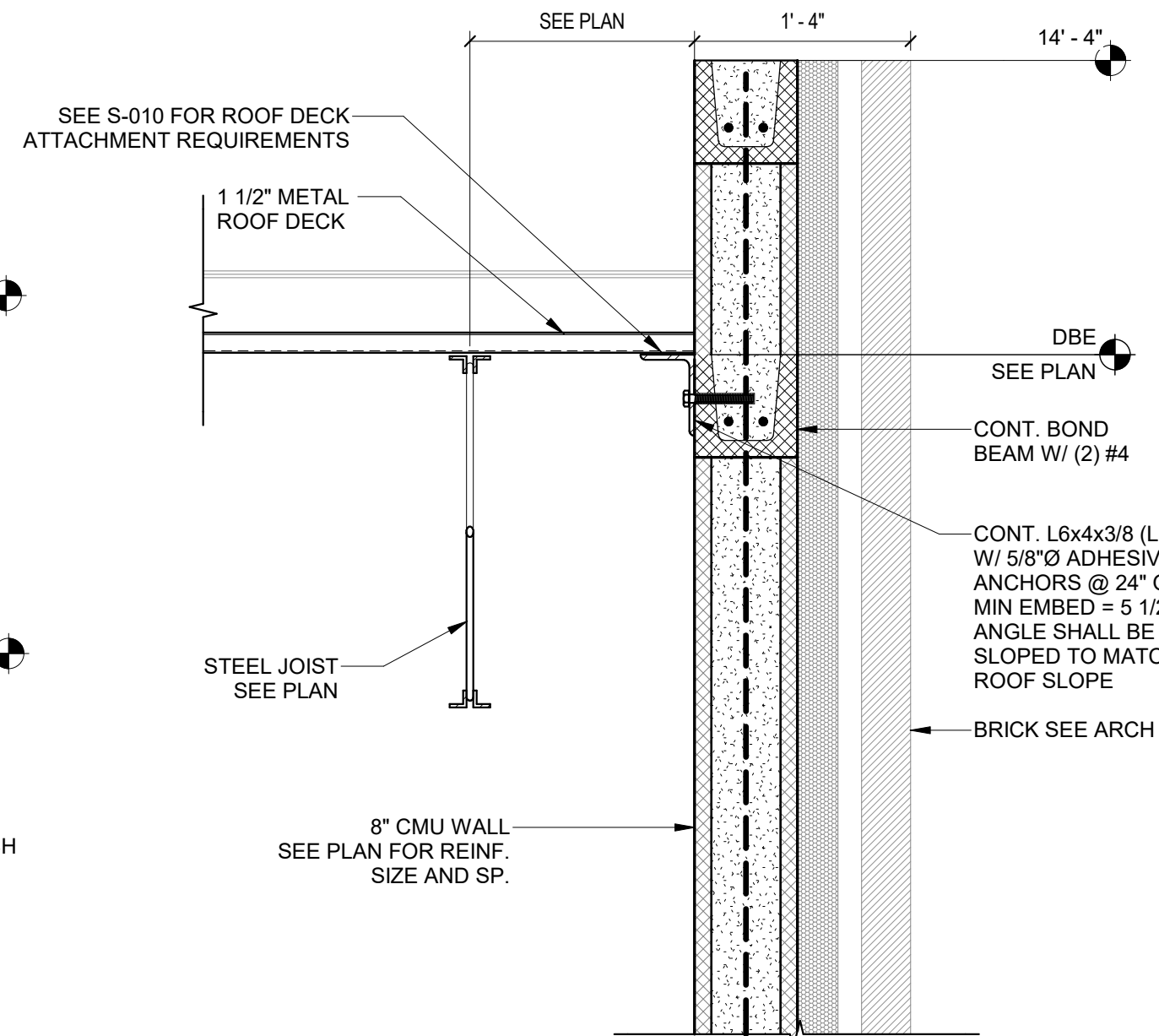
**10** **DETAIL AT CAST-IN-PLACE INFILL**  
S-403 SCALE: 1" = 1'-0"



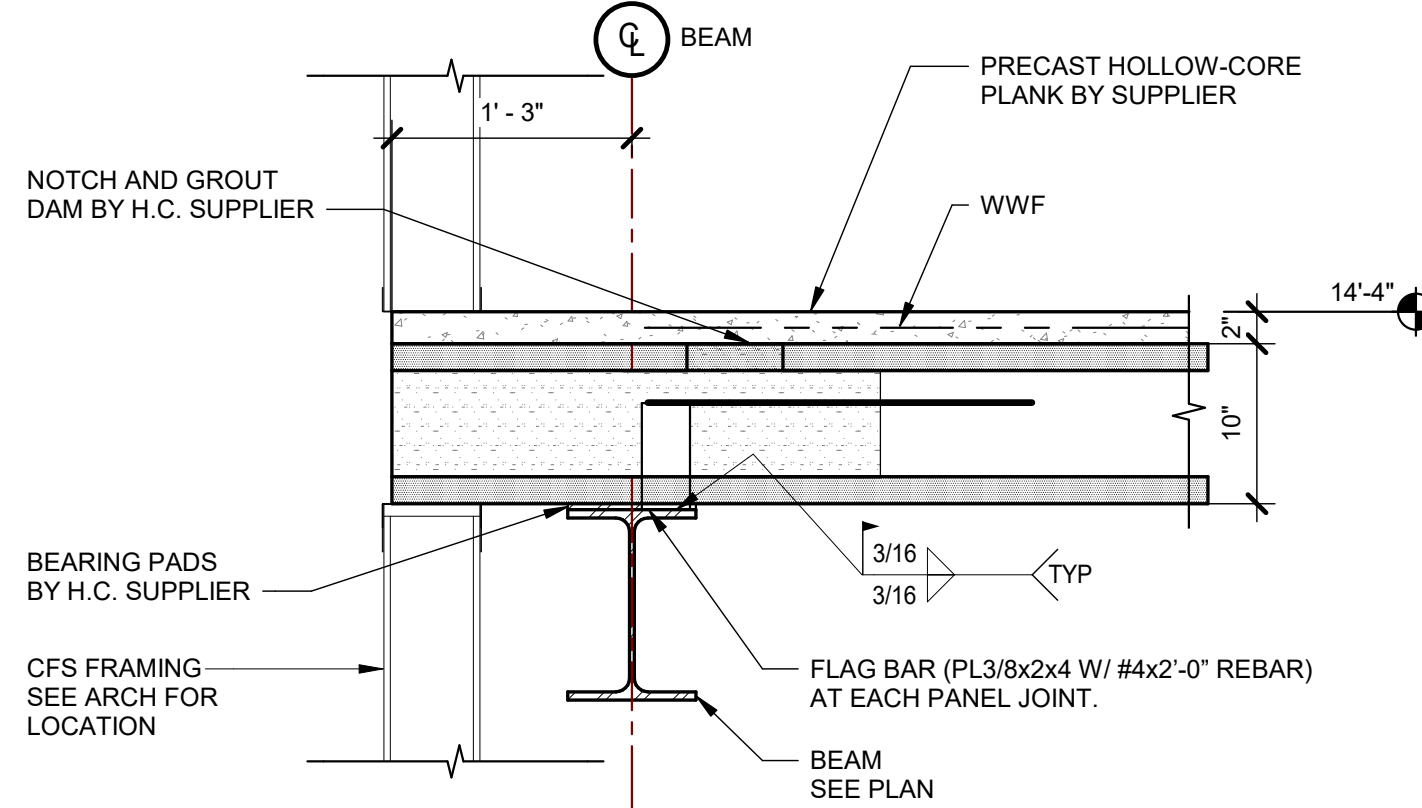
**13** **DETAIL AT CAST-IN-PLACE INFILL**  
S-403 SCALE: 1" = 1'-0"



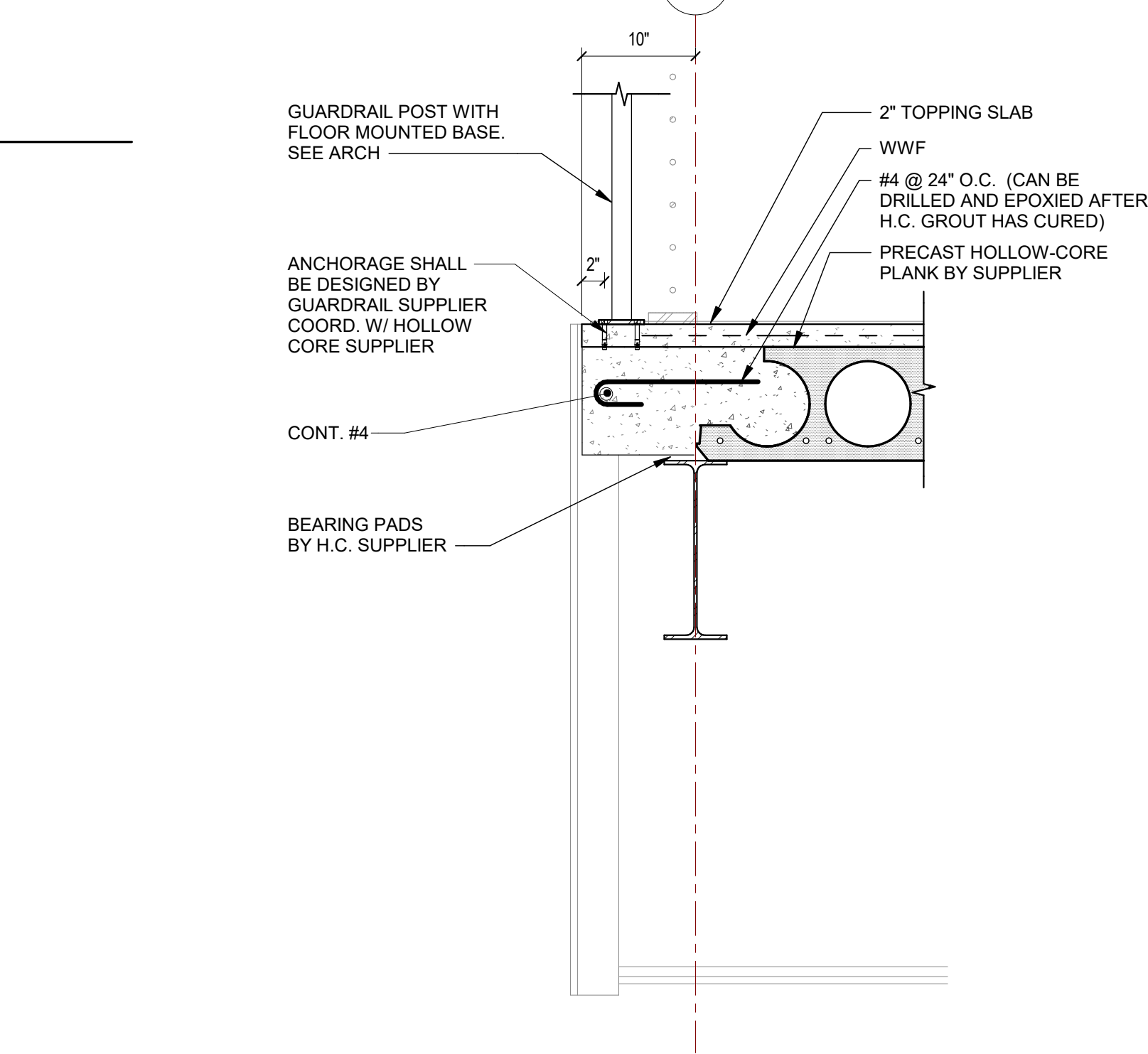
**2** **SECTION THRU LOW ROOF**  
S-403 SCALE: 1" = 1'-0"



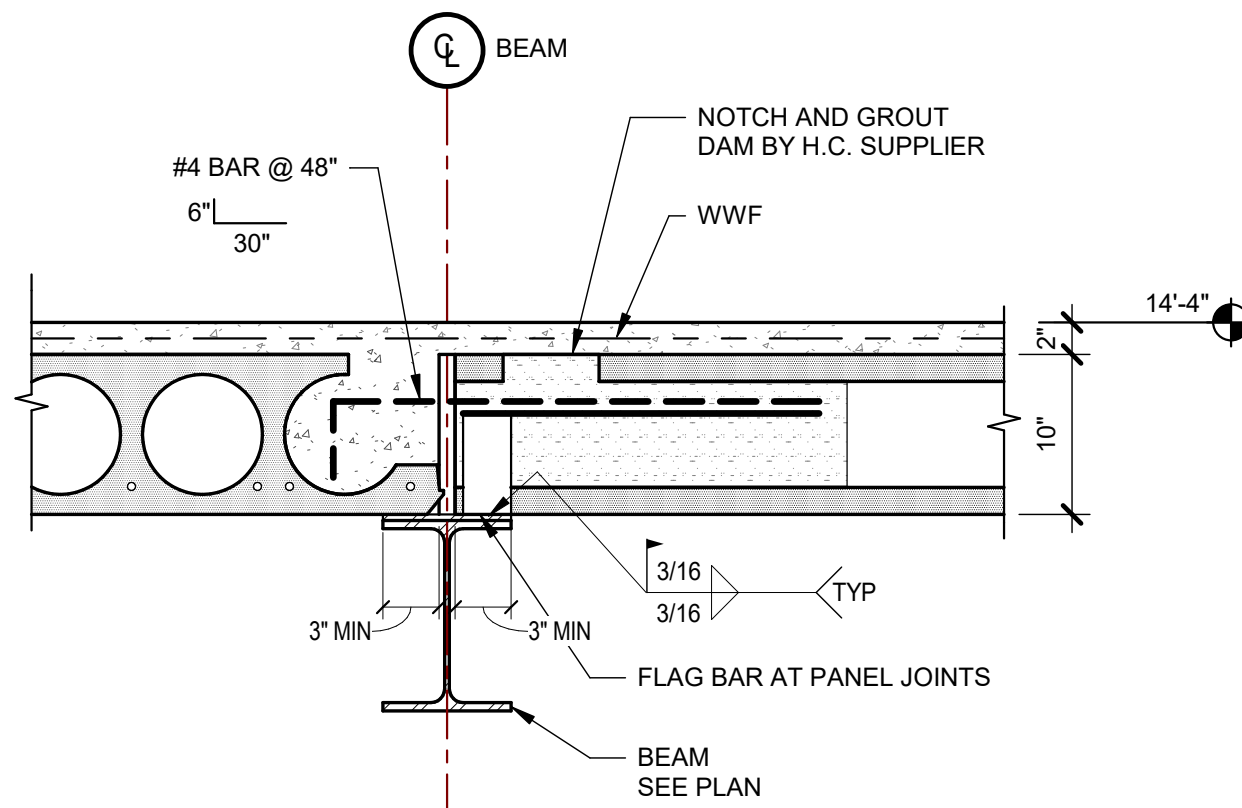
**6** **HOLLOW-CORE DETAIL**  
S-403 SCALE: 1" = 1'-0"



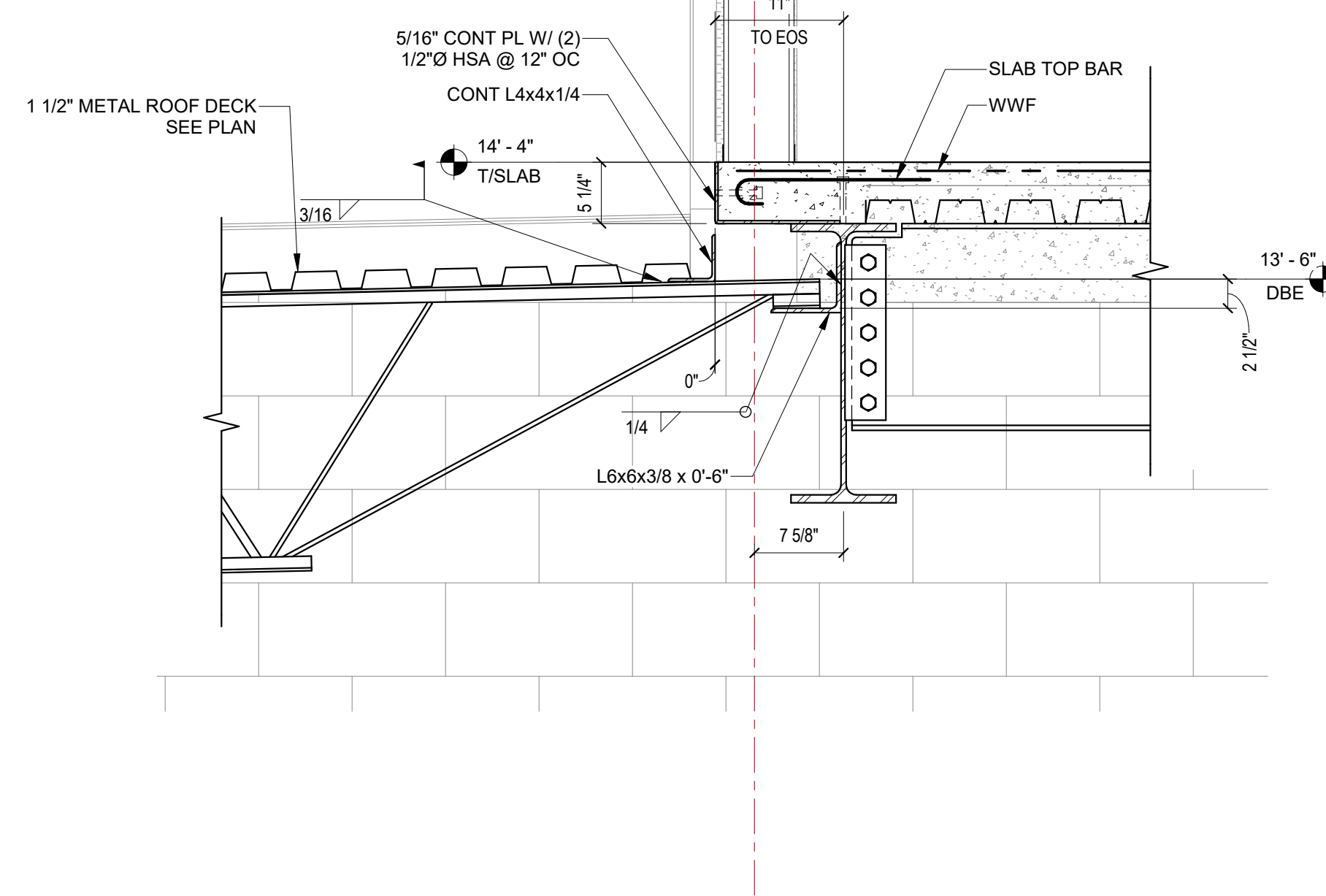
**9** **DETAIL AT HANDRAIL**  
S-403 SCALE: 1" = 1'-0"



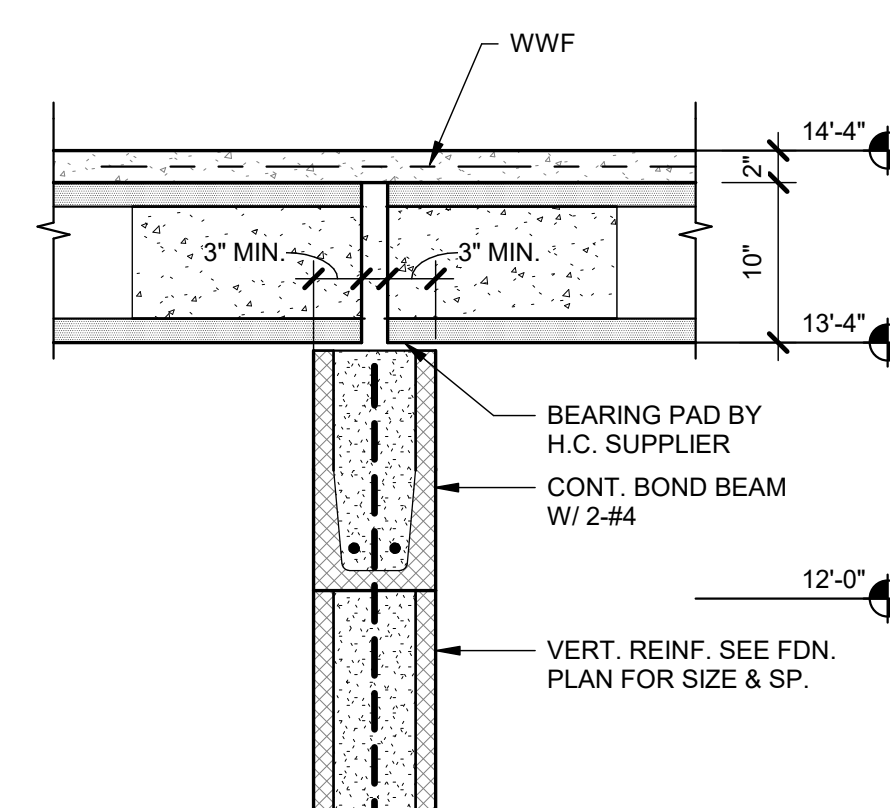
**14** **HOLLOW-CORE DETAIL**  
S-403 SCALE: 1" = 1'-0"



**1** **SECTION THRU MECHANICAL MEZZANINE**  
S-403 SCALE: 1" = 1'-0"



**15** **SECTION THRU FLOOR**  
S-403 SCALE: 1" = 1'-0"



**PAMLICO COUNTY**  
**PAMLICO 6-12 SCHOOL**  
601 Main Street, Bayboro, NC, 28515



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TH		CHECKED BY:

FRAMING DETAILS

S-403

CONSTRUCTION DOCUMENTS

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Specialty in Alternative  
Delivery Methods

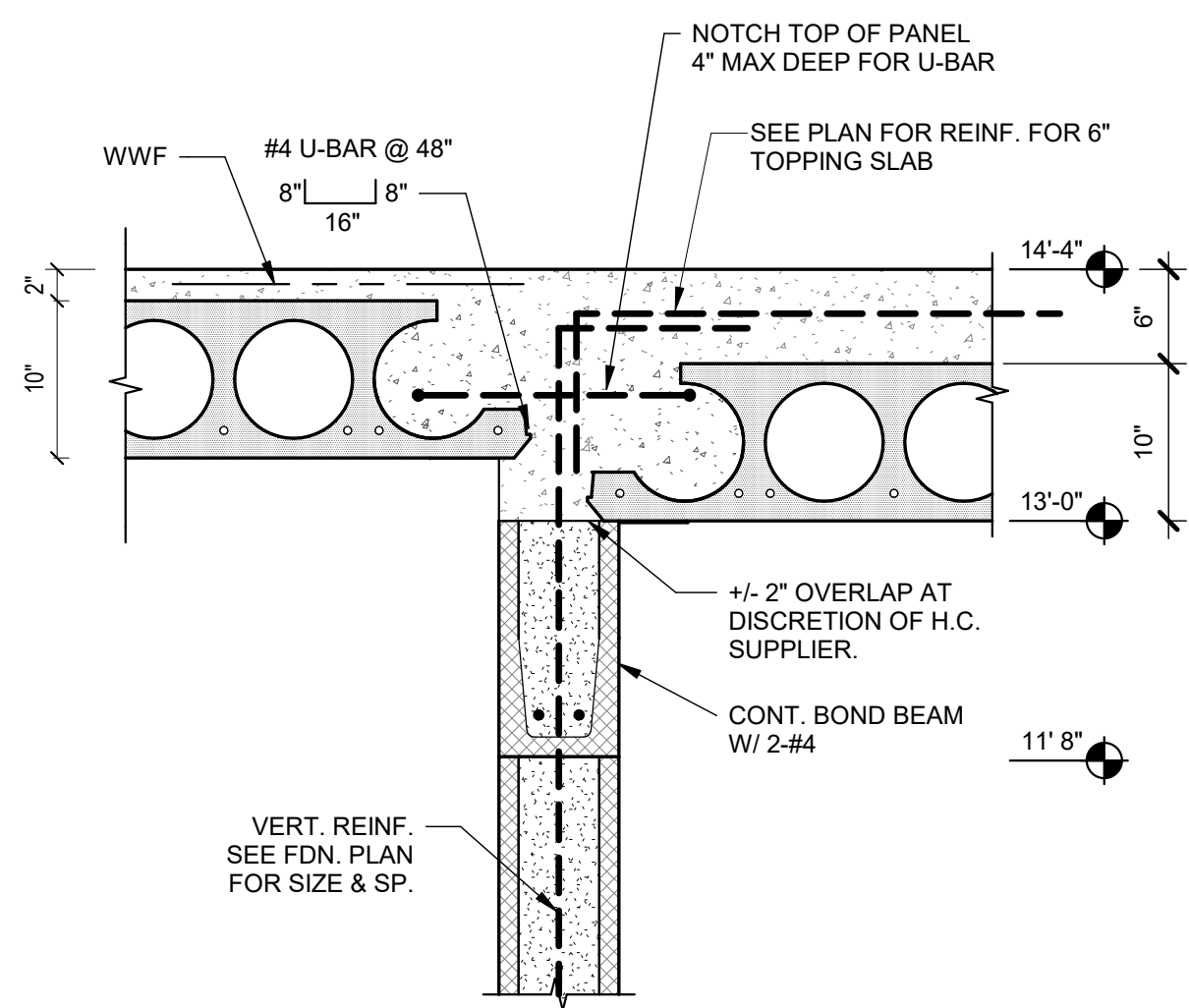
333 Fayetteville St., Ste. 225  
Raleigh, NC 27601  
P: 919.573.6350  
F: 919.573.6355  
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**sfla**  
ARCHITECTS

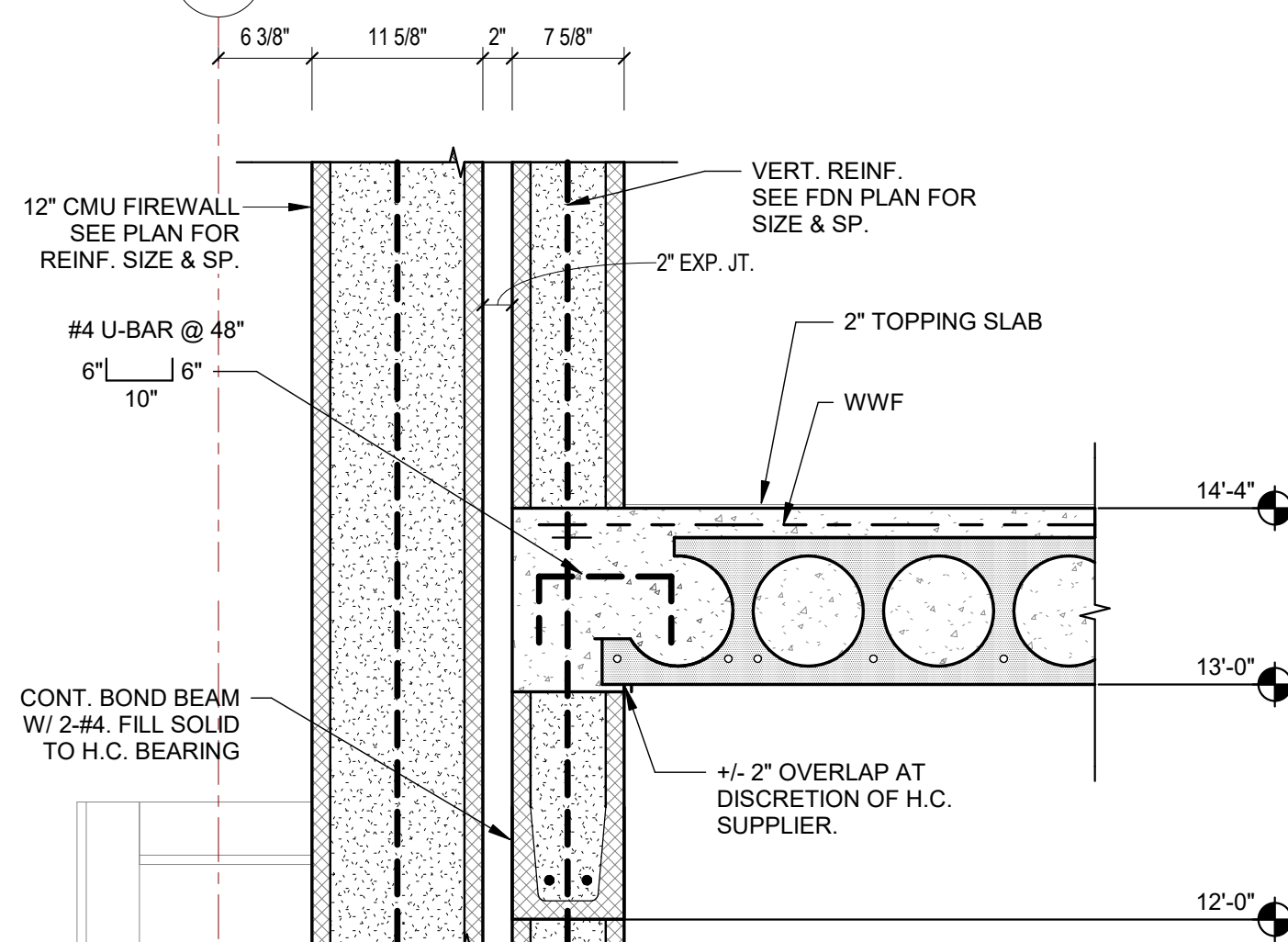
PROFESSIONAL  
ENGINEER  
037412  
NORTH CAROLINA  
J. MOHAWY K. WILSON  
6-12-24

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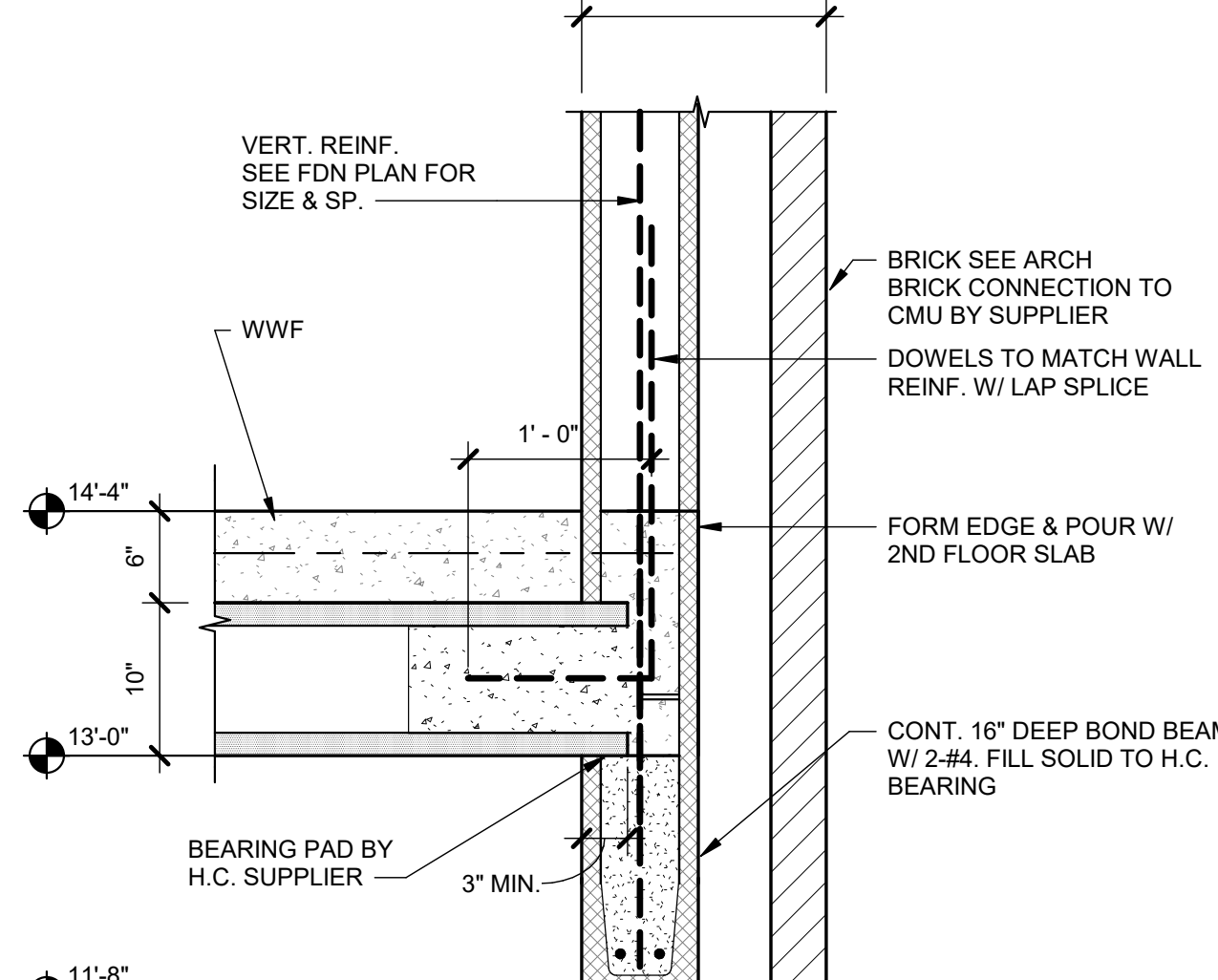




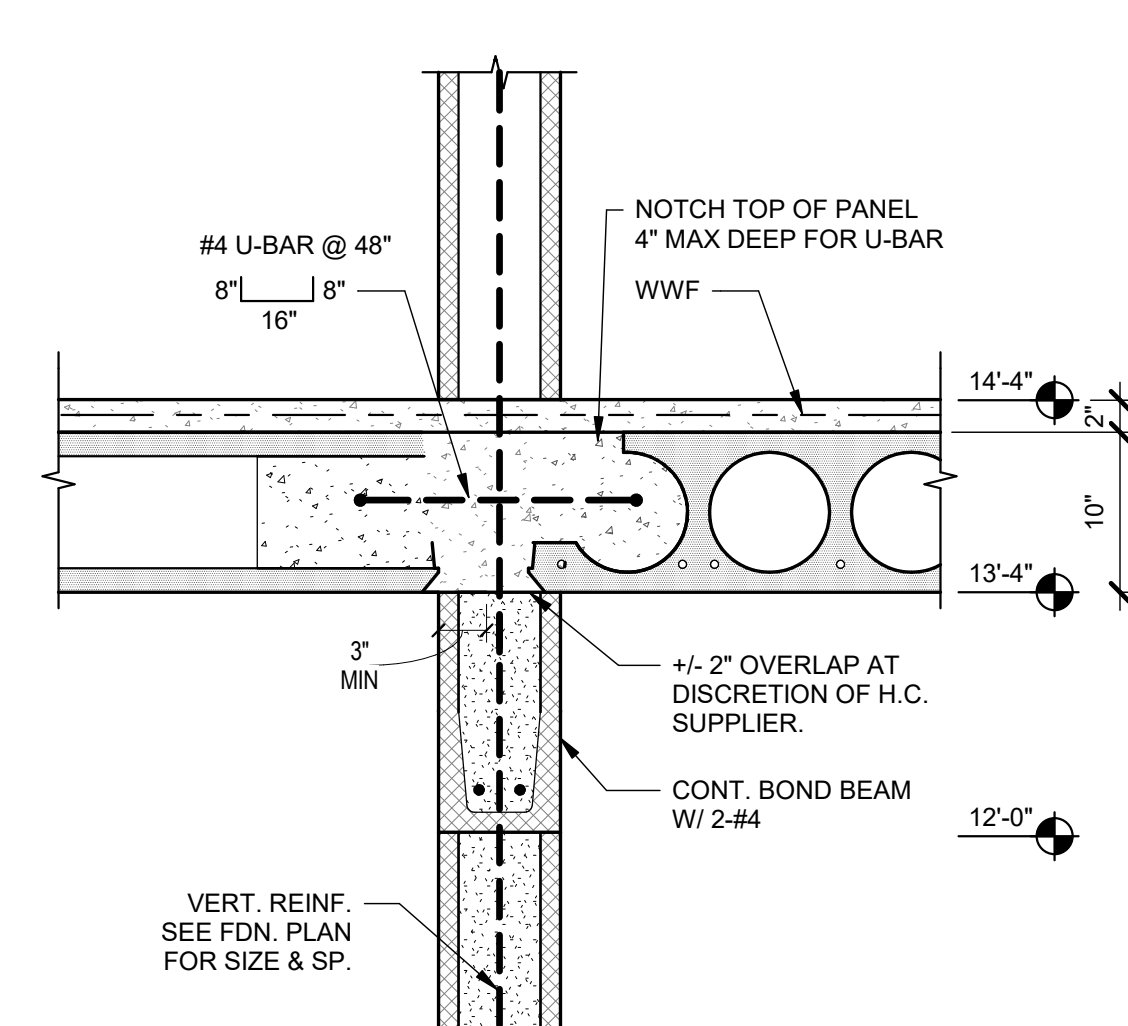
10 SECTION THRU SHEAR WALL  
S-404 SCALE: 1" = 1'-0"



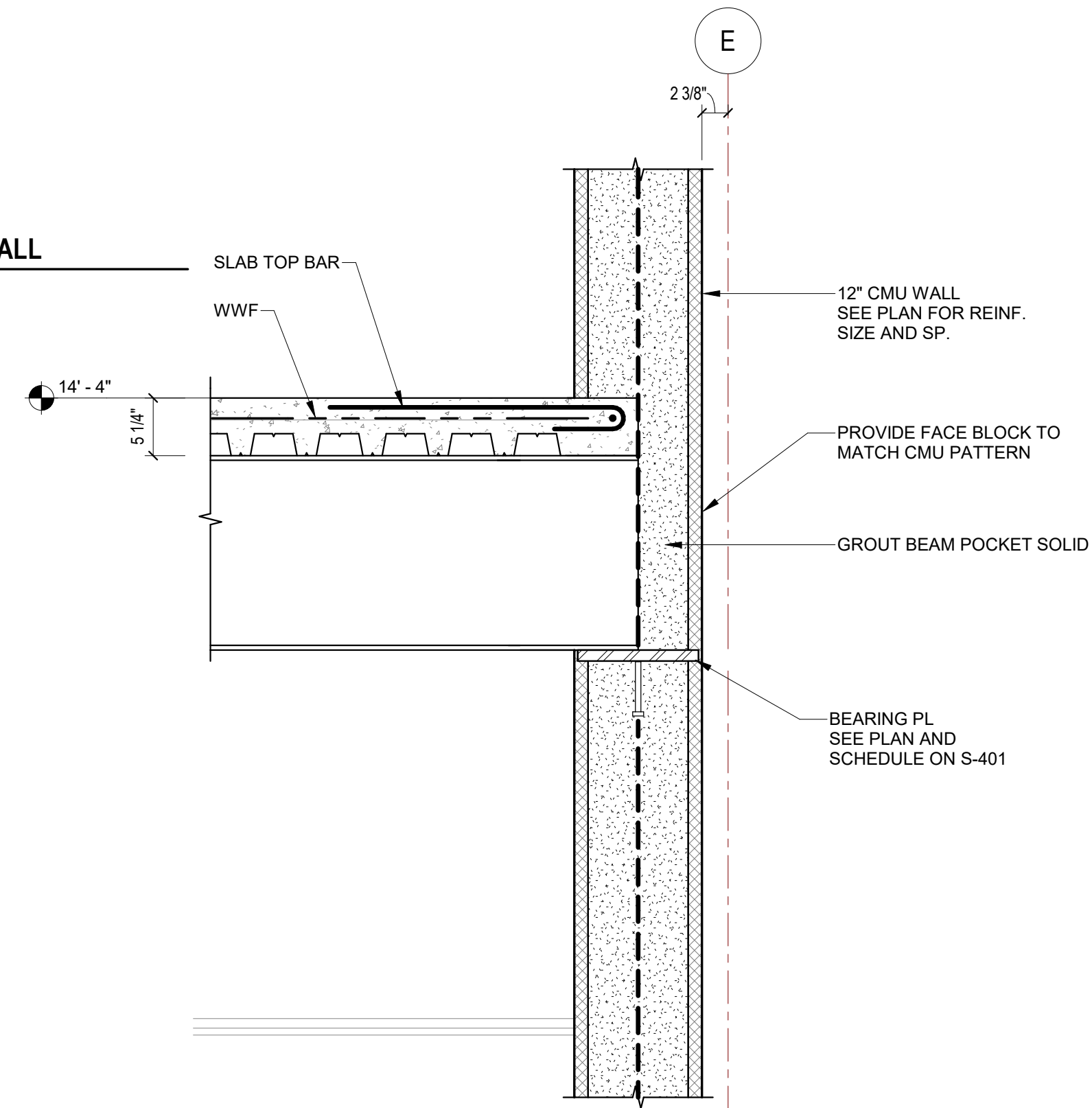
9 SECTION THRU FIREWALL  
S-404 SCALE: 1" = 1'-0"



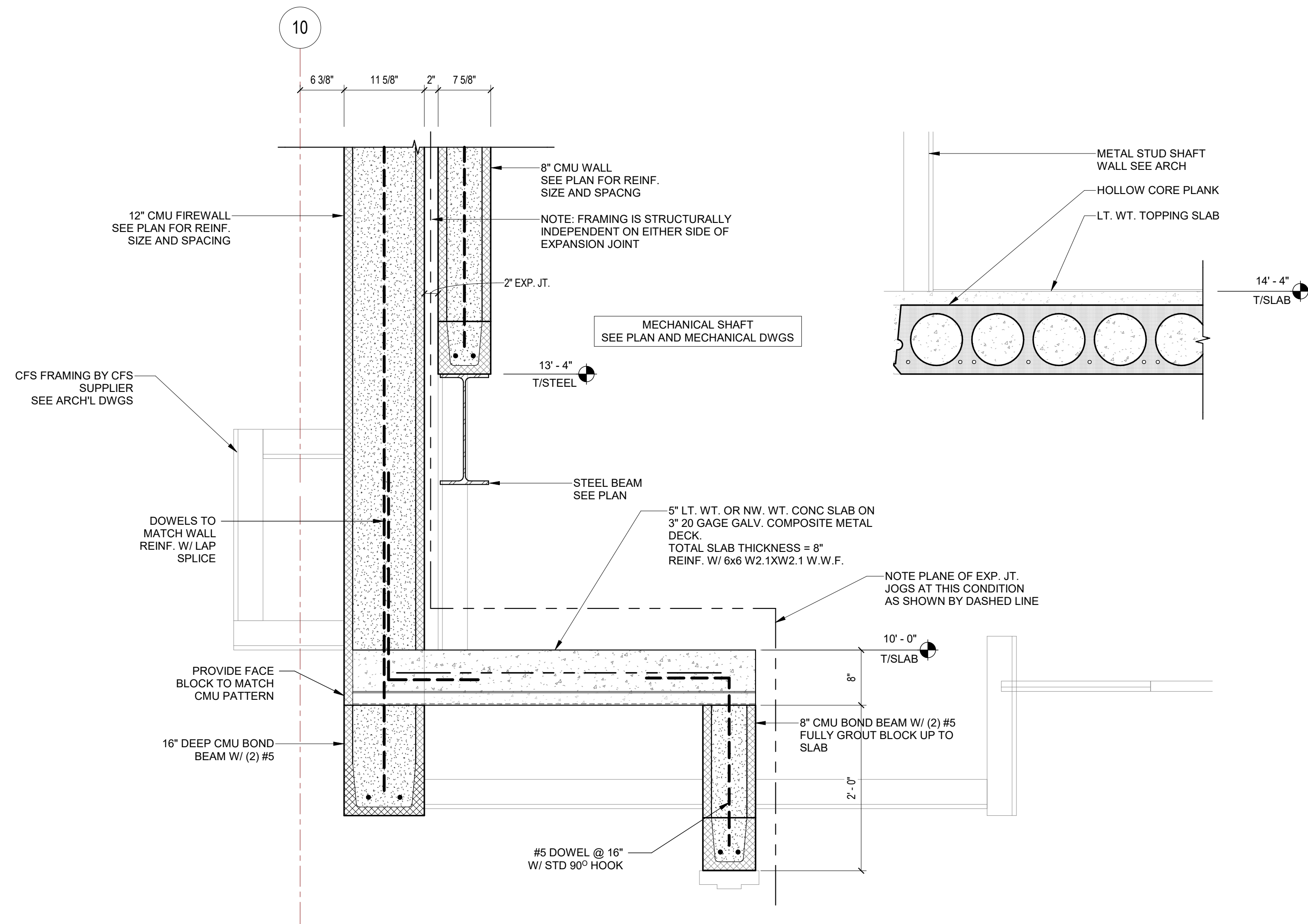
11 SECTION THRU FLOOR  
S-404 SCALE: 1" = 1'-0"



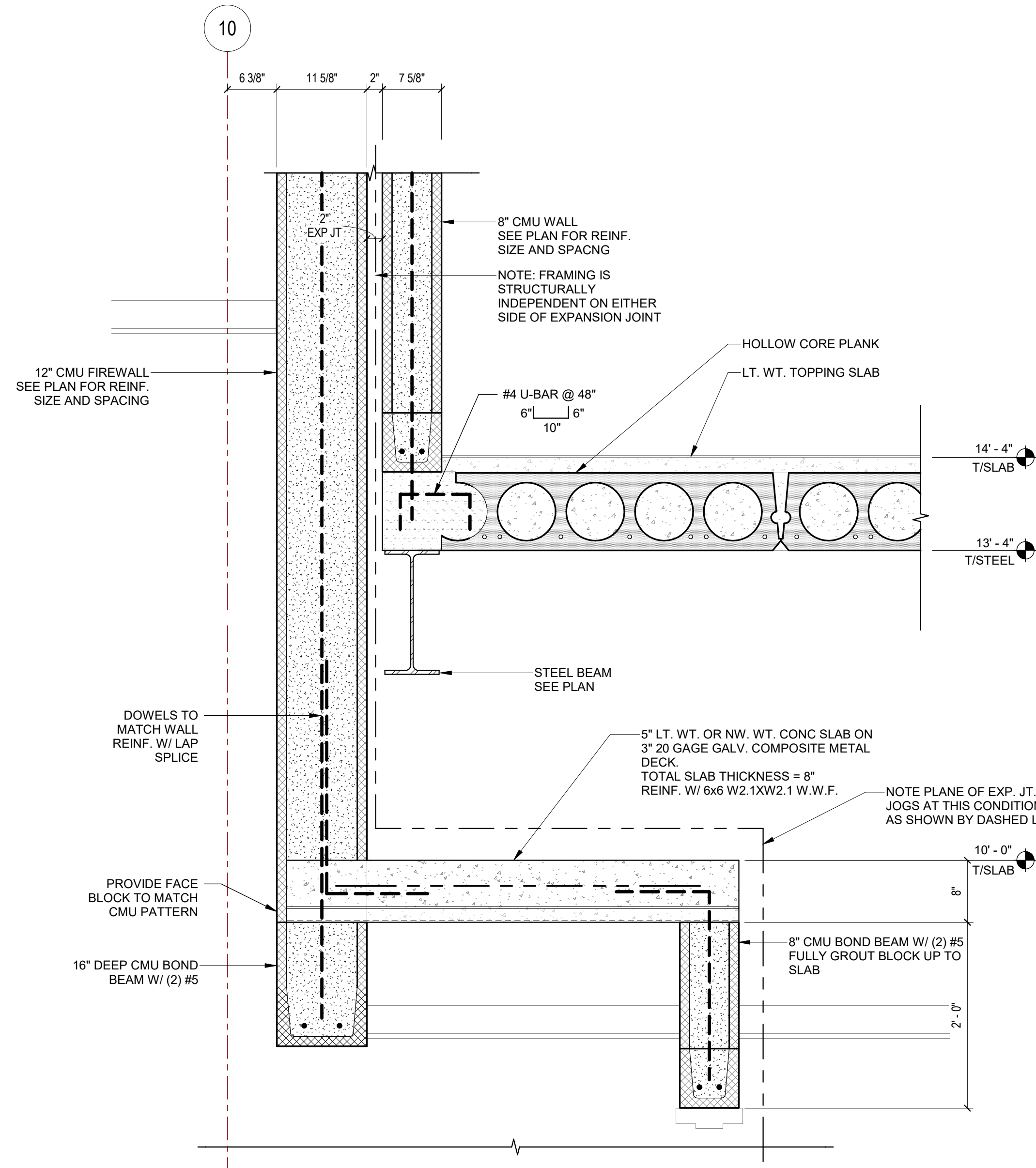
12 SECTION THRU SHEAR WALL  
S-404 SCALE: 1" = 1'-0"



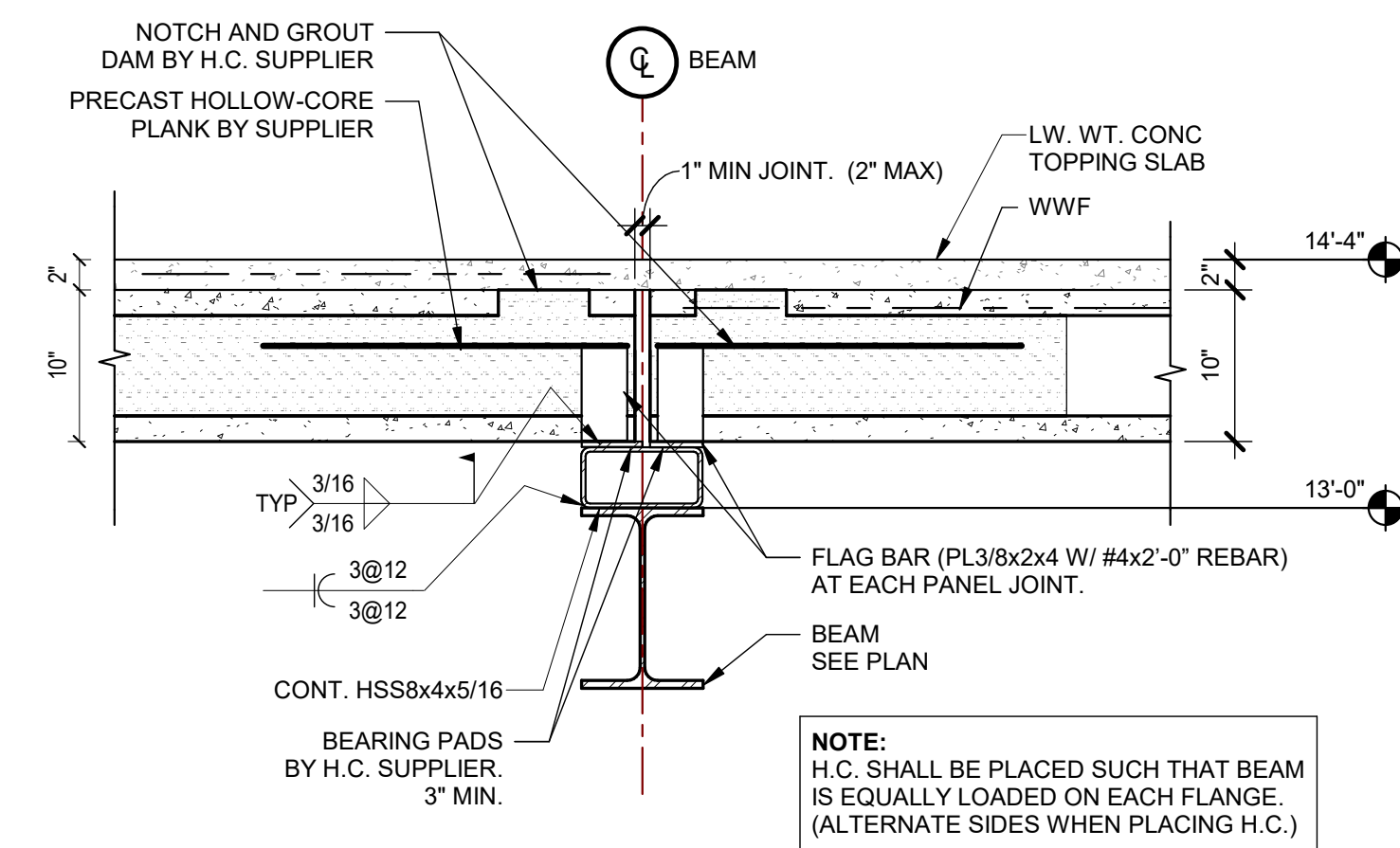
8 SECTION THRU MECHANICAL MEZZANINE  
S-404 SCALE: 1" = 1'-0"



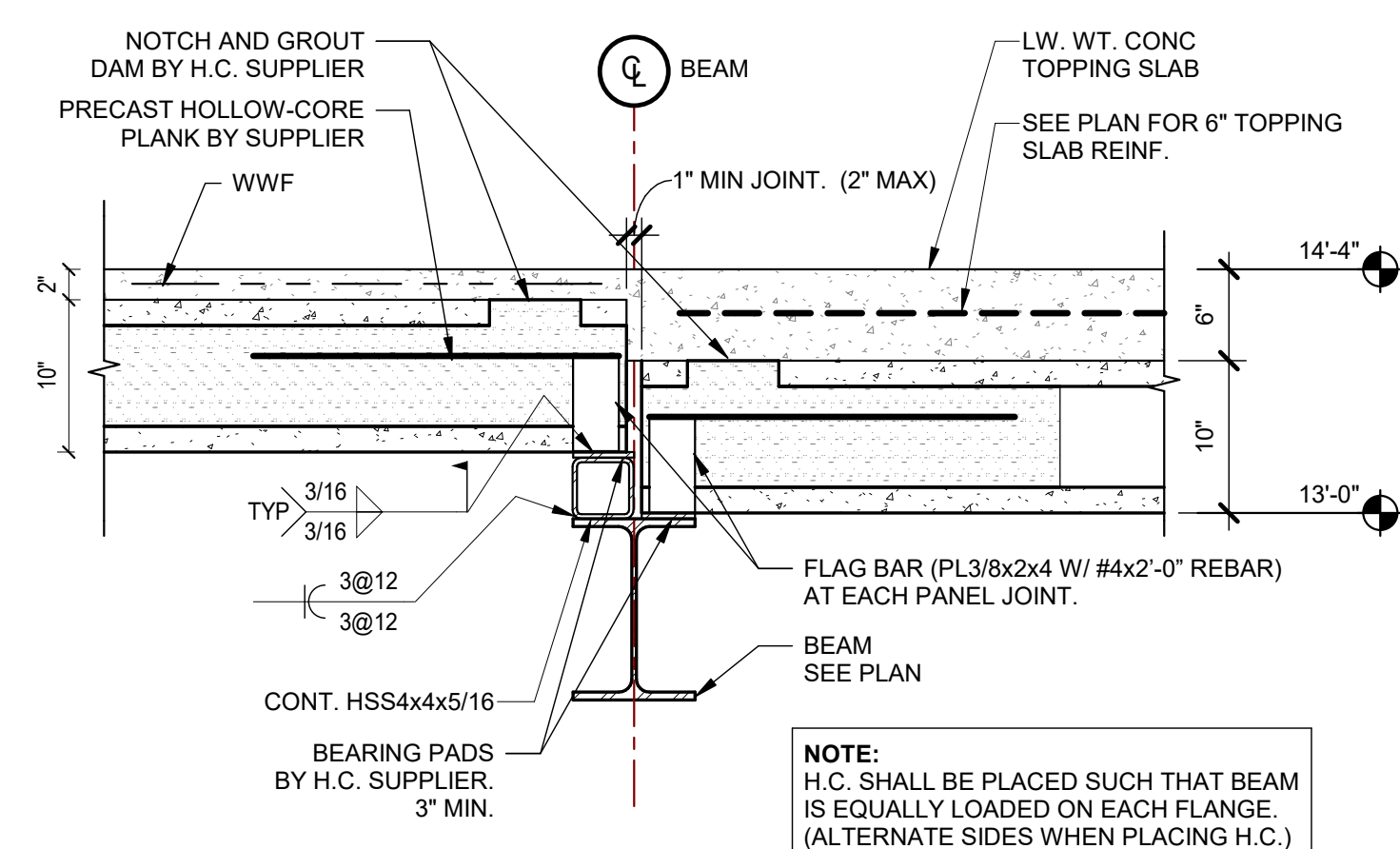
7 SECTION THRU FIREWALL ALCOVE  
S-404 SCALE: 1" = 1'-0"



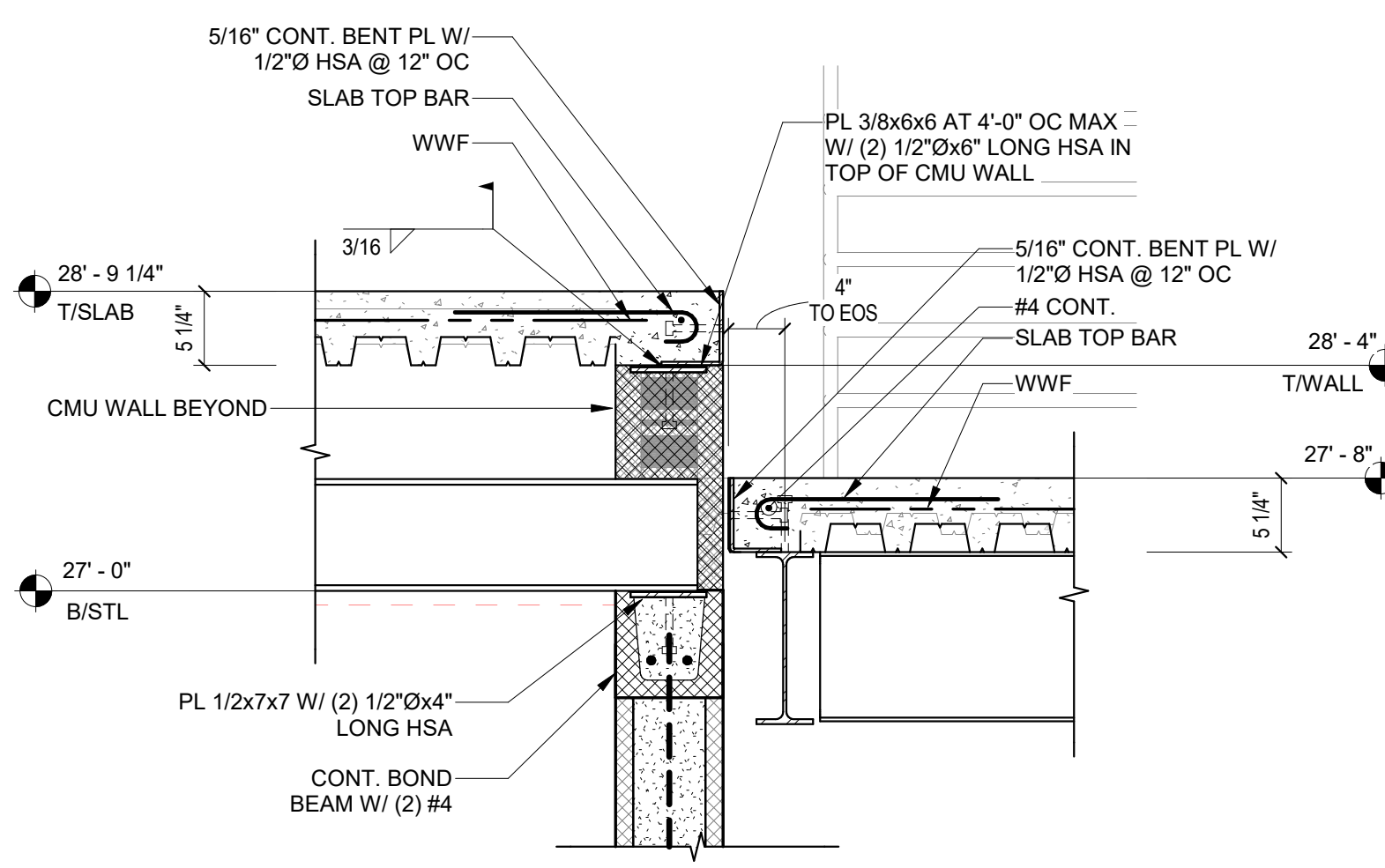
6 SECTION THRU FIREWALL ALCOVE  
S-404 SCALE: 1" = 1'-0"



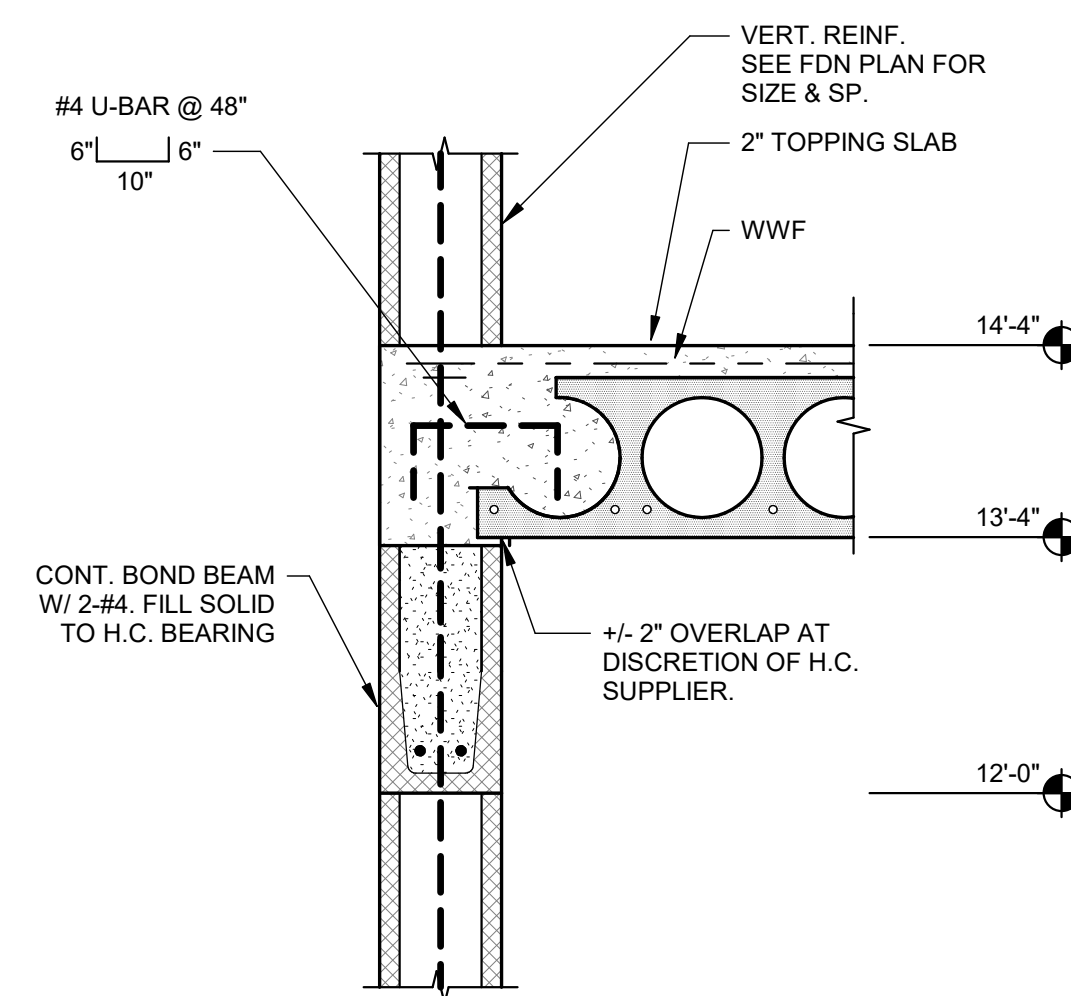
5 HOLLOW-CORE DETAIL  
S-404 SCALE: 1" = 1'-0"



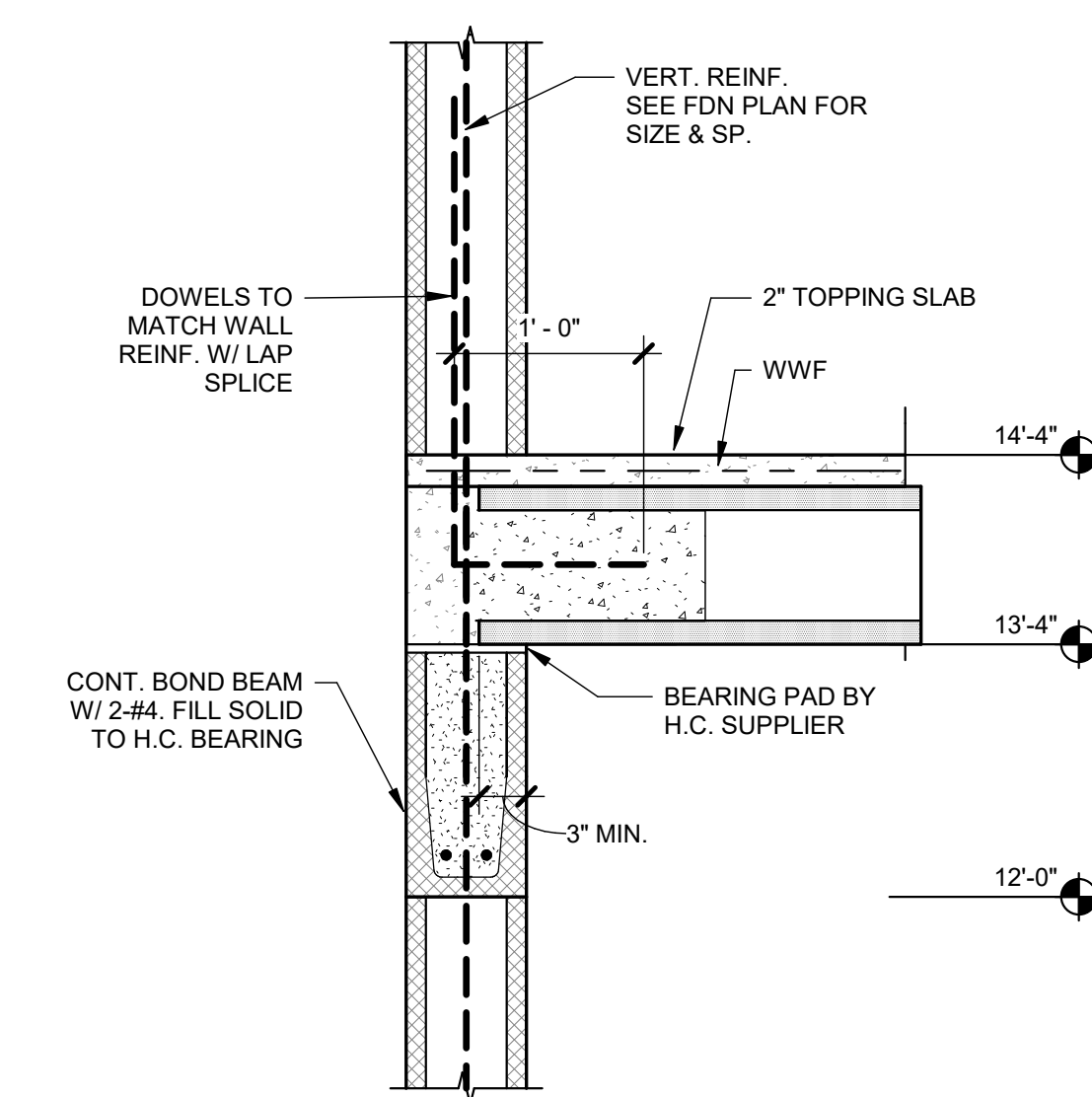
4 HOLLOW-CORE DETAIL  
S-404 SCALE: 1" = 1'-0"



3 SECTION THRU ELEVATOR SHAFT  
S-404 SCALE: 1" = 1'-0"



2 SECTION THRU ELEVATOR SHAFT  
S-404 SCALE: 1" = 1'-0"



1 SECTION THRU ELEVATOR SHAFT  
S-404 SCALE: 1" = 1'-0"

CONSTRUCTION  
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23.08.034		PROJECT #:
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FRAMING DETAILS



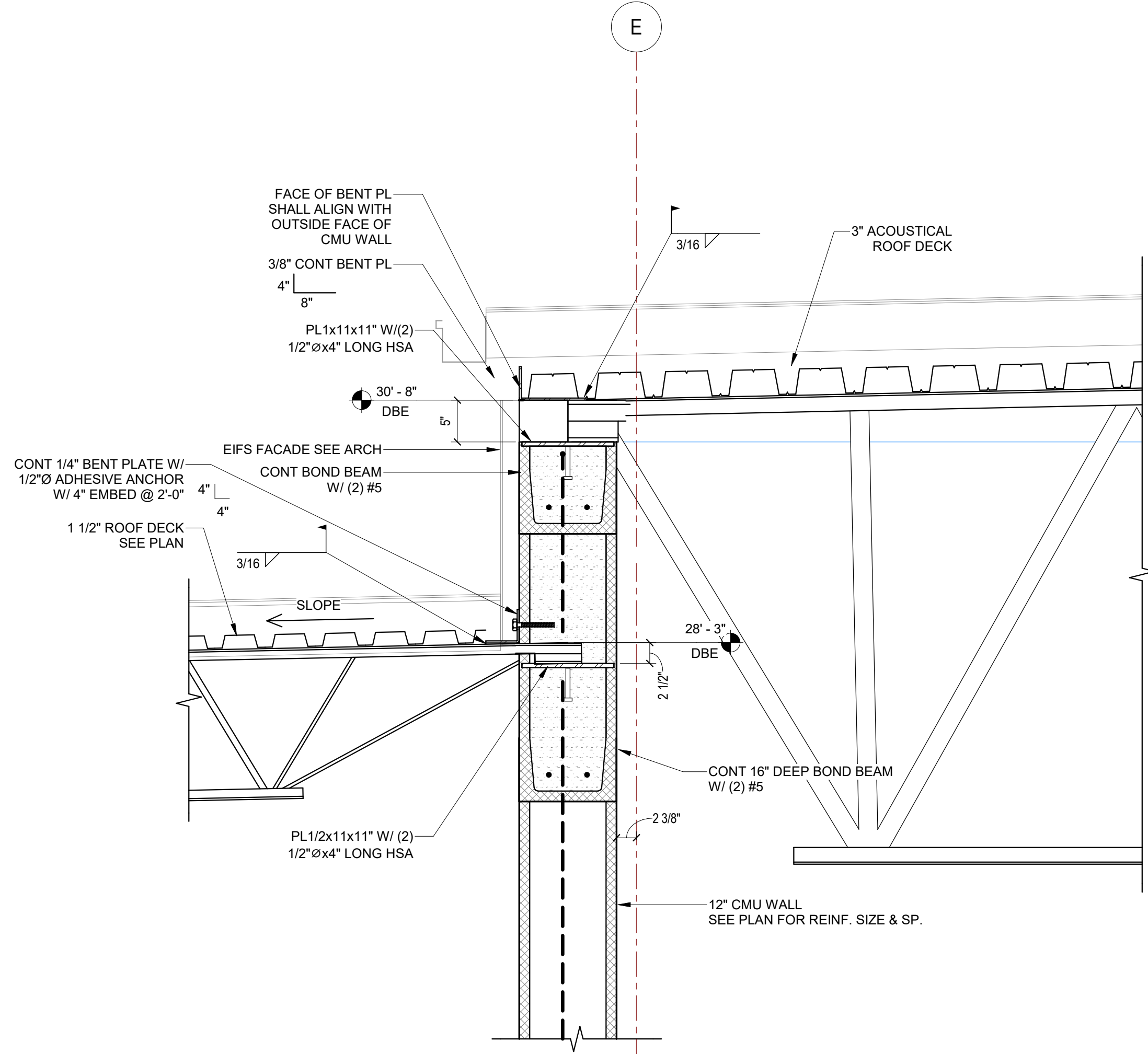




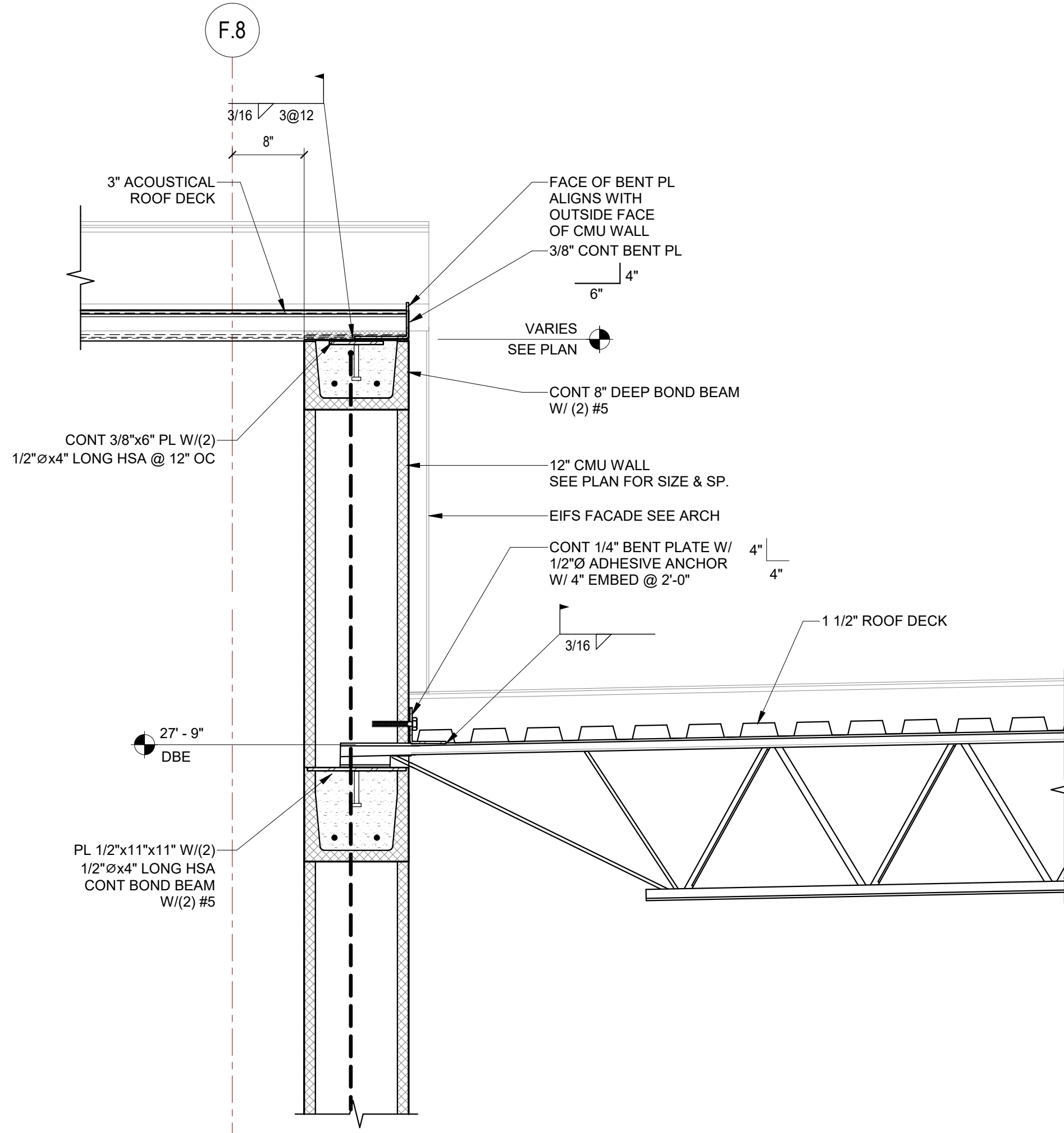




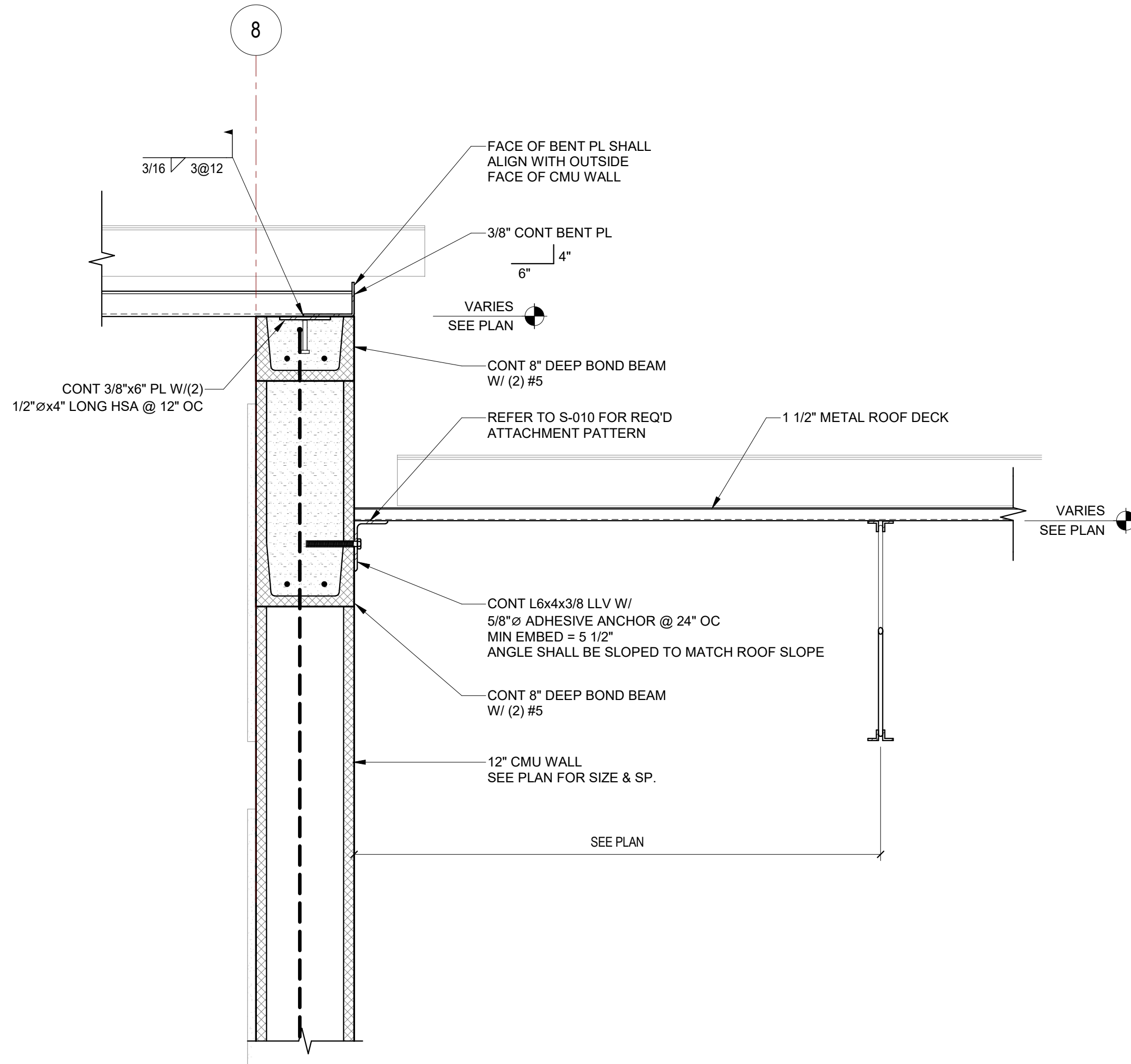
3 SECTION THRU ROOF  
S-407 SCALE: 1" = 1'-0"



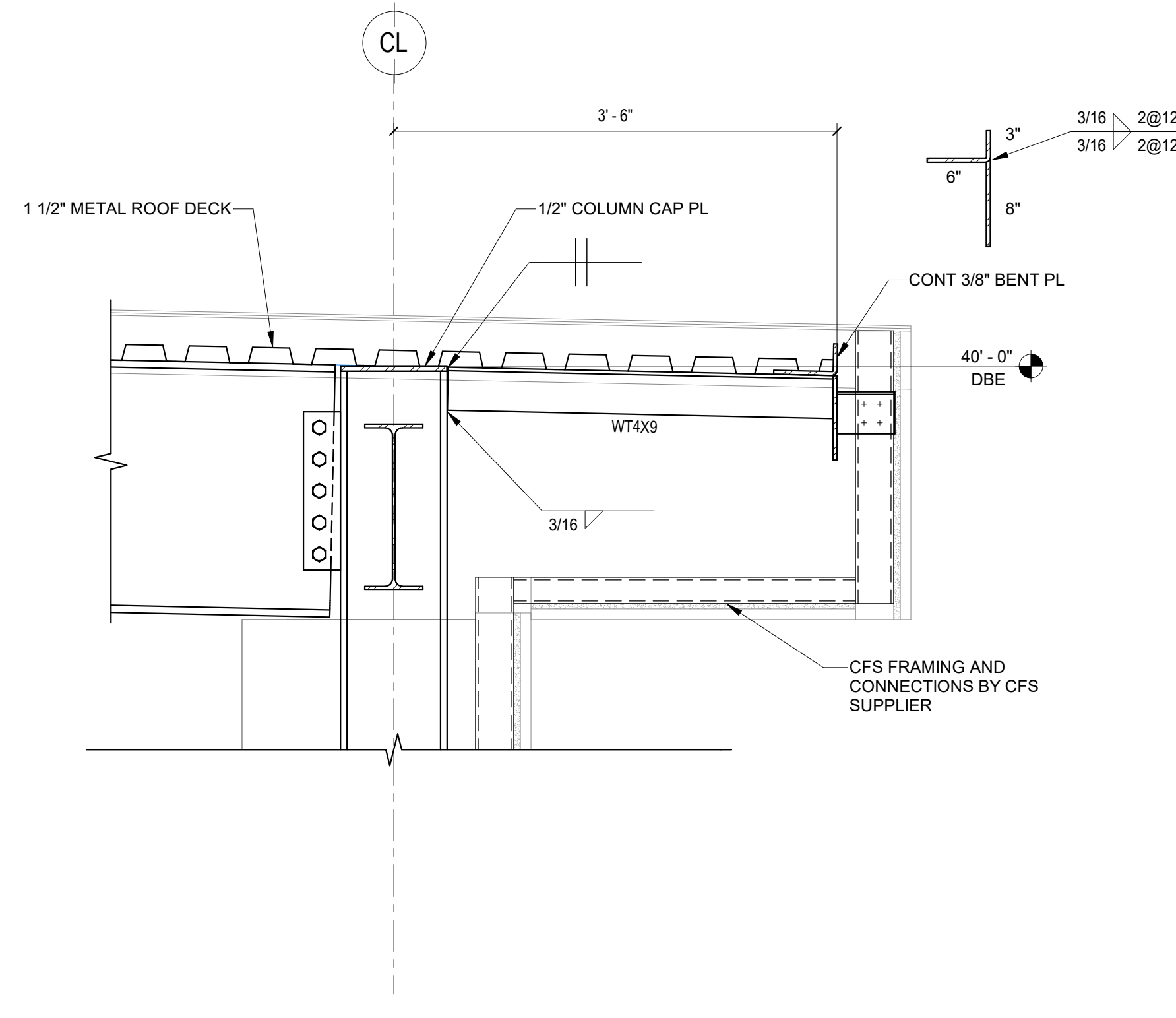
2 SECTION THRU ROOF  
S-407 SCALE: 1" = 1'-0"



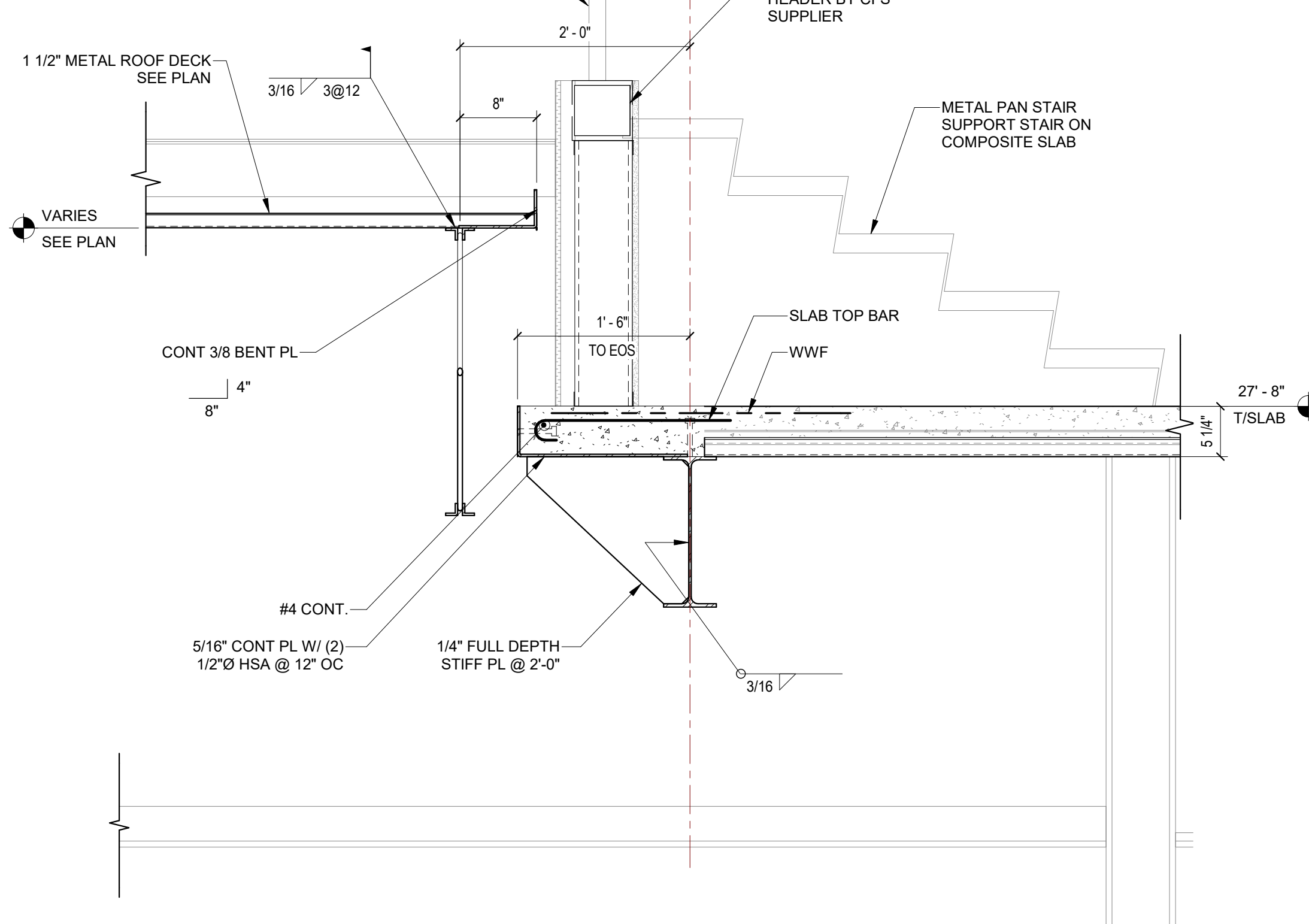
1 SECTION THRU ROOF  
S-407 SCALE: 1" = 1'-0"



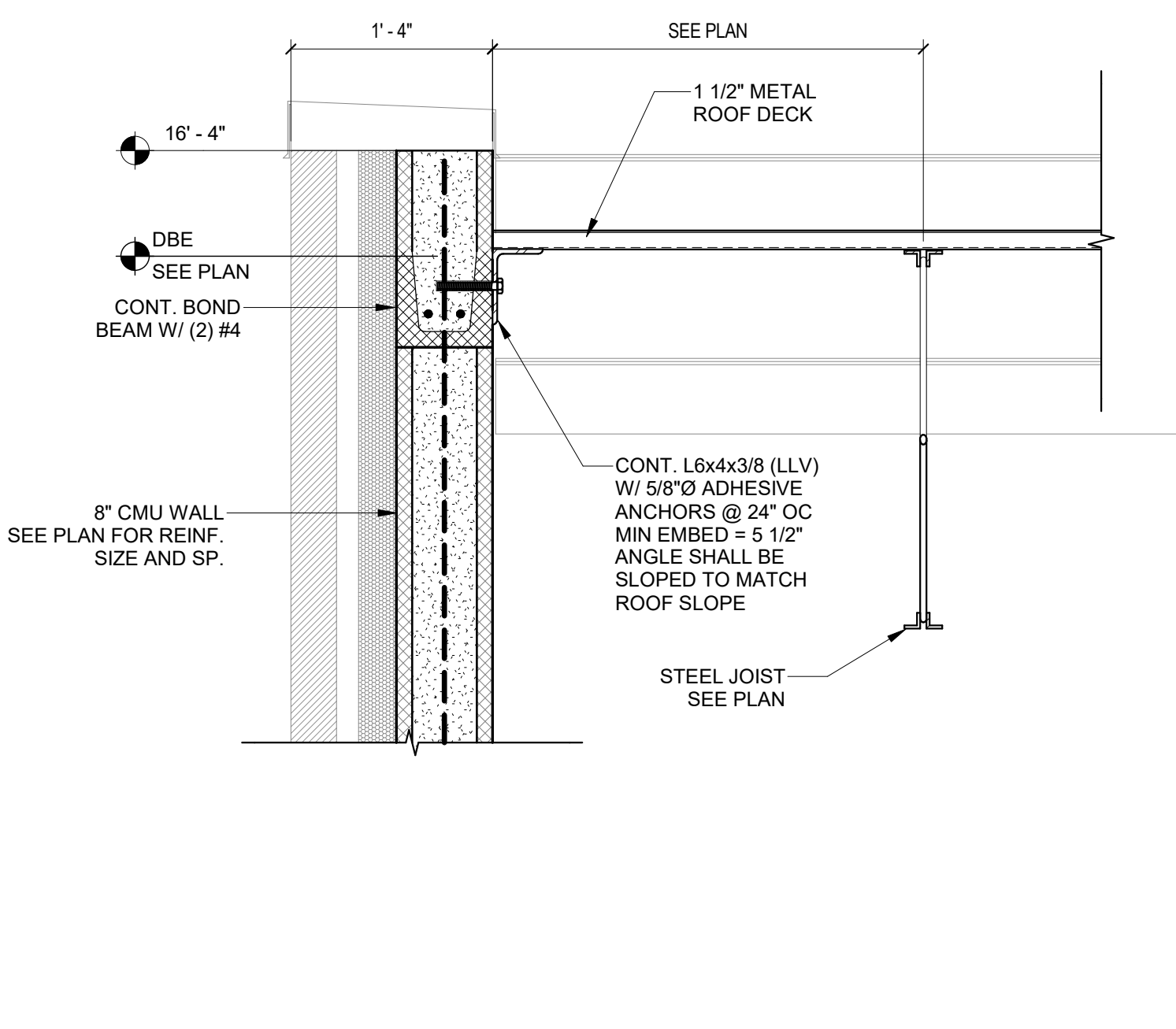
4 SECTION THRU CLERESTORY ROOF  
S-407 SCALE: 1" = 1'-0"



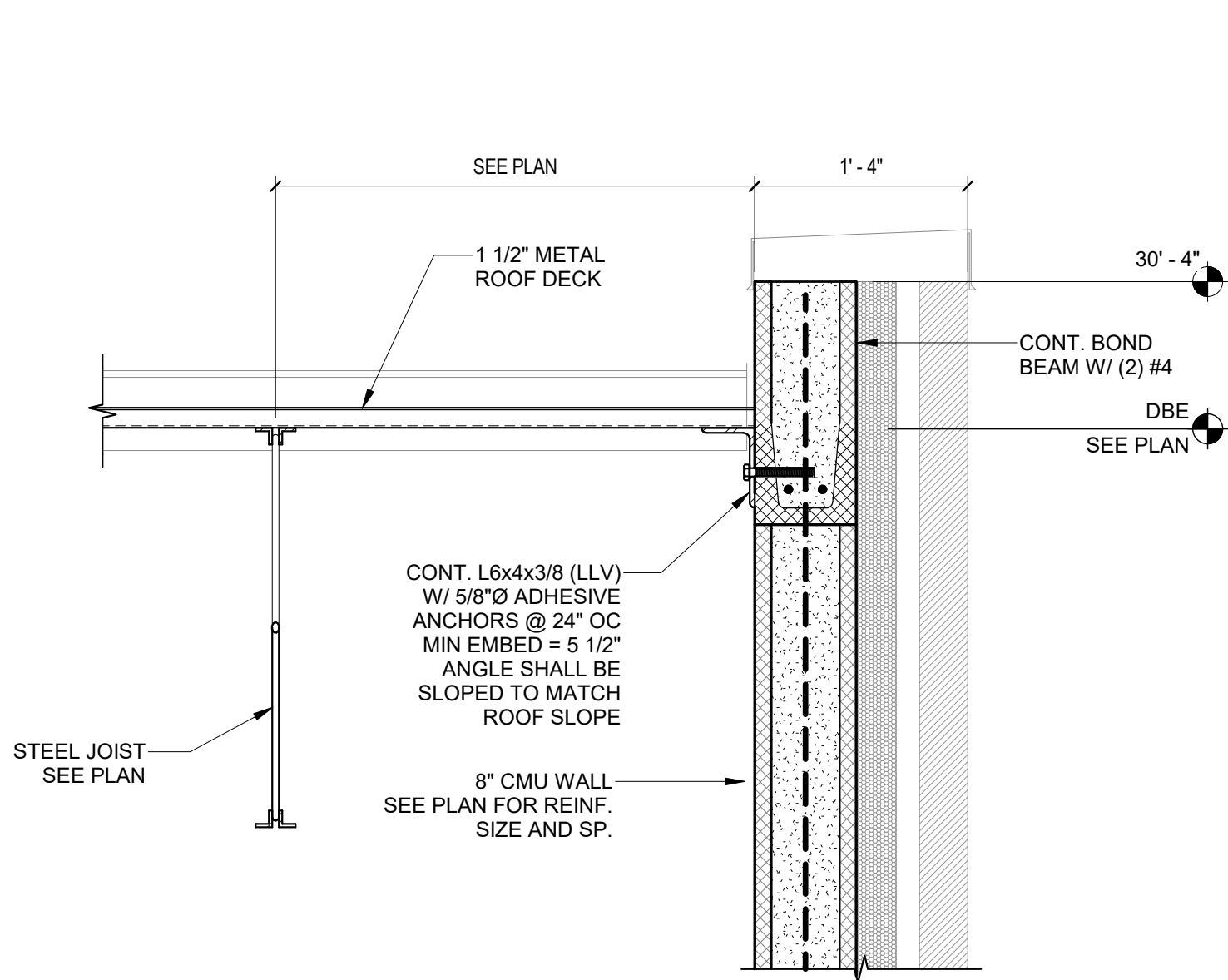
5 SECTION THRU ROOF  
S-407 SCALE: 1" = 1'-0"



6 SECTION THRU ROOF  
S-407 SCALE: 1" = 1'-0"

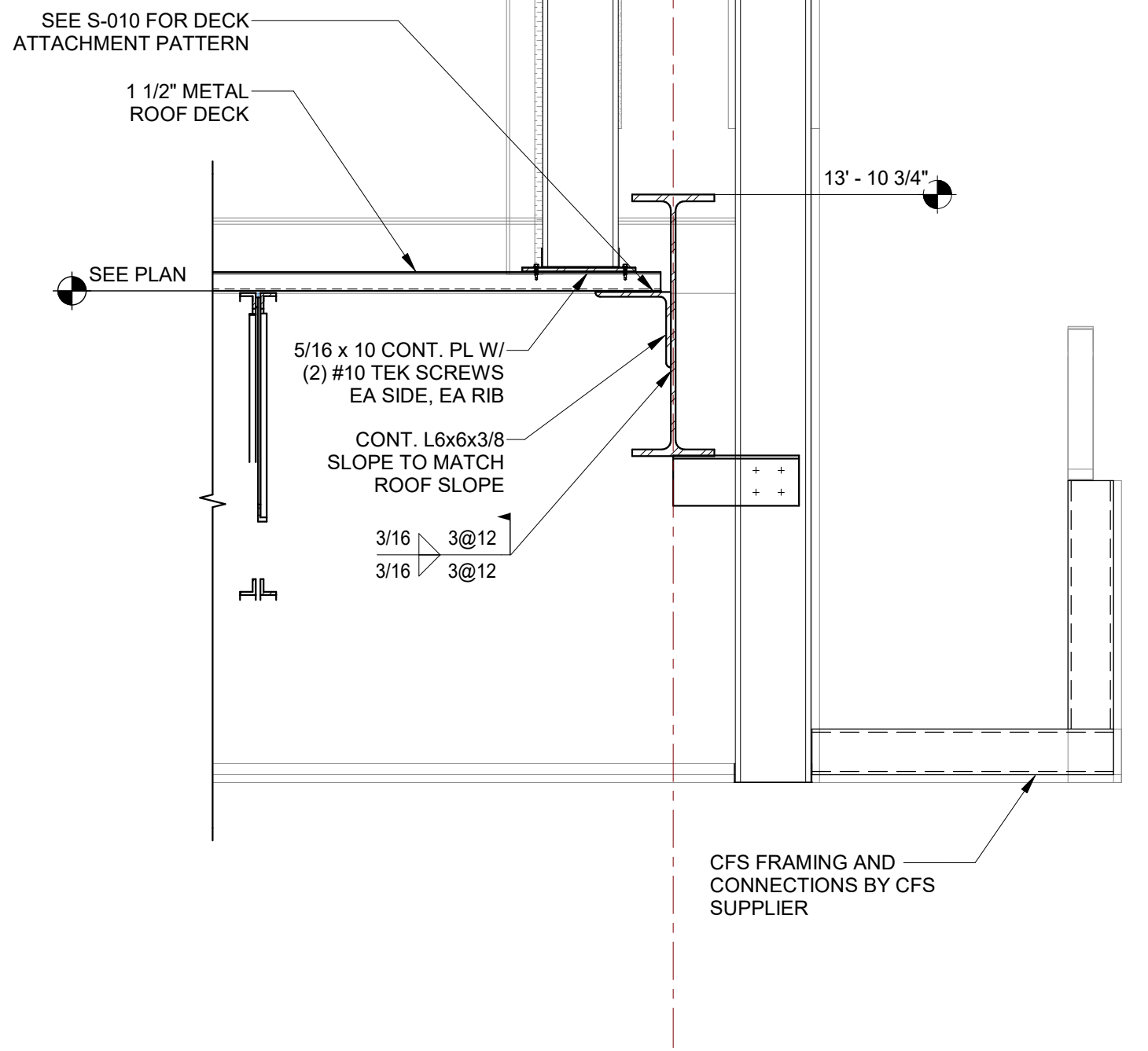


7 SECTION THRU ROOF  
S-407 SCALE: 1" = 1'-0"

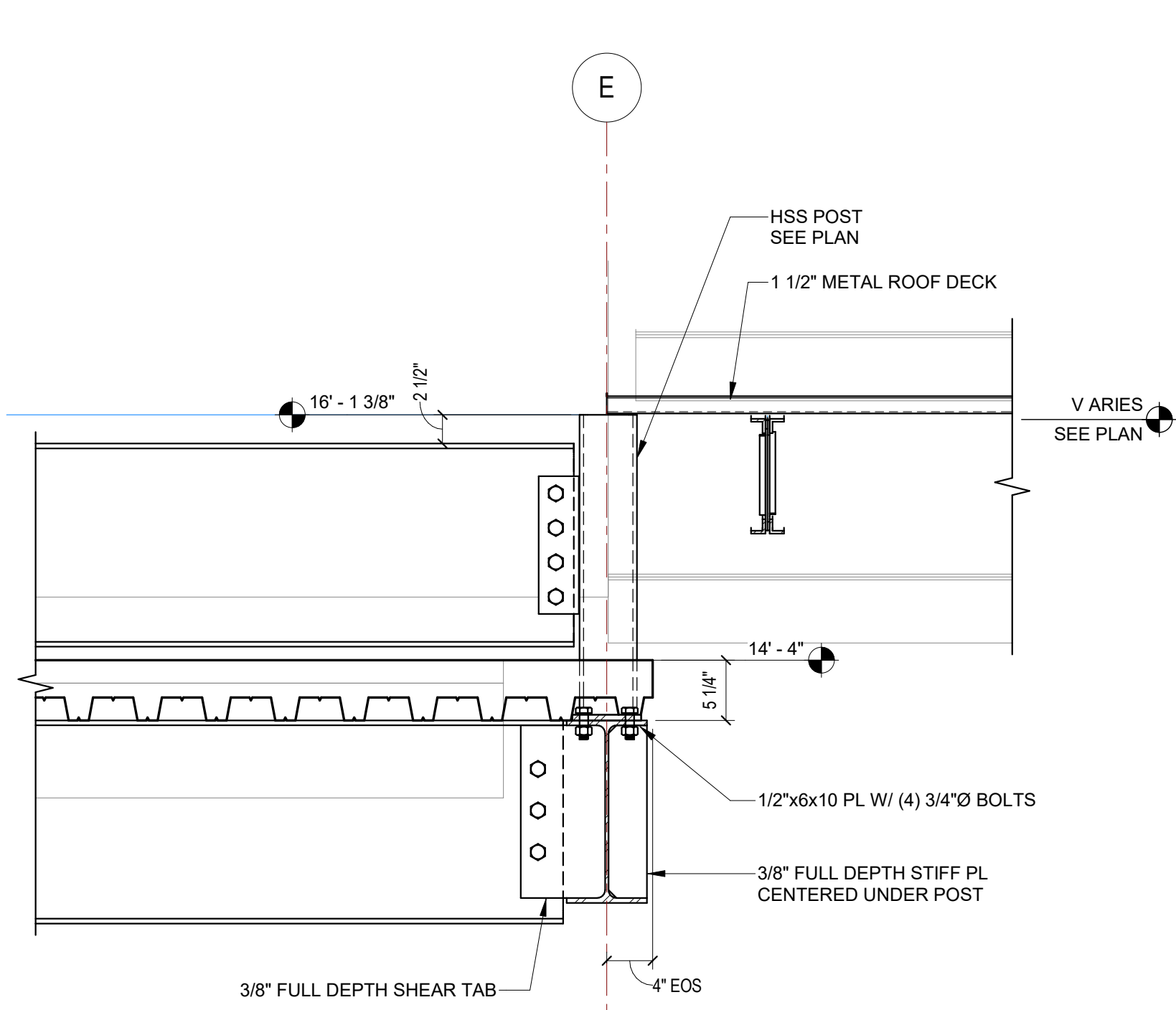




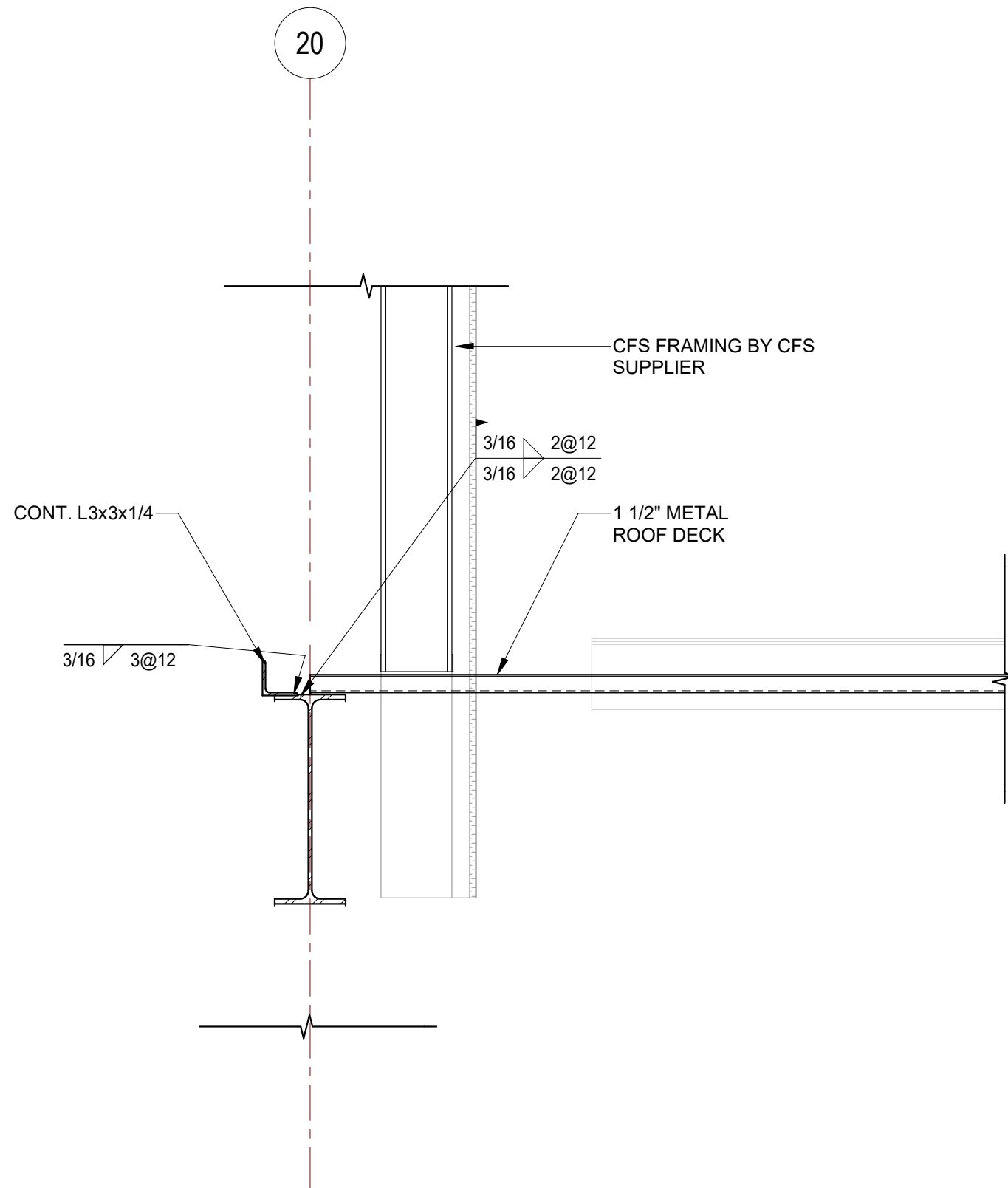
4 ROOF FRAMING DETAIL  
SCALE: 1" = 1'-0"



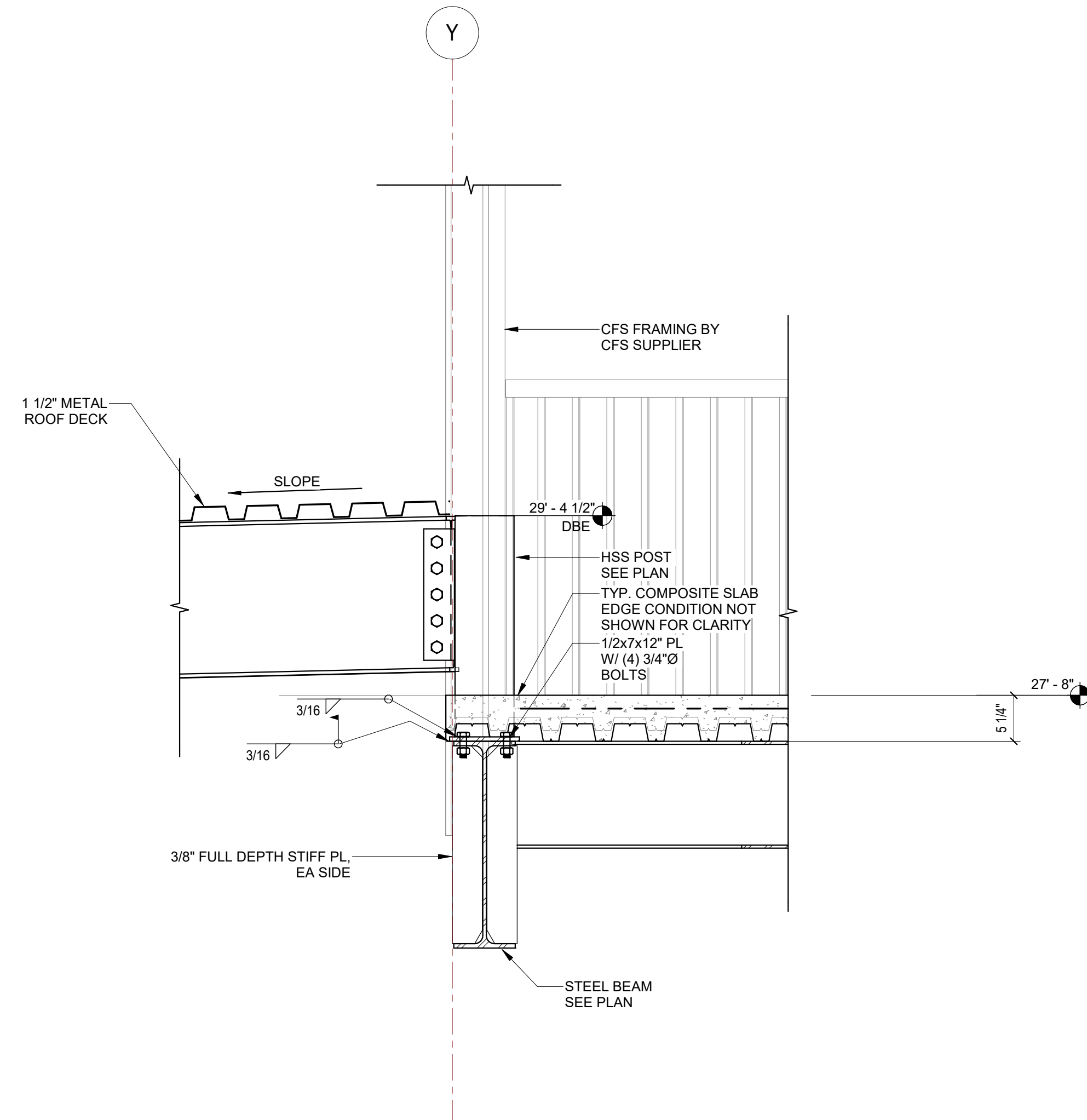
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SCALE: 1" = 1'-0"



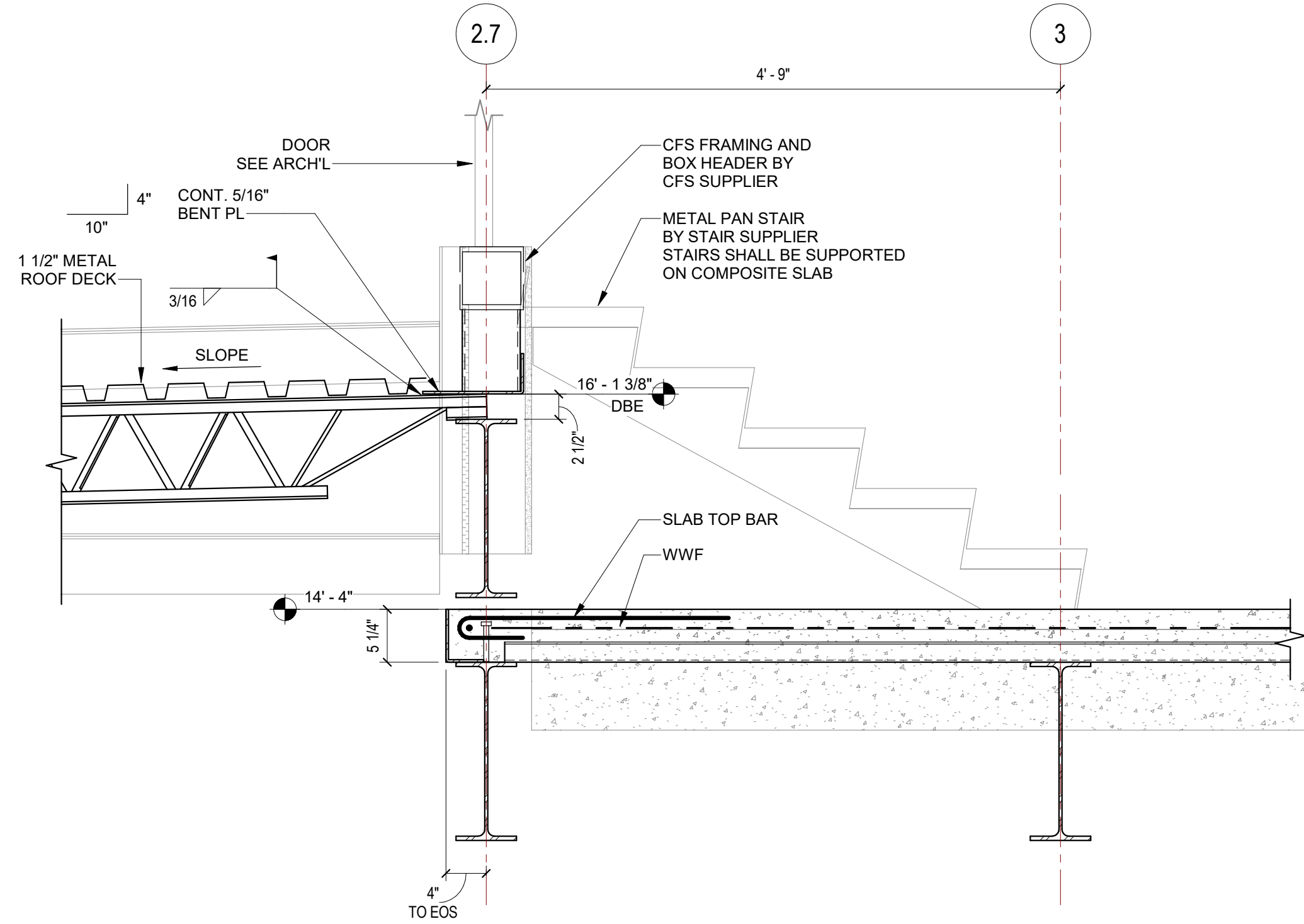
2 ROOF FRAMING DETAIL  
SCALE: 1" = 1'-0"



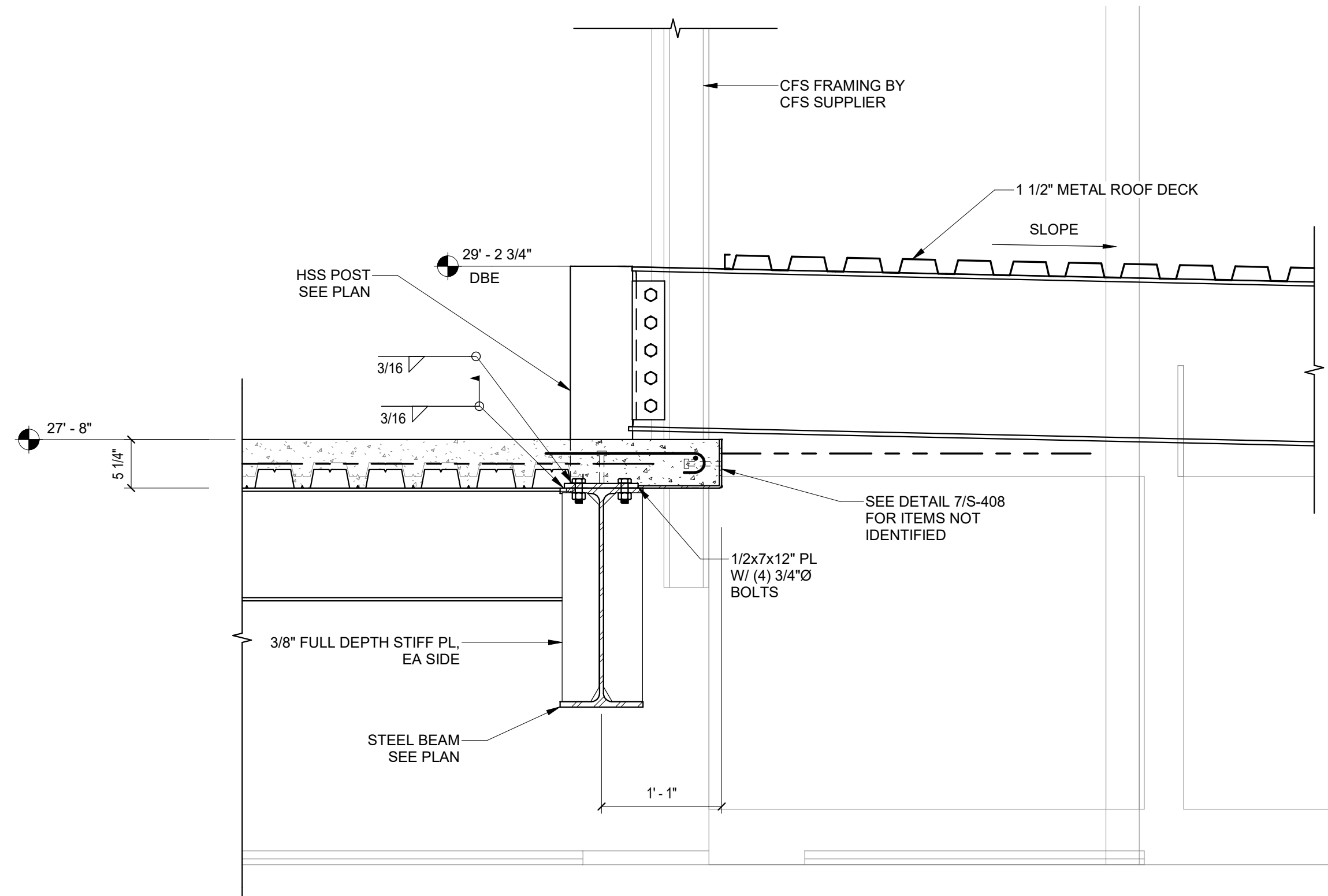
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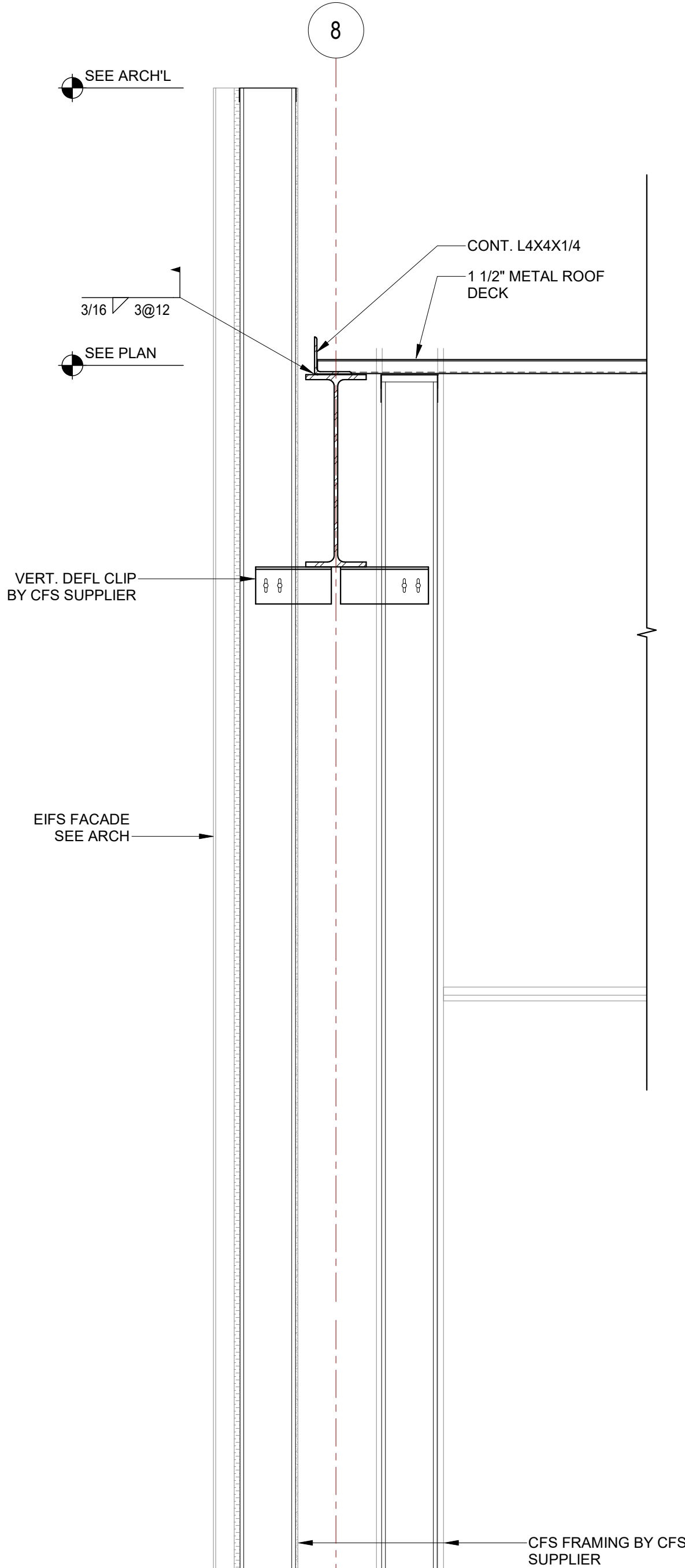
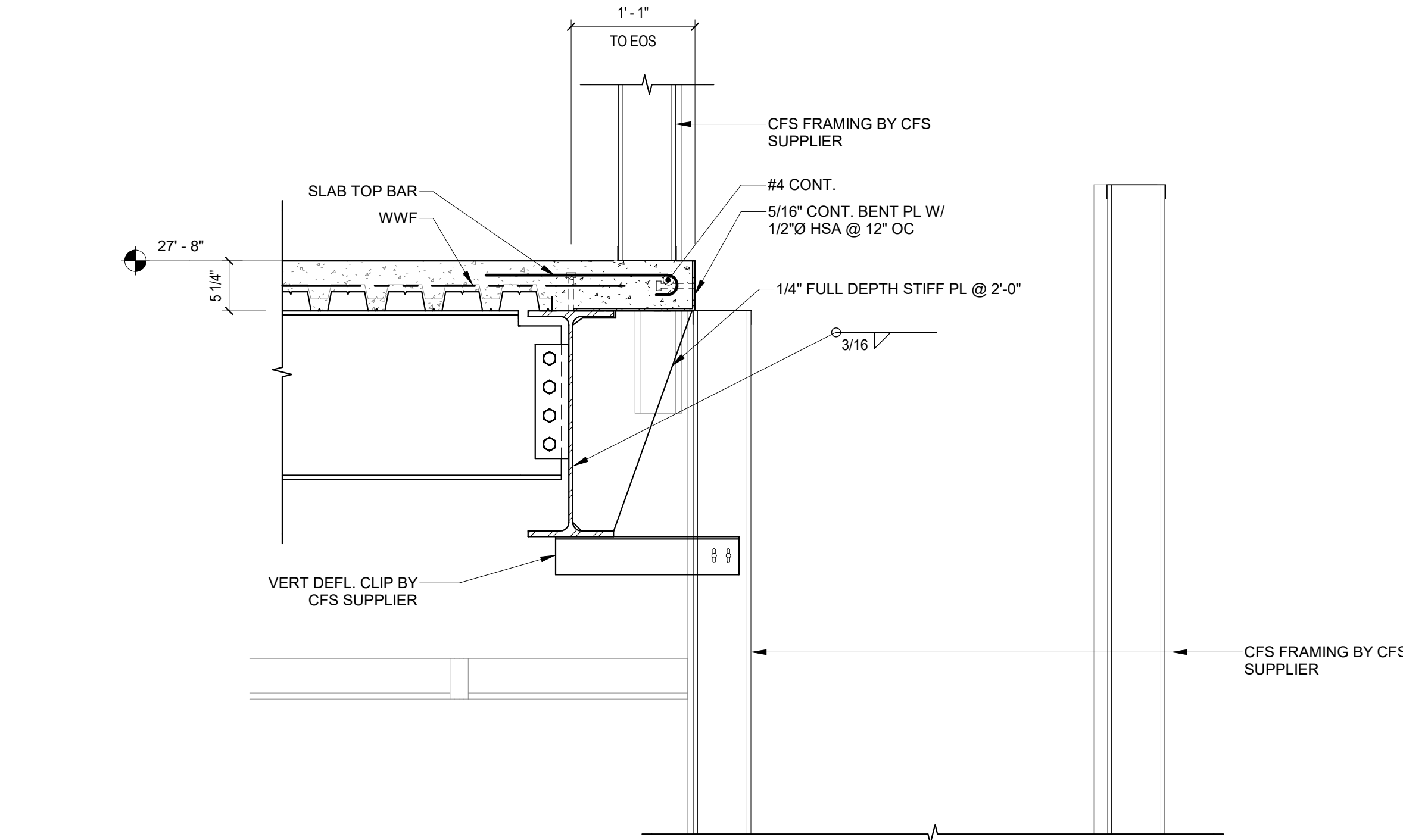
5 FRAMING DETAIL  
SCALE: 1" = 1'-0"



7 DETAIL  
SCALE: 1" = 1'-0"



6 ROOF FRAMING DETAIL  
SCALE: 1" = 1'-0"



PAMLICO COUNTY  
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5	TH	CHECKED BY:

ROOF FRAMING  
DETAILS

S-408

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333 Fayetteville St., Ste. 225  
Raleigh, NC 27601  
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F: 919.573.6355  
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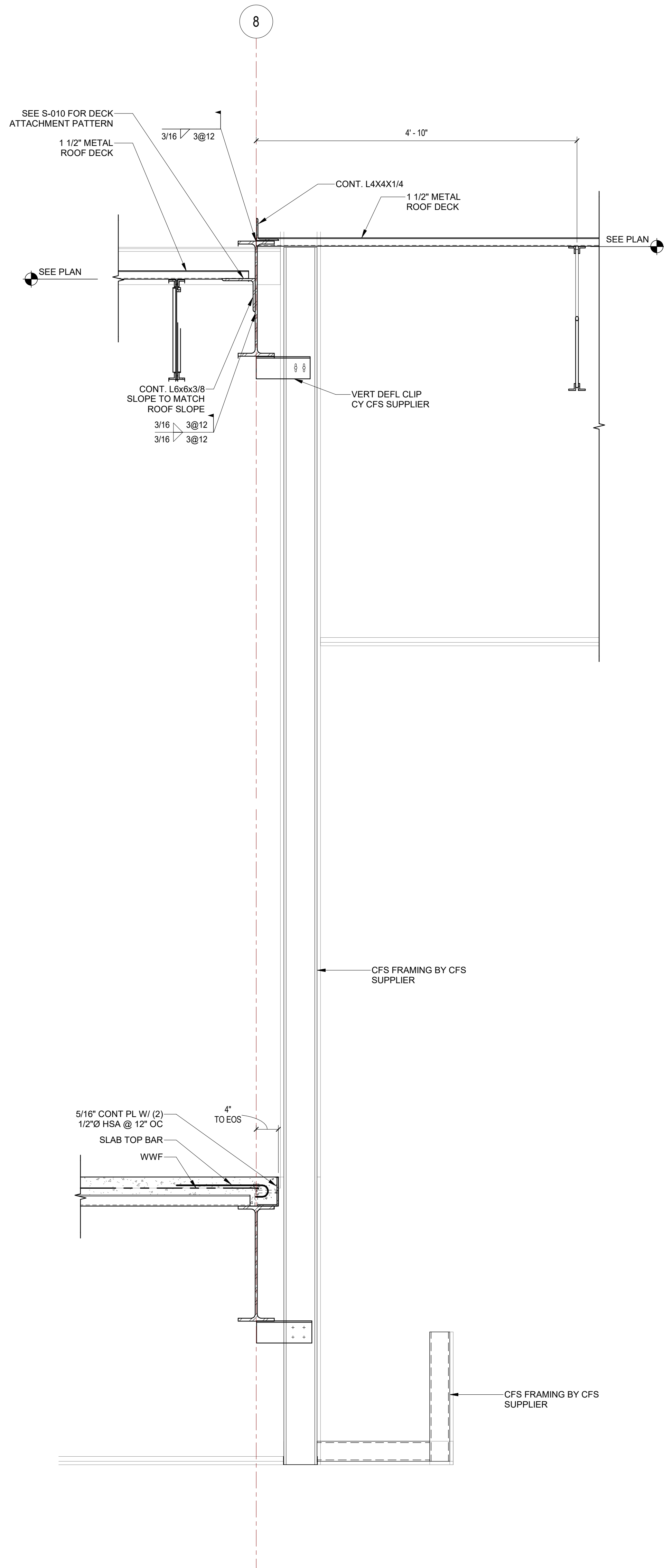
sfl+a  
ARCHITECTS

PROFESSIONAL  
ENGINEER  
037412  
NORTH CAROLINA  
J. MONTY K. WILSON  
6-12-24

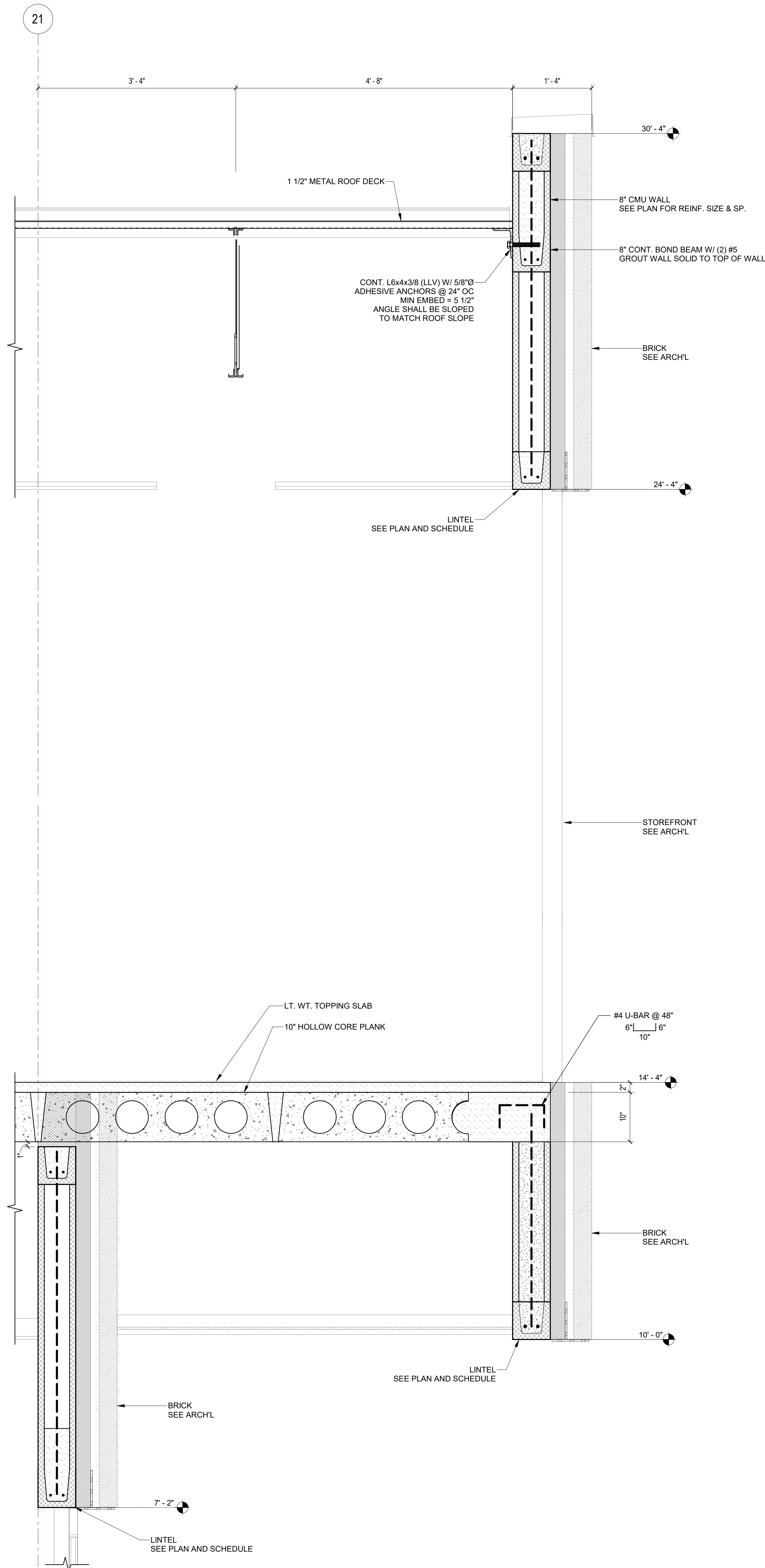
CONSTRUCTION  
DOCUMENTS



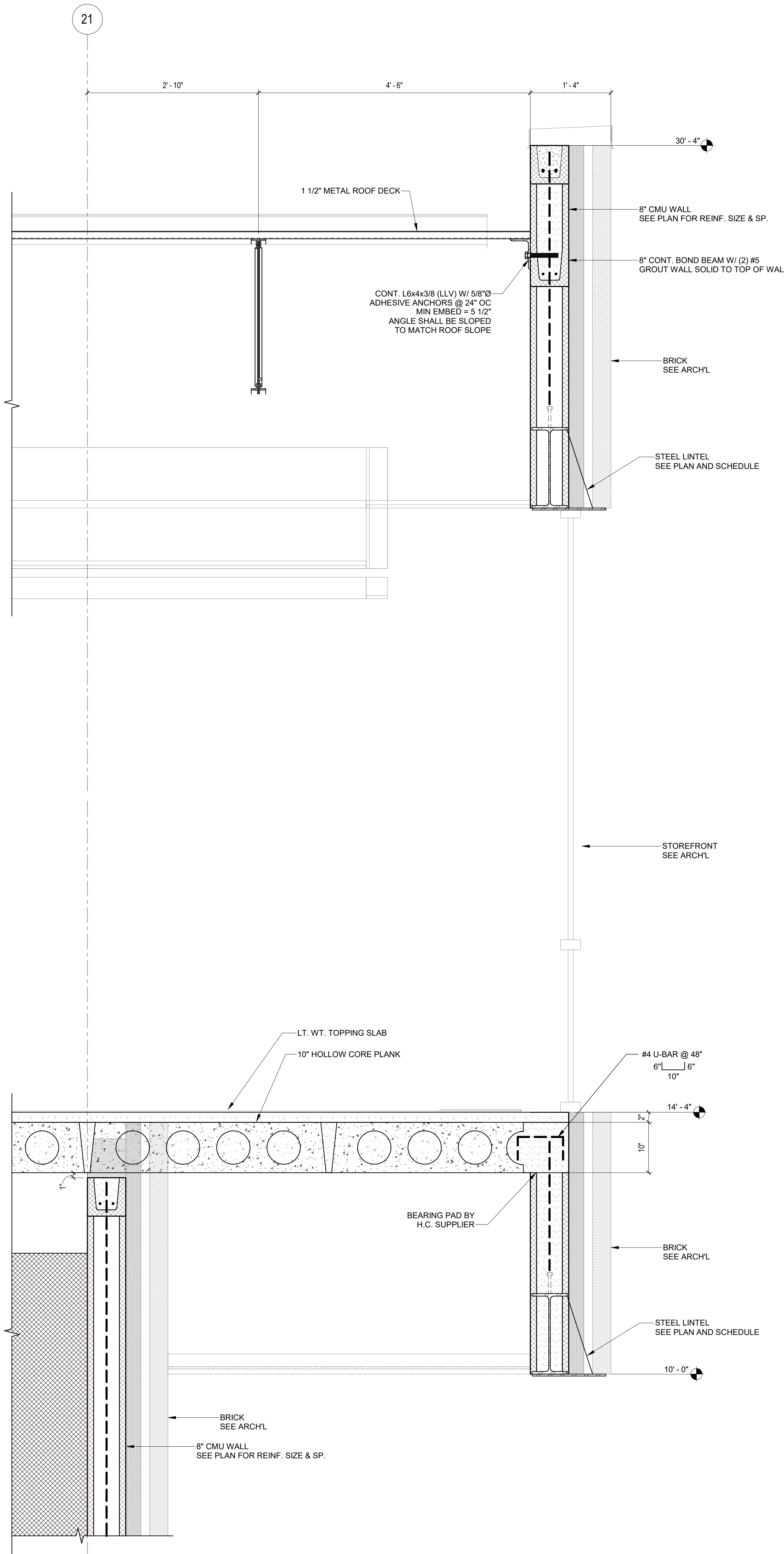
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3 WALL SECTION  
S-409 SCALE: 1" = 1'-0"



2 WALL SECTION  
S-409 SCALE: 1" = 1'-0"



1 WALL SECTION  
S-409 SCALE: 1" = 1'-0"

CONSTRUCTION  
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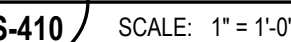


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06/12/2024		BID DOCUMENTS
ISSUE DATE:	06/12/2024	
PROJECT #:	23.08.034	
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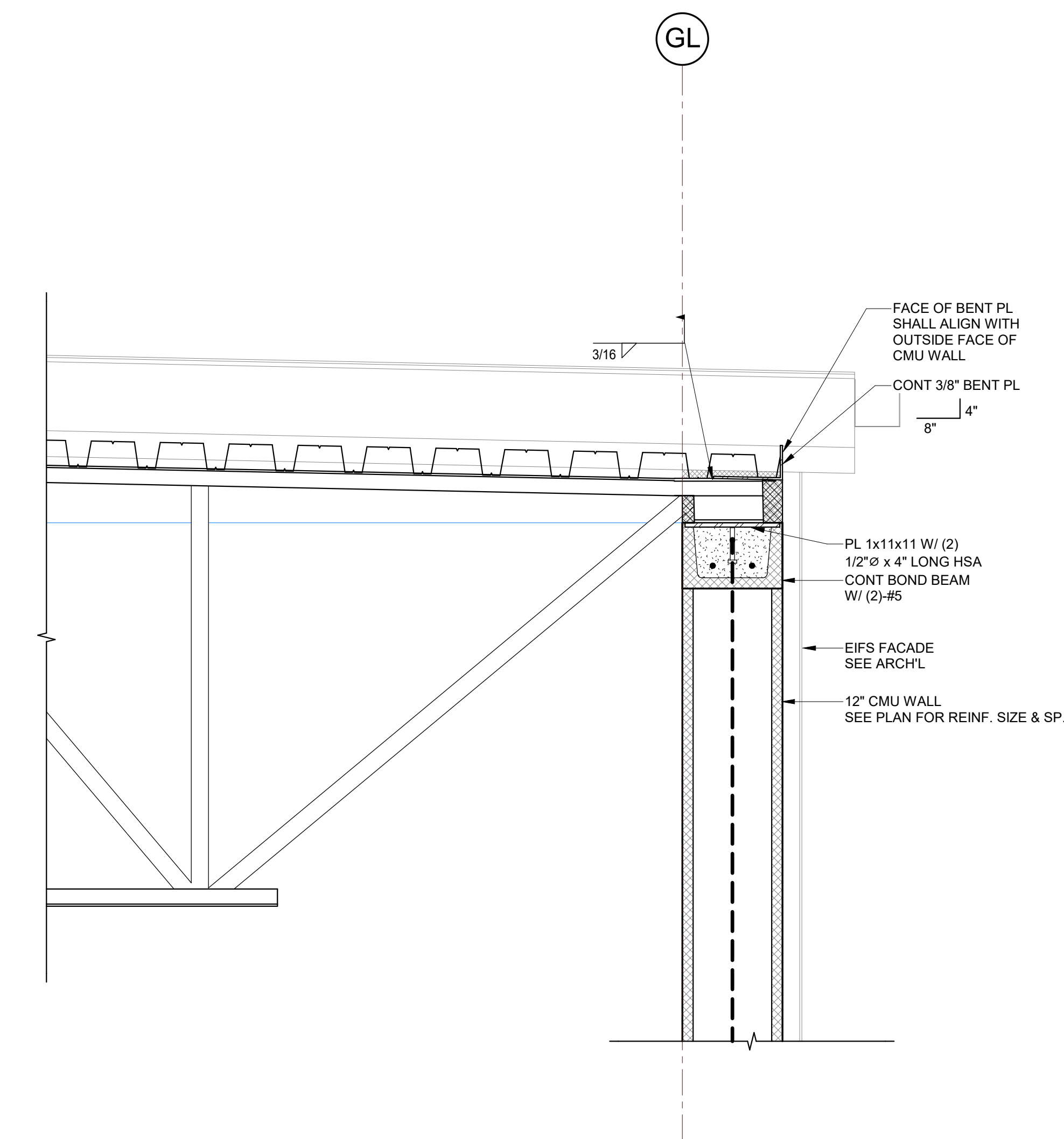
FRAMING DETAILS

S-409

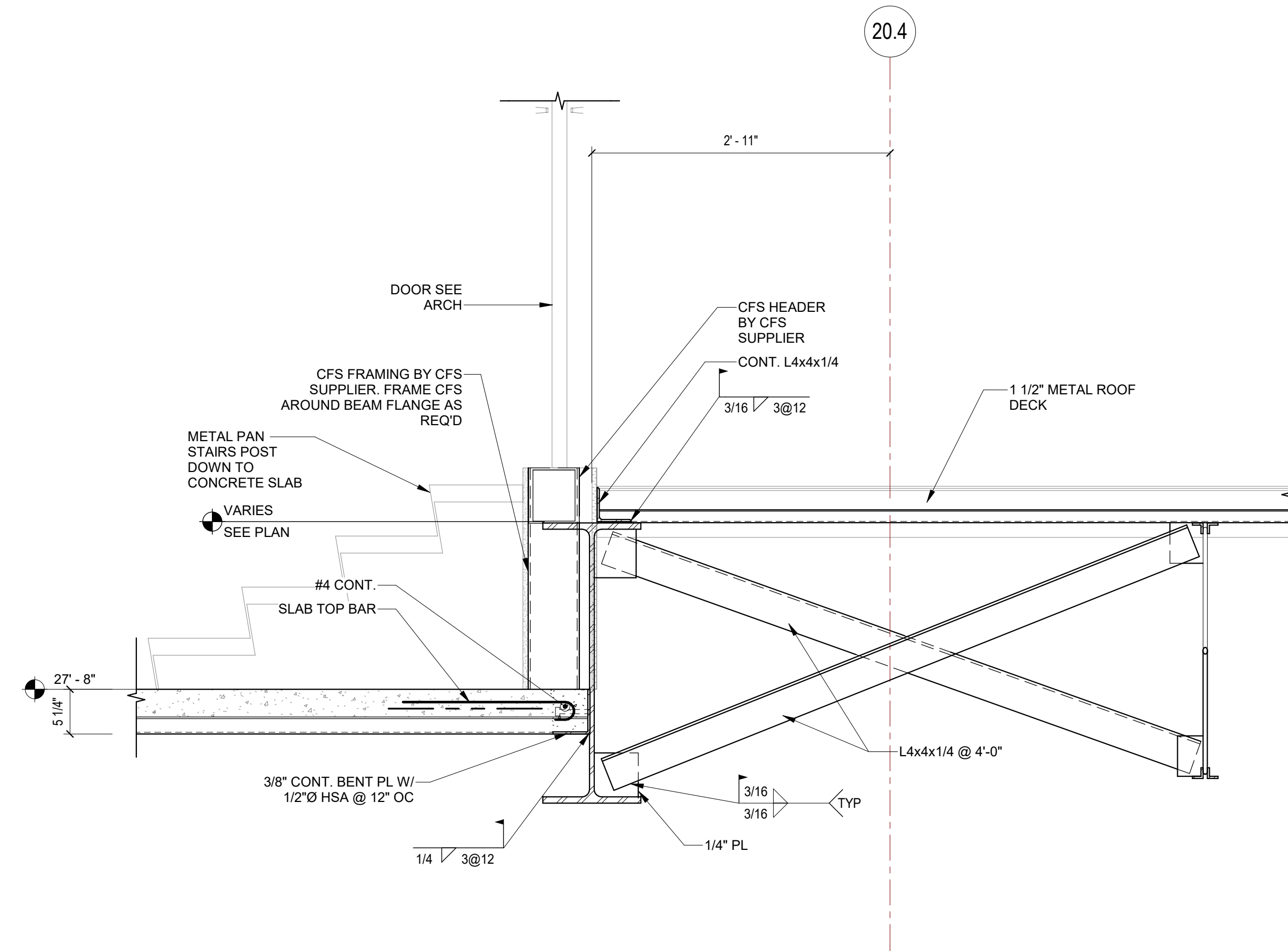




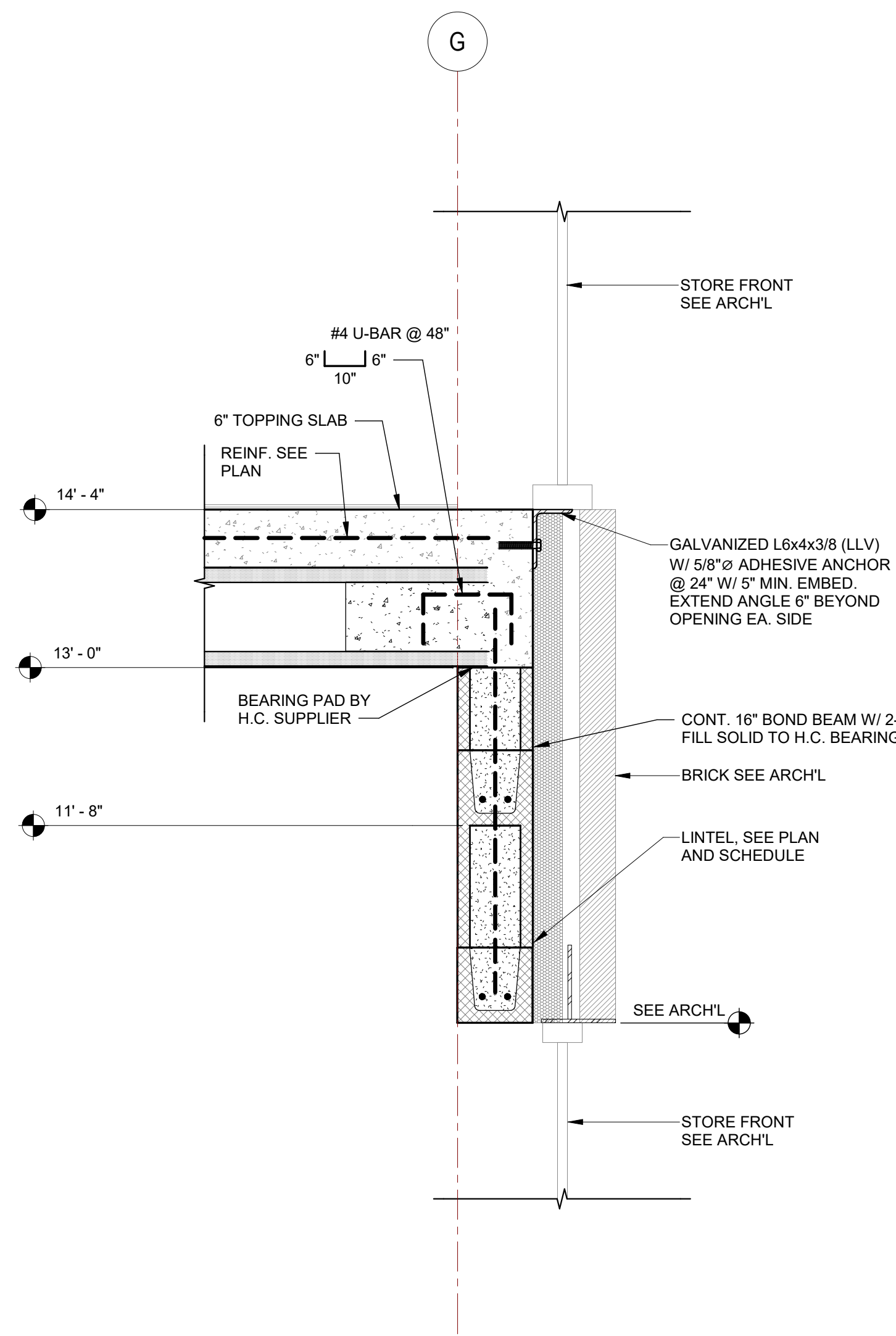




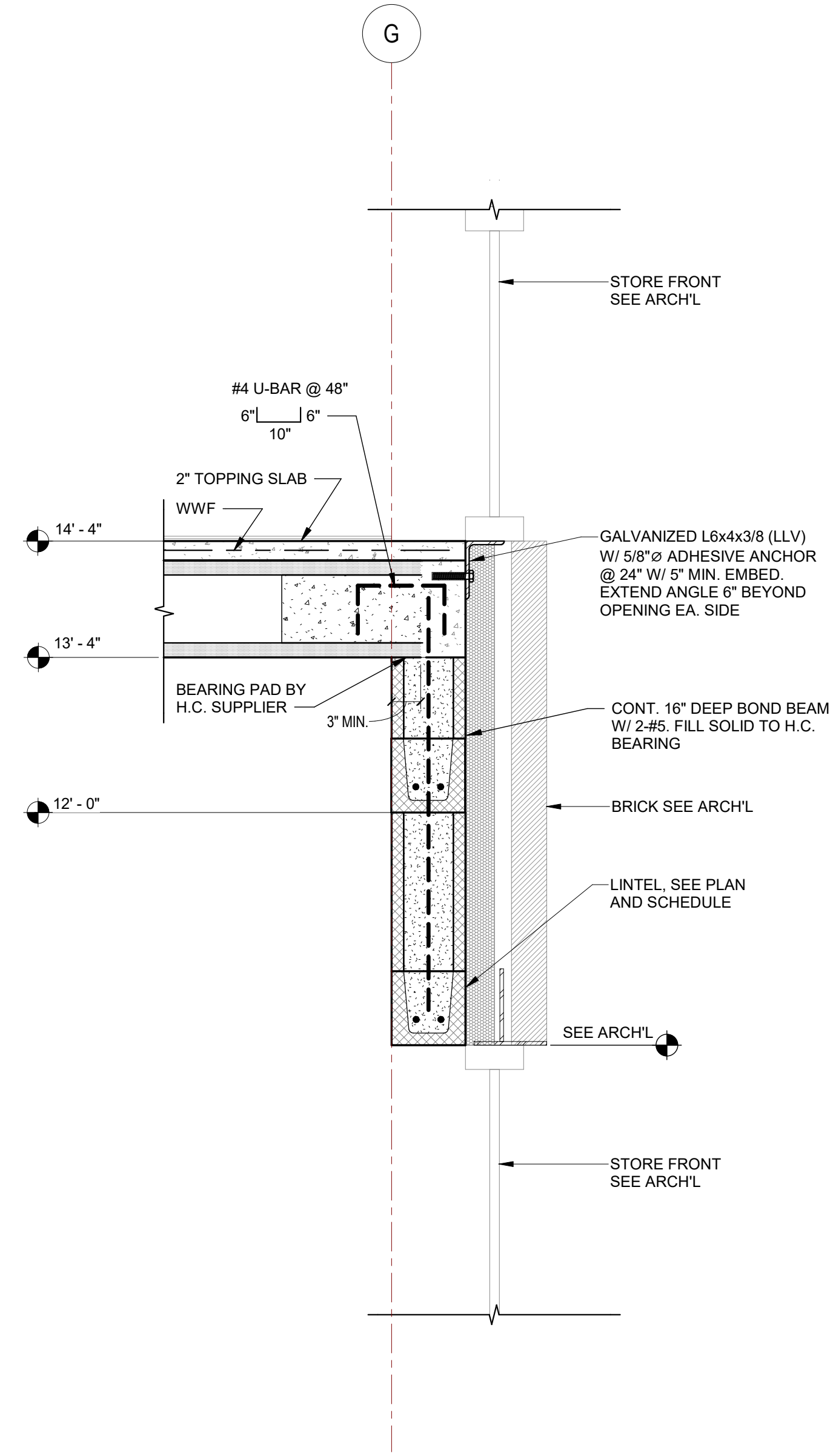
6  
S-411  
FRAMING DETAIL  
SCALE: 1" = 1'-0"



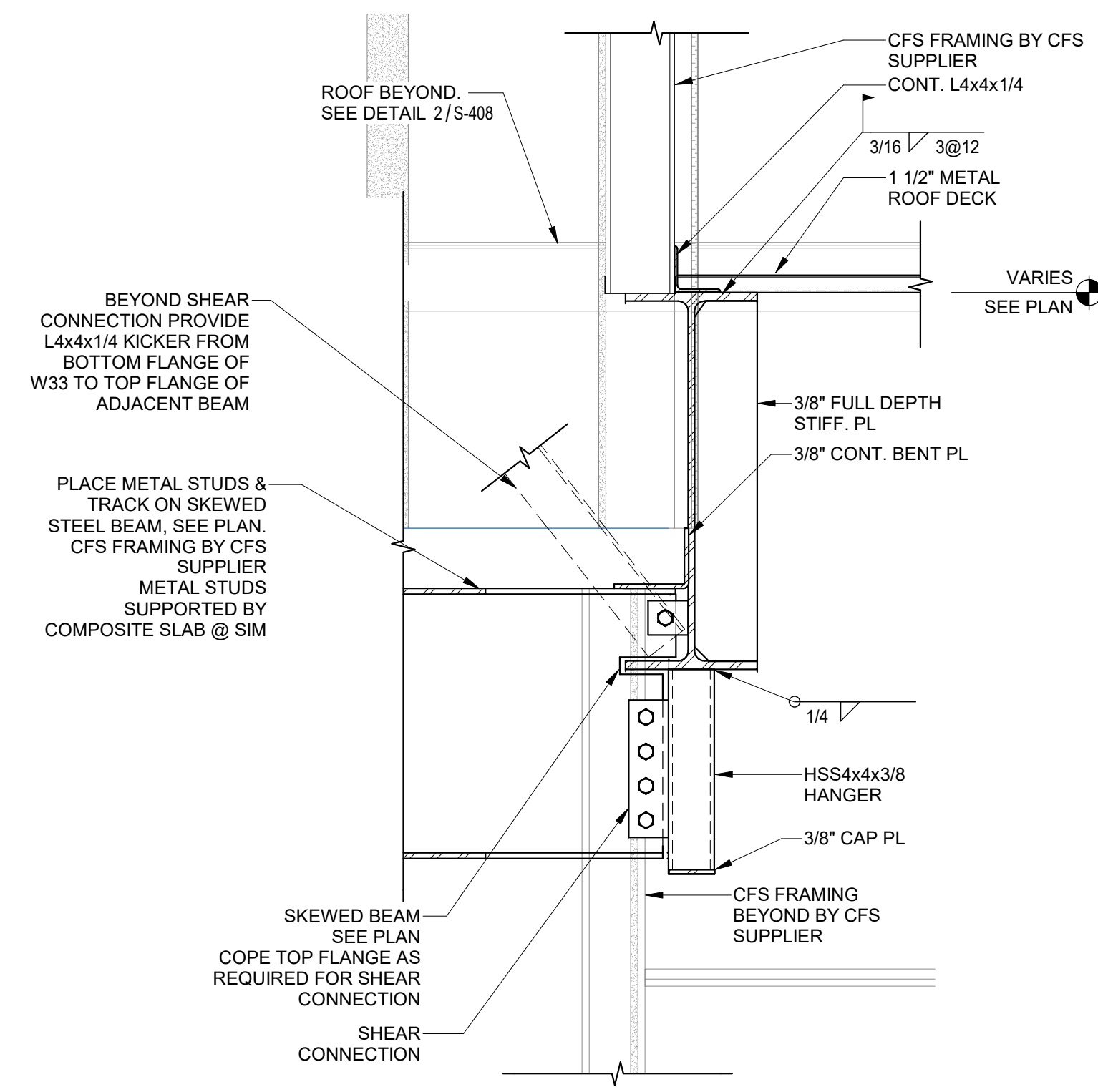
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S-411  
FRAMING DETAIL  
SCALE: 1" = 1'-0"



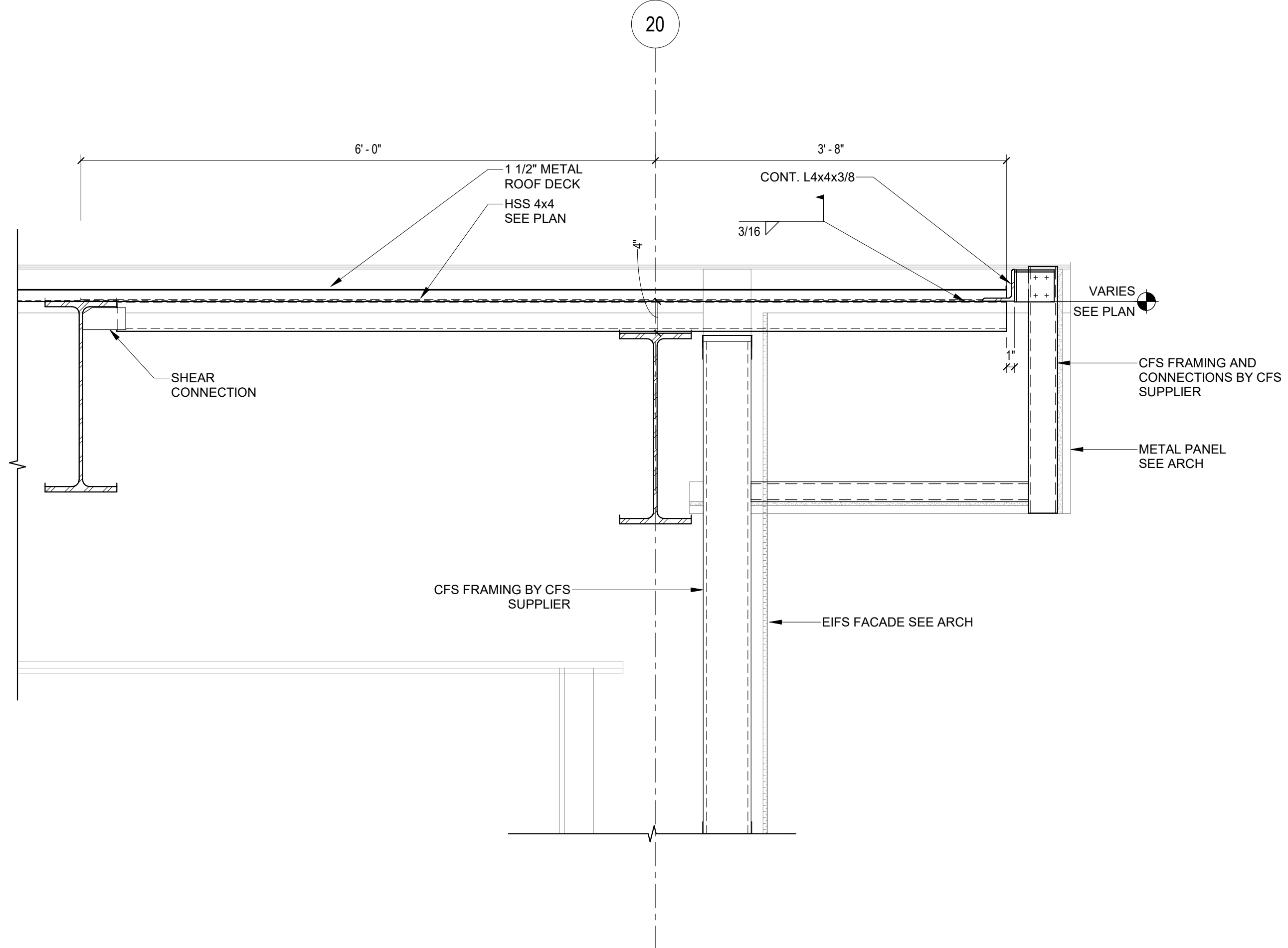
4  
S-411  
SECTION THRU FLOOR  
SCALE: 1" = 1'-0"



3  
S-411  
SECTION THRU FLOOR  
SCALE: 1" = 1'-0"



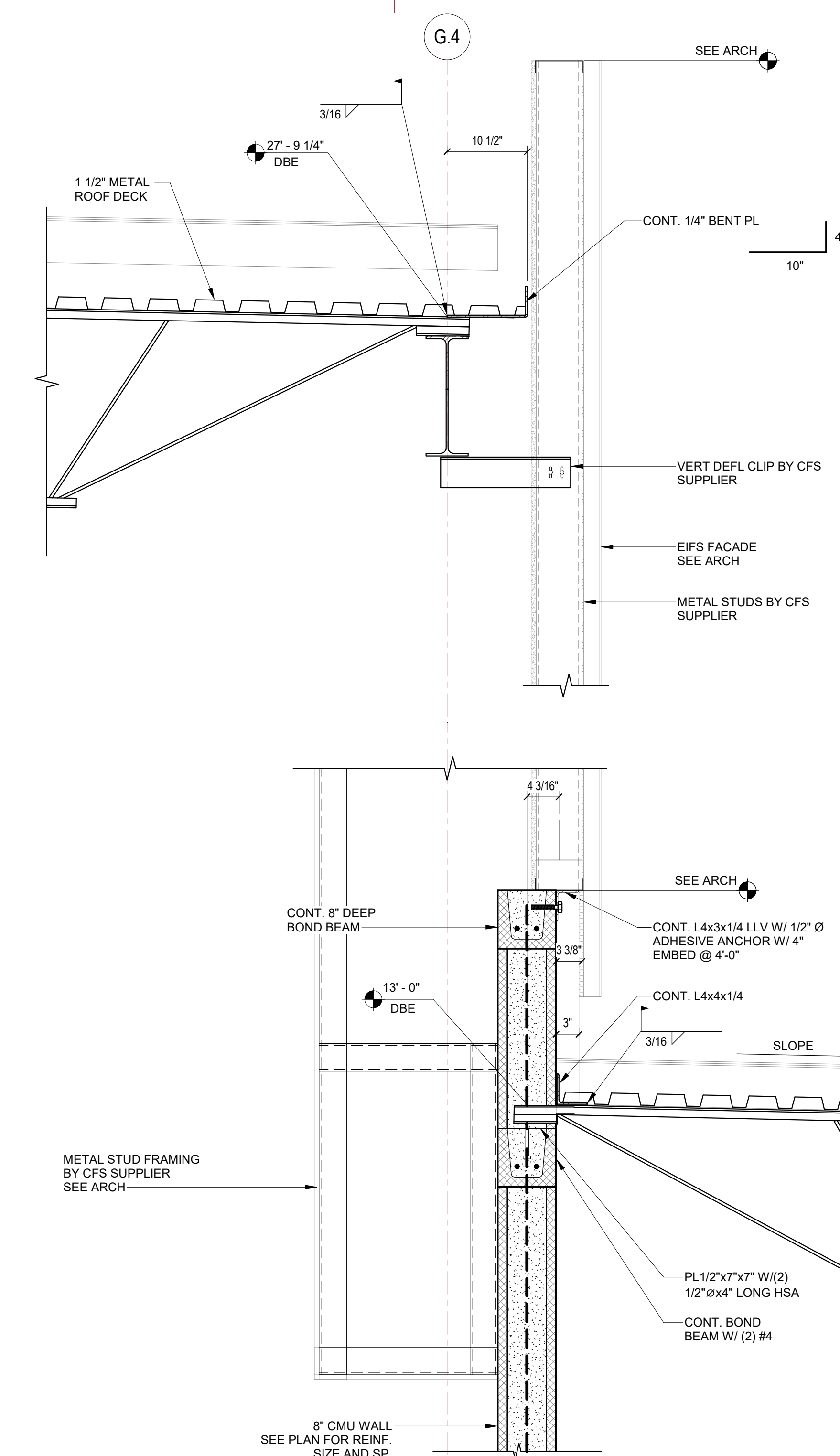
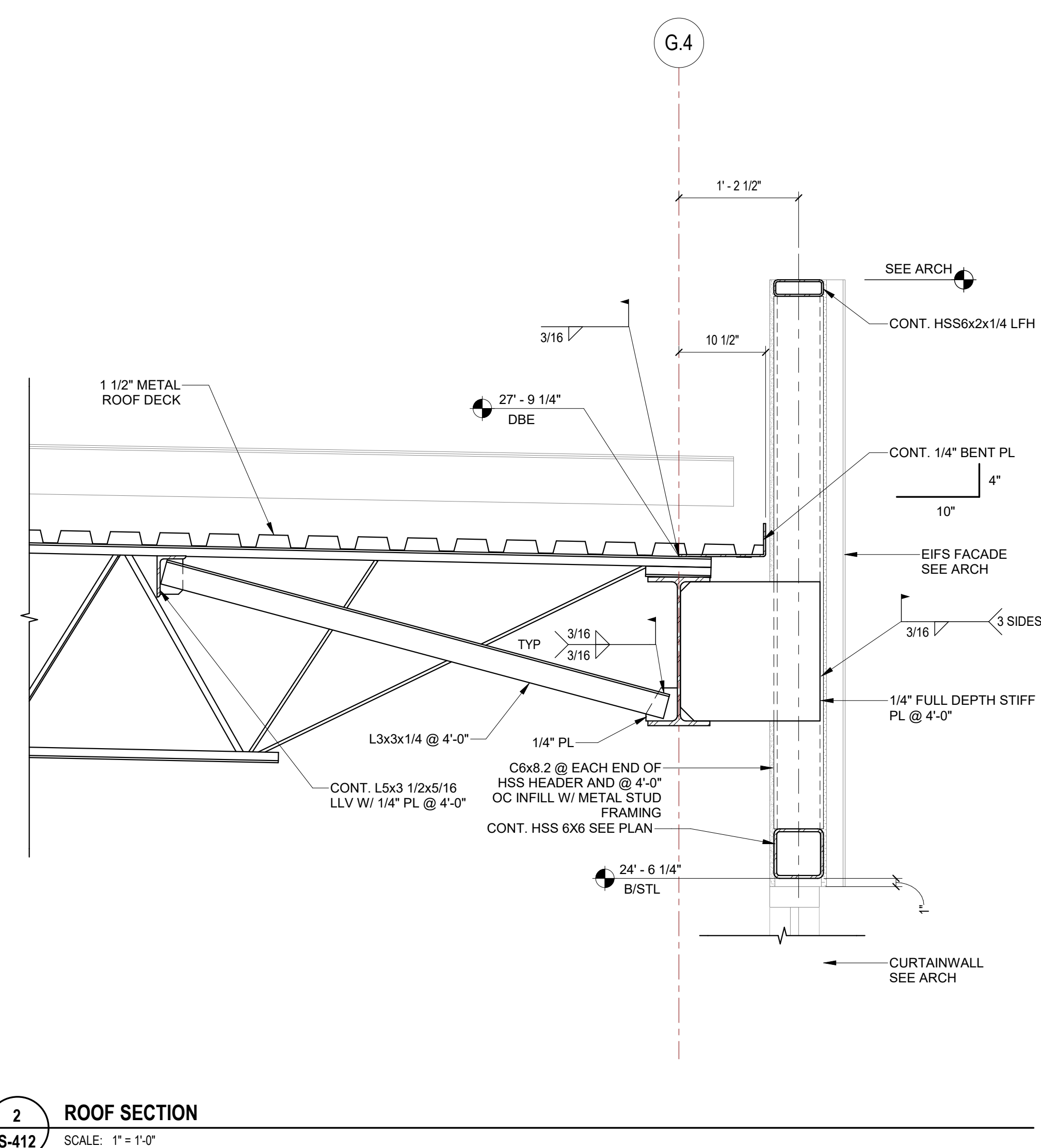
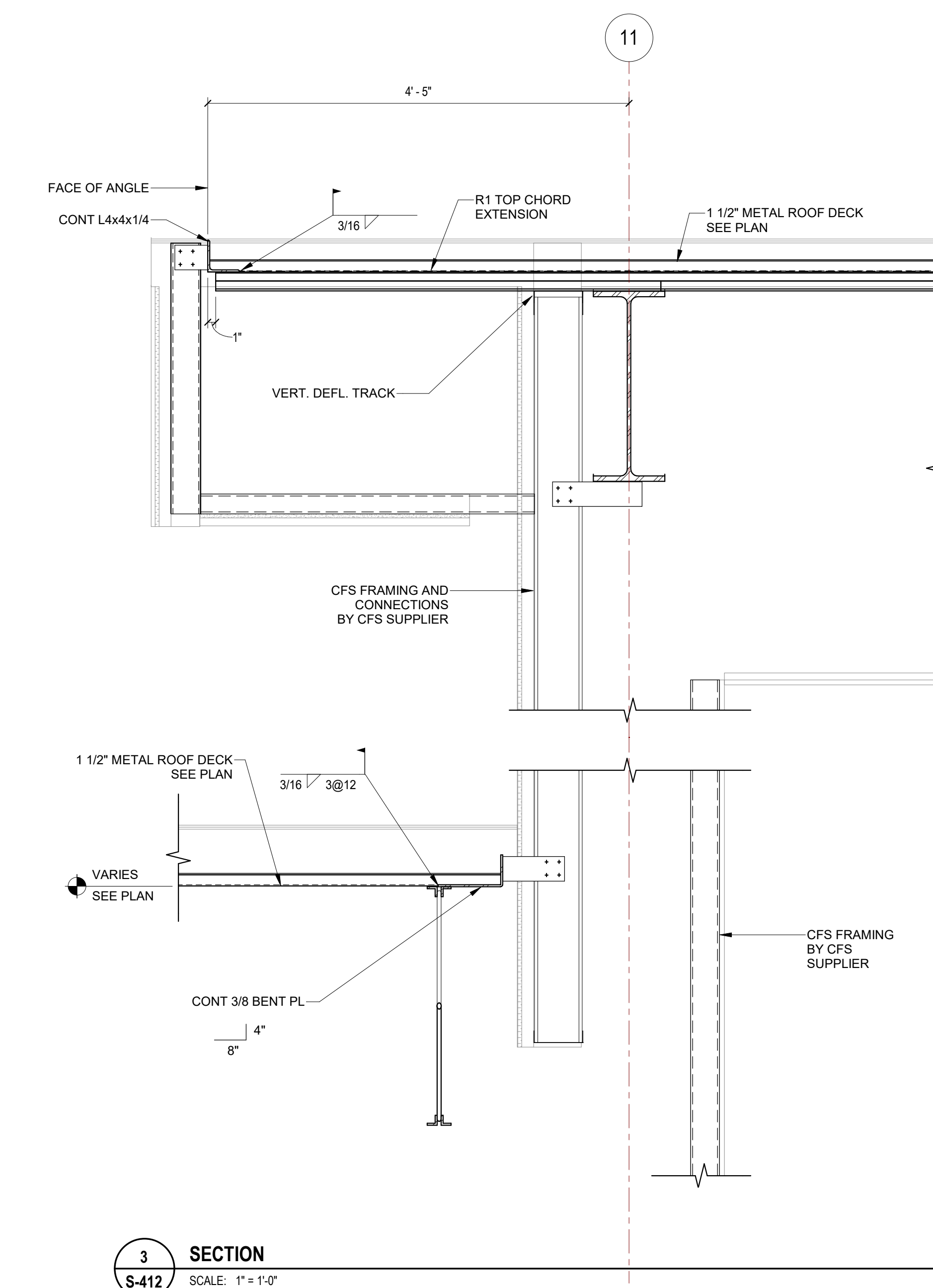
2  
S-411  
DETAIL  
SCALE: 1" = 1'-0"



1  
S-411  
HIGH ROOF SECTION  
SCALE: 1" = 1'-0"

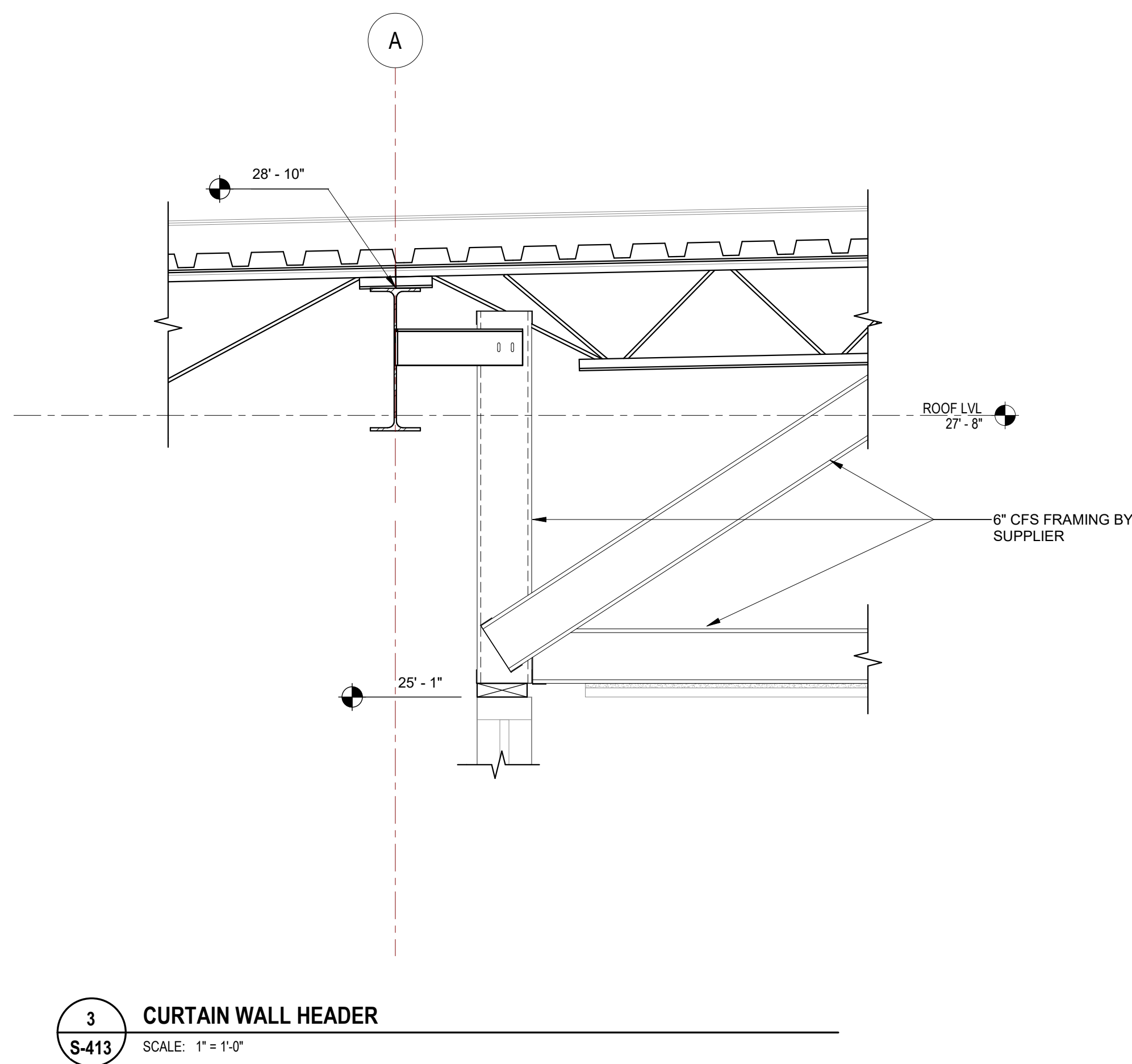


S-412

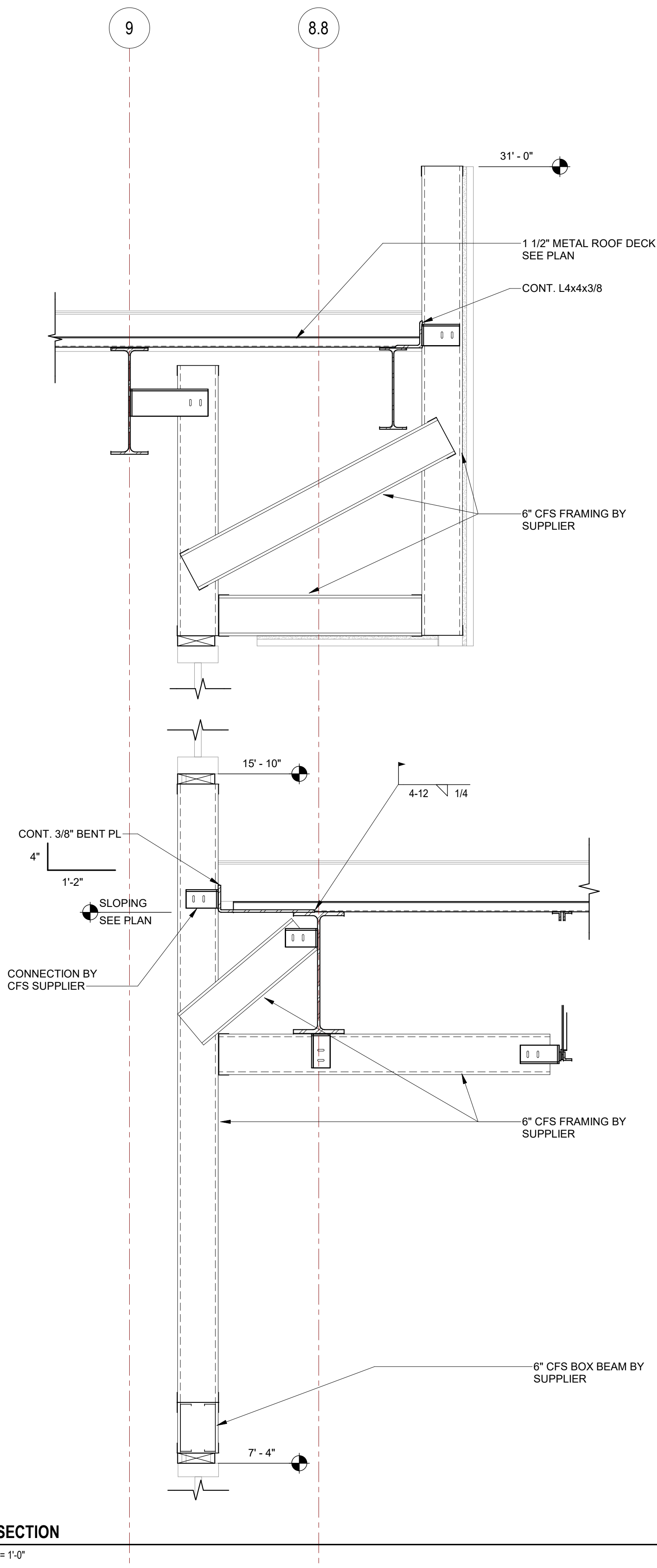




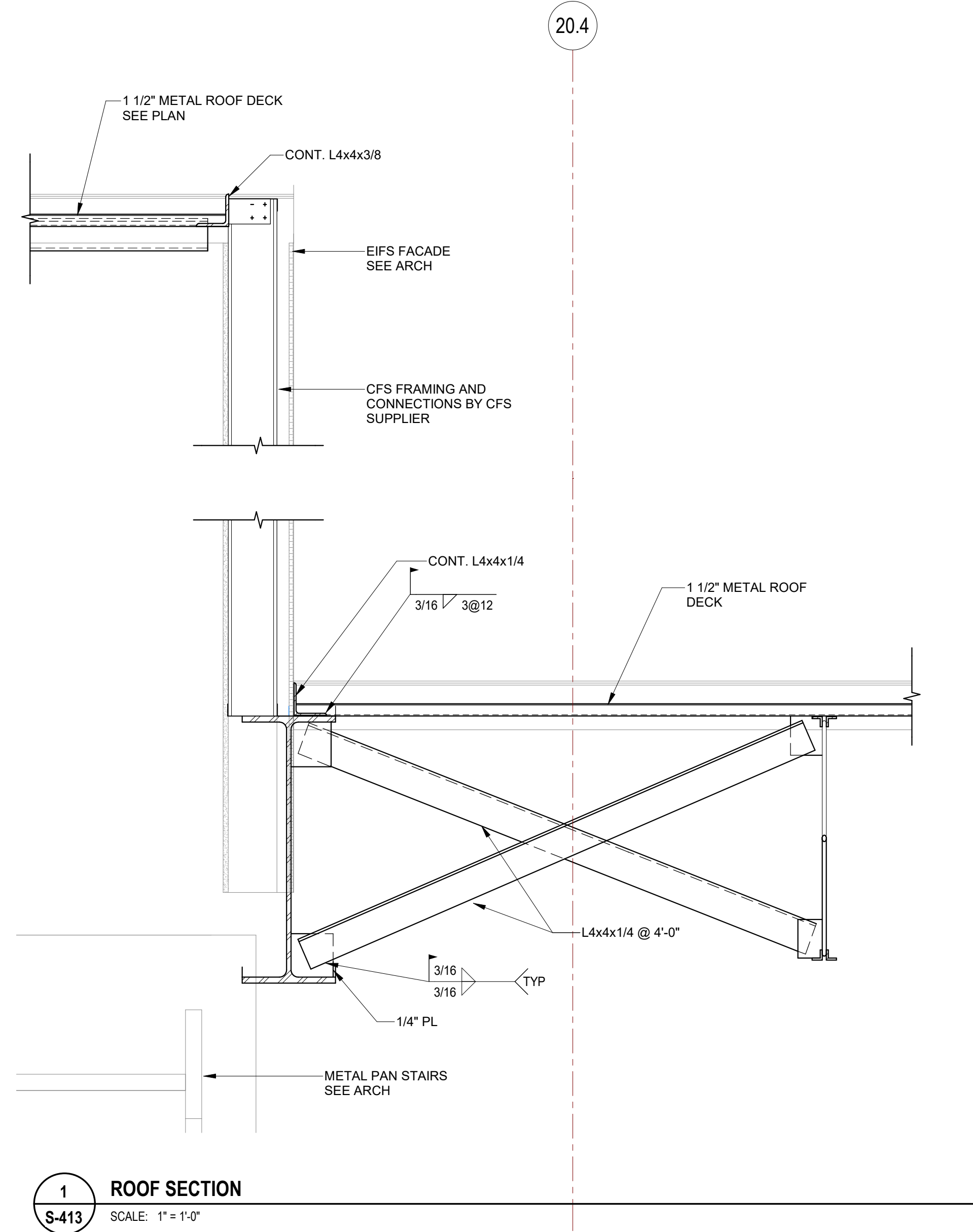
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3 CURTAIN WALL HEADER  
S-413 SCALE: 1" = 1'-0"



2 WALL SECTION  
S-413 SCALE: 1" = 1'-0"



1 ROOF SECTION  
S-413 SCALE: 1" = 1'-0"